



forestry, fisheries & the environment

Department:
Forestry, Fisheries and the Environment
REPUBLIC OF SOUTH AFRICA

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SPECIALIST DECLARATION FORM – AUGUST 2023

Specialist Declaration form for assessments undertaken for application for authorisation in terms of the National Environmental Management Act, Act No. 107 of 1998, as amended and the Environmental Impact Assessment (EIA) Regulations, 2014, as amended (the Regulations)

REPORT TITLE

Geotechnical Assessment Study for the proposed construction and maintenance of New System 1 at Rand Water Vereeniging Treatment Works, installation of approximately 7 km phase 2 Sludge Pipeline in Vereeniging, 1.5 km sludge line in Panfontein and associated infrastructure within the jurisdiction of Sedibeng District Municipality, Gauteng Province

Kindly note the following:

1. This form must always be used for assessment that are in support of applications that must be subjected to Basic Assessment or Scoping & Environmental Impact Reporting, where this Department is the Competent Authority.
2. This form is current as of August 2023. It is the responsibility of the Applicant / Environmental Assessment Practitioner (EAP) to ascertain whether subsequent versions of the form have been published or produced by the Competent Authority. The latest available Departmental templates are available at <https://www.dffe.gov.za/documents/forms>.
3. An electronic copy of the signed declaration form must be appended to all Draft and Final Reports submitted to the department for consideration.
4. The specialist must be aware of and comply with 'the Procedures for the assessment and minimum criteria for reporting on identified environmental themes in terms of sections 24(5)(a) and (h) and 44 of the act, when applying for environmental authorisation - GN 320/2020', where applicable.

1. SPECIALIST INFORMATION

Title of Specialist Assessment	Geotechnical Assessment Study for the proposed construction and maintenance of New System 1 at Rand Water Vereeniging Treatment Works, installation of approximately 7 km phase 2 Sludge Pipeline in Vereeniging, 1.5 km sludge line in Panfontein and associated infrastructure within the jurisdiction of Sedibeng District Municipality, Gauteng Province
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2. DECLARATION BY THE SPECIALIST

I, Limpho Alphonse Phatela_ declare that –

- I act as the independent specialist in this application;
- I am aware of the procedures and requirements for the assessment and minimum criteria for reporting on identified environmental themes in terms of sections 24(5)(a) and (h) and 44 of the National Environmental Management Act (NEMA), 1998, as amended, when applying for environmental authorisation which were promulgated in Government Notice No. 320 of 20 March 2020 (i.e. "the Protocols") and in Government Notice No. 1150 of 30 October 2020.
- I will perform the work relating to the application in an objective manner, even if this results in views and findings that are not favourable to the applicant;
- I declare that there are no circumstances that may compromise my objectivity in performing such work;
- I have expertise in conducting the specialist report relevant to this application, including knowledge of the Act, Regulations and any guidelines that have relevance to the proposed activity;
- I will comply with the Act, Regulations and all other applicable legislation;
- I have no, and will not engage in, conflicting interests in the undertaking of the activity;
- I undertake to disclose to the applicant and the competent authority all material information in my possession that reasonably has or may have the potential of influencing –
 - any decision to be taken with respect to the application by the competent authority; and
 - the objectivity of any report, plan or document to be prepared by myself for submission to the competent authority;
- All the particulars furnished by me in this form are true and correct; and
- I realise that a false declaration is an offence in terms of Regulation 48 and is punishable in terms of section 24F of the NEMA Act.



Signature of the Specialist

Lilibara Projects_

Name of Company:

_____ 28 - SEPTEMBER 2025 _____

Date

SPECIALIST DECLARATION FORM – AUGUST 2023

3. UNDERTAKING UNDER OATH/ AFFIRMATION

I, Limpho Alphonse Phatela, swear under oath / affirm that all the information submitted or to be submitted for the purposes of this application is true and correct.

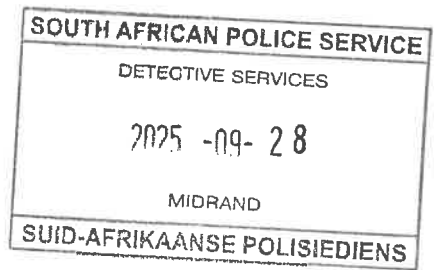
[Signature]
Signature of the Specialist

Lilibara Projects
Name of Company

28 - SEPTEMBER 2025
Date

Click or tap here to enter text. [Signature]
Signature of the Commissioner of Oaths

Click or tap to enter Date. 2025.09.28
Date



**FINAL GEOTECHNICAL INVESTIGATION REPORT ON THE
 RANDWATER PROJECT: B19 SLUDGE PIPELINE PHASE 2
 ZUIKERBOSCH-VEREENIGING**

REPORT DATE: NOVEMBER 2016

<p>PREPARED FOR: WAX ENGINEERING CONSULTANTS (PTY) LTD ELECTRICAL AND MECHANICAL</p> 	<p>PREPARED BY: LILIBARA PROJECTS (PTY) LTD CIVIL ENGINEERING AND PROJECT MANAGEMENT</p>  <p align="center">CIVIL ENGINEERING CONSULTANTS AND PROJECT MANAGERS</p>
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Executive summary

Wax Engineering Consultants (Pty) Ltd Electrical and Mechanical appointed the Geotechnical Consultants, Lilibara Projects (Pty) Ltd, to conduct geotechnical investigations for Randwater Project: B19 Sludge Pipeline Phase 2 Zuikerbosch-Vereeniging.

The regional geology along the B19 Sludge Pipeline has been retrieved from the geology map (1:250 000 scale) Sheet 2626 West-Rand. The pipeline route is underlain by predominately mudrock that have been indurated into khaki brown blotch orange and calcareous white sandy clayey silts of firm soil consistency forming concealed geological contacts with sandy shales that are weakly cemented light brown speckled yellow firm clayey sandy silt soils, interpreted as Quaternary soils deposits Aeolian / Hillwash soils.

The material along the pipeline route is classified as soft in accordance to SANS 1200D based on the test pits excavated to 3m depth therefore pipe trenches can be excavated using a Tractor-Loader-Backhoe (TLB). It should be noted however that this conclusion is based on test pits spaced at 250m centres and material properties between the test pits are extrapolated.

Unstable trench or test pit side walls were encountered in some of the test pits. It is thus recommended that the sidewalls of excavations be either battered back to 1 (vertical) to 2 (horizontal) or shored to ensure stability of the sides of the excavations.

De-watering during pipeline installation will be required in areas where groundwater seepage was encountered.

Corrosion protection of steel elements will be required because the soils are moderately corrosive.

Laboratory test results indicate that the material excavated for installation of the pipeline is unsuitable for use as pipe bedding. Suitable material will have to be imported from commercial sources in the vicinity of the project area.

Road crossings shall be done through soft material and trenchless or open trench methods of construction can be used for excavations at the crossings.

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PHASE 2 ZUIKERBOSCH-VEREENIGING**

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Annexure A - Google Earth Test Pit Positions

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1 B19 Sludge Pipeline Project Introductions

1.1 Terms of Reference

Wax Engineering Consultants (Pty) Ltd Electrical and Mechanical (the Client) appointed the Geotechnical Consultants, Lilibara Projects (Pty) Ltd, to conduct geotechnical investigations for Randwater Project: B19 Sludge Pipeline Phase 2 Zuikerbosch-Vereeniging.

1.2 Project Information

The following information was made available by the Client and used in the study, also presented within **Annexure A**.

- JEFFERES & GREEN report number 2940. Prepared by March 2012 as draft version 01 for project titled: ***Rand Water B19 Pipeline Geotechnical Report***. Authored by *Ryan Freese Engineering geologist consultant at Jefferes & Green*.
- RAND WATER. B19 Sludge Pipeline Phase 2 Zuikerbosch-Vereeniging Layout Plan CH 0.000m - CH 4964.792m. Drawn by W.K and checked by A. Mur. Drawing RW201501/ LAY/ 01, issued for information purpose only.
- Drawing No. P4444-3-67-VSL-005. B19 Sludge Pipeline Phase 2 LS1. Bosch Munitech.
- Drawing No. RW201501/ SLS / 01. B19 Sludge Pipeline Phase 2 Zuikerbosch-Vereeniging Plan and Longitudinal Section CH 0.000m - CH 2000.000m.
- Drawing No. RW201501 / SLS / 02. B19 Sludge Pipeline Phase 2 Zuikerbosch-Vereeniging Plan and Longitudinal Section CH 2000.000m - CH 4000.000m.
- Drawing No. RW201501/ SLS / 03. B19 Sludge Pipeline Phase 2 Zuikerbosch-Vereeniging Plan and Longitudinal Section CH 4000.000m - CH 4964.792m.

2 Geotechnical Scope of Work for B19 Sludge Pipeline

2.1 Geotechnical Investigation Stages

The geotechnical investigations were carried out in stages as indicated below.

Stage 1: Desktop study

Desktop study i.e. topographical maps, aerial photographs and existing records were studied to gain information on the general topography, prominent geological features, anticipated geotechnical behaviour of the ground along the pipeline route etc.

Stage 2: Field Work

This involved excavation of test pits, test pits profiling and obtaining of soil samples from the test pits for material classification and geotechnical evaluation in order to enable safe and economical design and construction of the pipeline.

Stage 3: Preparation of detailed geotechnical report

This stage involves;

- analysis of the results of the field data and laboratory tests results to provide information on excavatability, excavation stability, suitability of materials for pipe bedding etc.
- interpretation of data for use in the design.
- highlighting areas which could potentially present challenges during construction and providing solutions for such areas.

The geotechnical investigations were performed in accordance with the Code for Practice for Geotechnical Site Investigation published by South African Institute for Civil Engineers (January, 2010).

2.2 Expected geotechnical outputs

The Client provided the geotechnical investigations outputs as follows;

- *Conduct field investigations for ±8 km pipeline for 600 mm diameter steel pipeline by means of exploration trial pits excavated by an excavator. The depth of trial pits shall be 4m and at every 250m along the proposed pipeline.*

GEOTECHNICAL INVESTIGATION REPORT ON THE B19 SLUDGE PIPELINE PHASE 2 ZUIKERBOSCH-VEREENIGING

- *Geotechnical survey to be conducted on either side of all the pipe jacking crossings required.*
- *Provide the detailed description and recommendation on the stratigraphy at all anticipated pipe jack crossings.*
- *Study the usability of trench materials as bedding and selected backfill.*
- *Identify the alternative potential sources of construction materials, either in areas adjacent to the pipeline or commercial sources.*
- *Conduct soil Resistivity surveys for the entire route of the pipeline*
- *Provide recommendations on areas where the pipeline route pass through the soils with high collapsible grain structure, expansive and sink hole characteristics.*
- *Demarcate all areas of high water table, and provide recommendations and procedures for dewatering.*
- *Carry out laboratory tests on the Grading (sieve analysis and Atterberg limits), Chemical Analysis (soil and ground Water including Acid Mine Drainage (AMD)) , Moisture sensitivity analysis(Mod AASHTO), Oedometer test (stress deformation analysis) and compatibility factor tests etc.*
- *Provide excavation and backfill quantities (soft and rock) and quantities for soiled and imported materials as per SANS 1200 DB and SANS 1200 LB.*
- *Provide soil profile on the final long section that will be given to by Rand Water (RW) in DWG format.*
- *Provide trench excavations and backfilling details specific for the project.*

GEOTECHNICAL INVESTIGATION REPORT ON THE B19 SLUDGE PIPELINE PHASE 2 ZUIKERBOSCH-VEREENIGING

3 Location B19 Sludge Pipeline

Lilibara Project (Pty) Ltd has visited the site within the month of July and August to establish site access and conduct the geotechnical test pits excavation along the proposed B19 Sludge Pipeline.

3.1 Site location

B19 Sludge Pipeline and the areas trans-versed by B19 Sludge Pipeline has been presented within **Figure 1 Locality Map for B19 Sludge Pipeline**. The pipeline is drawn in light yellow colour and the dark yellow orange circles represent the geotechnical test pits spaced at 250m centre to centre.

Test pit number (TP1), coordinates, Latitude 26°41'31.59"S Longitude 27°54'35.53"E, located at Rand Water Treatment Works located at the perimeter fence entrance to the Rand Water plant in Vereeniging (Chainage 0.0m), and TP 35 is located at the Vaal River Chamber Connection point adjacent to the Vaal River (Chainage 6053.7m), Latitude 26°40'57.88"S Longitude 27°56'11.90"E.

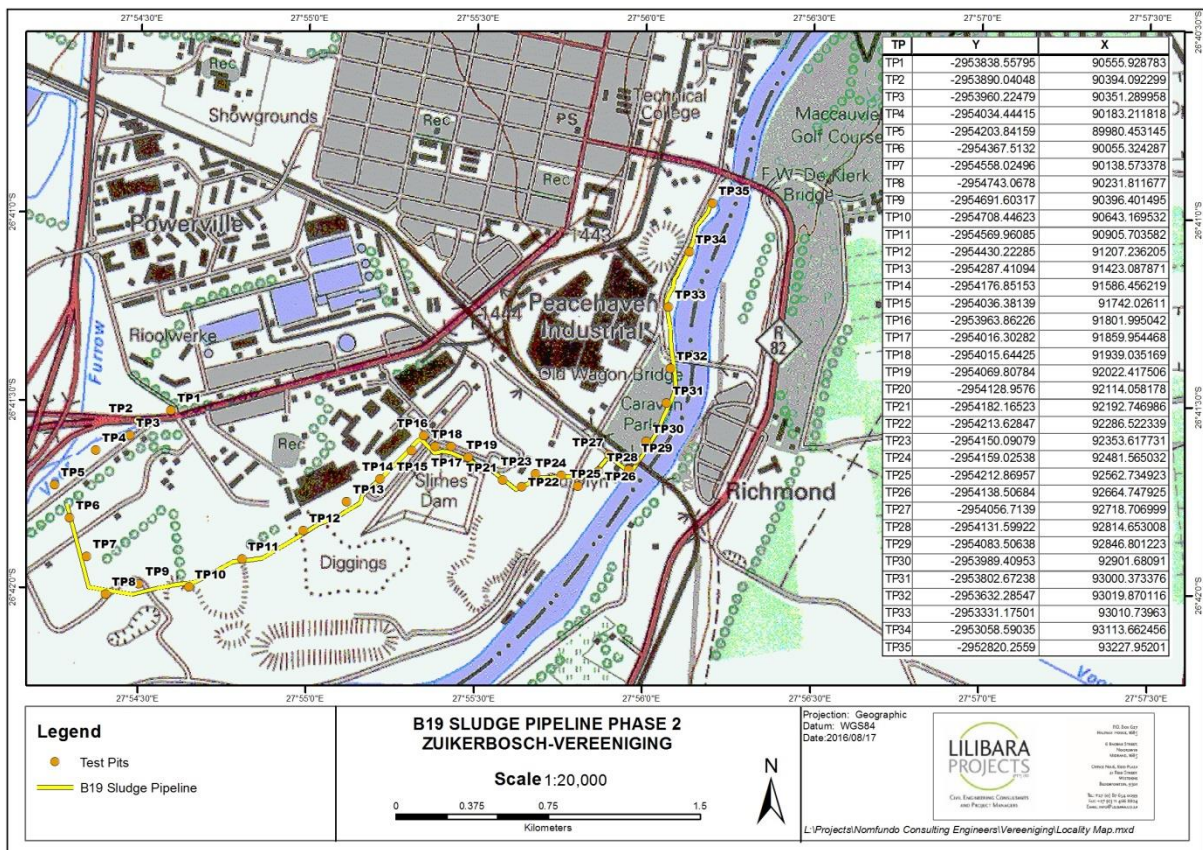


Figure 1: Locality Map for B19 Sludge Pipeline

GEOTECHNICAL INVESTIGATION REPORT ON THE B19 SLUDGE PIPELINE PHASE 2 ZUIKERBOSCH-VEREENIGING

3.2 Site Access B19 Sludge Pipeline Route

The site is located in Vereeniging, the Central Business District of Vereeniging which is situated south east of Johannesburg and accessible along R59 Dual Carriage Way that connects Vaal and Johannesburg. In Vereeniging, the B19 Sludge Pipeline route is accessible along the intersection of R42 Voortrekker Street/Barrage Road and R82 Victoria Avenue. The B19 Sludge Pipeline route is presented by test pit locations (marked as TP) along the route in the Google Earth Image, Figure 2 below.



Figure 2: Google Earth B19 Sludge Pipeline

3.3 Topography along the B19 Sludge Pipeline

The detailed site topography along the B19 Sludge Pipeline has been presented in RAND WATER. B19 Sludge Pipeline Phase 2 Zuikerbosch-Vereeniging Layout Plan, Drawing no. RW201501/LAY/01, issued for information purpose only.

The physiographical descriptors for the B19 Sludge Pipeline are describe as rolling and irregular plains of fairly low relief with high percentage of near-level land and no high steep slope. Several areas along the route are underlain by ash and mine dump backfill.

GEOTECHNICAL INVESTIGATION REPORT ON THE B19 SLUDGE PIPELINE PHASE 2 ZUIKERBOSCH-VEREENIGING

4 Geotechnical Investigation Results on B19 Sludge Pipeline

Lilibara Projects (Pty) Ltd completed field work along the B19 Sludge Pipeline route during the month of July – August 2016.

Tractor Loader Backhoe, manufactured by VOLVO was used for excavation of the test pits. The test pits were excavated to the reach of the TLB and logged by an experienced engineering geologist.



Picture taken at Rand Water Treatment Works, Test Pit number 1, Chainage 0.0m.

GEOTECHNICAL INVESTIGATION REPORT ON THE B19 SLUDGE PIPELINE
 PHASE 2 ZUIKERBOSCH-VEREENIGING

4.1 Test Pit Chainage and Coordinates for B19 Sludge Pipeline

Table 1: Geotechnical test pits chainages and coordinates for B19 Sludge Pipeline.

CHAINANGE (m)	TEST PIT NO	COORDINATES	
		LATITUDE	LONGITUDE
263.5	TP1 (RandWater Treatment Works)	26°41'31.59"S	27°54'35.53"E
369.1	TP2	26°41'33.30"S	27°54'29.69"E
499.9	TP3	26°41'35.59"S	27°54'28.16"E
647.0	TP4	26°41'38.04"S	27°54'22.10"E
957.0	TP5	26°41'43.59"S	27°54'14.81"E
1076.5	TP6	26°41'48.89"S	27°54'17.56"E
1310.5	TP7	26°41'55.06"S	27°54'20.62"E
1538.0	TP8	26°42'1.05"S	27°54'24.04"E
1703.2	TP9	26°41'59.34"S	27°54'29.98"E
1951.2	TP10	26°41'59.83"S	27°54'38.91"E
2240.2	TP11	26°41'55.27"S	27°54'48.37"E
2590.4	TP12	26°41'50.66"S	27°54'59.24"E
2855.6	TP13	26°41'45.97"S	27°55'7.01"E
3077.0	TP14	26°41'42.34"S	27°55'12.89"E
3254.0	TP15	26°41'37.74"S	27°55'18.48"E
3333.2	TP16	26°41'35.37"S	27°55'20.63"E
3411.2	TP17	26°41'37.06"S	27°55'22.74"E
3510.4	TP18	26°41'37.02"S	27°55'25.60"E
3602.1	TP19	26°41'38.76"S	27°55'28.63"E
3704.3	TP20	26°41'40.66"S	27°55'31.96"E
3798.3	TP21	26°41'42.37"S	27°55'34.82"E
3913.5	TP22	26°41'43.37"S	27°55'38.22"E
3999.5	TP23	26°41'41.29"S	27°55'40.63"E
4116.1	TP24	26°41'41.55"S	27°55'45.26"E
4209.1	TP25	26°41'43.28"S	27°55'48.21"E
4345.3	TP26	26°41'40.84"S	27°55'51.88"E
4428.3	TP27	26°41'38.17"S	27°55'53.81"E
4535.8	TP28	26°41'40.58"S	27°55'57.30"E
4601.8	TP29	26°41'39.01"S	27°55'58.45"E
4728.4	TP30	26°41'35.94"S	27°56'0.41"E
4945.7	TP31	26°41'29.85"S	27°56'3.93"E
5111.7	TP32	26°41'24.31"S	27°56'4.59"E
5419.7	TP33	26°41'14.53"S	27°56'4.18"E
5686.7	TP34	26°41'5.65"S	27°56'7.83"E
6053.7	TP35 (Vaal River Chamber Connection)	26°40'57.88"S	27°56'11.90"E

GEOTECHNICAL INVESTIGATION REPORT ON THE B19 SLUDGE PIPELINE PHASE 2 ZUIKERBOSCH-VEREENIGING

4.2 Site Geology for B19 Sludge Pipeline

The site geology has been retrieved from the regional geology map Sheet 2626 West Rand published by Council for Geosciences at a Scale of 1:250 000. West-Rand. 1986. Compiled by Keyser, N. from mapping by Nel, L. T (1931-34), Von Backstrom, J. W., (1949), Anhaeusser, C. R. (EGRU circular 62, 1971), Bredell, J.H. (1974-5) *et al.*

Sheet 2626 West Rand Geology Map showing the pipeline route is presented in Figure 3 below.

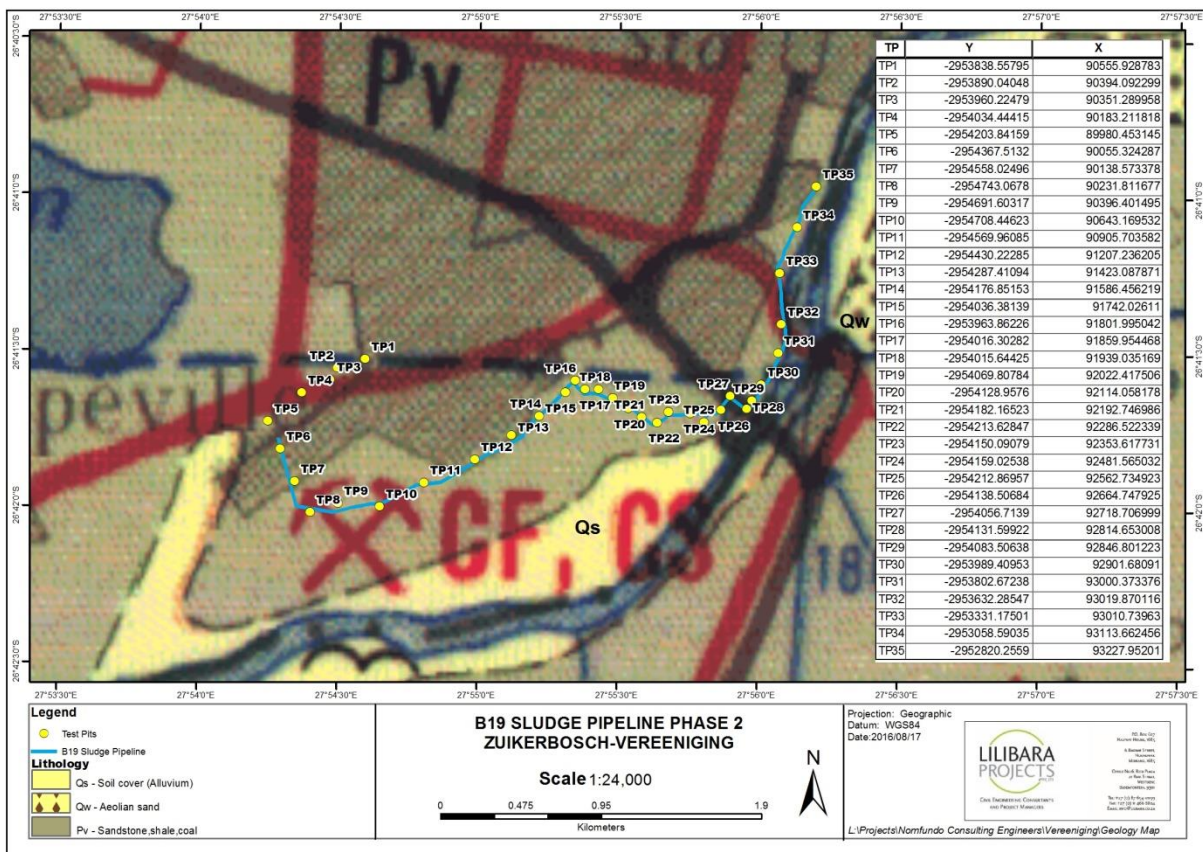


Figure 3: Regional Geology Sheet 2626 West Rand for B19 Sludge Pipeline

GEOTECHNICAL INVESTIGATION REPORT ON THE B19 SLUDGE PIPELINE PHASE 2 ZUIKERBOSCH-VEREENIGING

The regional geology along the B19 Sludge Pipeline has been retrieved from the geology map Sheet 2626 West-Rand at scale of 1:250 000. The regional geology is presented in **Table 2: Regional Geology for B19 Sludge Pipeline from Sheet 2626 West Rand** below.

Table 2: Regional Geology for B19 Sludge Pipeline from Sheet 2626 West.

SUPERGROUP	GROUP	FORMATIONS	ROCKTYPES
Quaternary		Quaternary (Qs)	Soil cover Alluvium
		Quaternary (Qa)	Soil cover Aeolian sand
Karoo Supergroup	Ecca Group (Pe): Rock types; shale, mudstone, sandstone	Volksrust (Pvo)	Shale
		Vryheid (Pv)	Sandstone, shale, coal
		Pietermaritzburg (Pp)	Shale
	Dwyka Group (C-Pd)		Tillite, sandstone, mudstone, shale

4.3 Soil horizons along B19 Sludge Pipeline

The geotechnical soil profile and photographs for the soil horizons encountered within the B19 Sludge Pipeline have been presented in the **Annexure C – Soil profiles** and **Annexure D – Soil Photographs**.

Table 3 and 4 below provide summary of the test pits data for the B19 Sludge Pipeline.

Table 3: Soil horizons along B19 Sludge Pipeline.

TEST PIT NUMBER	CHAINAGE (m)	LAYER THICKNESSES (m)																			
		TRANSPORTED SOILS											RESIDUAL SOILS								
		Topsoil/ Organic (From – to)			Fill, Ash/brick rubble (From – to)			Alluvium (From – to)		Ferruginous Hillwash (From – to)			Ferruginous residual mudstone (From – to)			Calcareous residual mudstone (From – to)			Ferruginous residual sandy shale (From – to)		
TP1	263.5	0.0	-	0.2						0.2	-	1.0	1.0	-	2.2	2.2	-	3.0		-	
TP2	369.1	0.0	-	0.2	0.2	-	0.5		-	0.5	-	1.0	1.0	-	2.0	2.0	-	3.0		-	
TP3	499.9	0.0	-	0.2	0.2	-	0.5		-	0.5	-	1.1	1.1	-	2.0	2.0	-	3.0		-	
TP4	647.0	0.0	-	0.1	0.1	-	1.0		-	1.0	-	1.5	1.5	-	2.2	2.2	-	3.0		-	
TP5	957.0	0.0	-	0.1	0.1	-	0.8		-	0.8	-	2.2	2.2	-	3.0		-			-	
TP6	1076.5	0.0	-	0.2						0.2	-	1.0	1.0	-	1.9	1.9	-	3.0		-	
TP7	1310.5	0.0	-	0.2						0.2	-	1.0	1.0	-	1.9	1.9	-	2.8		-	
TP8	1538.0	0.0	-	0.6	0.6	-	0.9		-		-		0.9	-	2.0	2.0	-	3.0		-	
TP9	1703.2	0.0	-	0.2						0.2	-	0.9	0.9	-	2.0	2.0	-	3.0		-	
TP10	1951.2	0.0	-	0.2						0.2	-	1.2	1.2	-	1.9	1.9	-	2.8		-	
TP11	2240.2	0.0	-	0.1	0.1	-	1.5		-	1.5	-	2.0	2.0	-	2.5	2.5	-	3.0		-	
TP12	2590.4	0.0	-	0.2						0.2	-	1.0	1.0	-	2.2	2.2	-	3.0		-	
TP13	2855.6	0.0	-	0.1	0.1	-	1.0		-	1.0	-	1.8							1.8	-	3.0
TP14	3077.0	0.0	-	0.1	0.1	-	1.8		-	1.8	-	2.5							2.5	-	3.0
TP15	3254.0	0.0	-	0.1	0.1	-	1.8		-	1.8	-	2.5							2.5	-	3.0
TP16	3333.2	0.0	-	0.1	0.1	-	2.0		-		-									-	
TP17	3411.2	0.0	-	0.3	0.3	-	2.0	2.0	-	3.0										-	
TP18	3510.4	0.0	-	0.3	0.3	-	1.3	1.3	-	2.0									2.0	-	3.0
TP19	3602.1	0.0	-	0.2	0.2	-	1.0	1.0	-	2.0									2.0	-	3.0
TP20	3704.3	0.0	-	0.2		-		0.2	-	2.0	2.0	-	2.6						2.6	-	3.0
TP21	3798.3	0.0	-	0.2		-		0.2	-	1.6	1.6	-	2.6						2.6	-	3.0
TP22	3913.5	0.0	-	0.2						0.2	-	0.8							0.8	-	3.0
TP23	3999.5	0.0	-	0.3						0.3	-	1.0							1.0	-	3.0
TP24	4116.1	0.0	-	0.2						0.2	-	0.9							0.9	-	3.0
TP25	4209.1	0.0	-	0.2		-		0.2	-	0.9		-							0.9	-	3.0
TP26	4345.3	0.0	-	0.2		-		0.2	-	1.0		-							1.0	-	3.0
TP27	4428.3	0.0	-	0.2		-		0.2	-	1.0		-							1.0	-	3.0
TP28	4535.8	0.0	-	0.2		-		0.2	-	0.8		-							0.8	-	3.0
TP29	4601.8	0.0	-	0.2		-		0.2	-	0.7		-							0.7	-	3.0
TP30	4728.4	0.0	-	0.2		-		0.2	-	0.7		-							0.7	-	3.0
TP31	4945.7	0.0	-	0.2		-		0.2	-	1.4	1.4	-	2.4						2.4	-	2.8
TP32	5111.7	0.0	-	0.2	0.2	-	1.1		-		-								1.1	-	3.0
TP33	5419.7	0.0	-	0.2	0.2	-	1.2		-		-								1.2	-	3.0
TP34	5686.7	0.0	-	0.2	0.2	-	1.2		-		-								1.2	-	3.0
TP35	6053.7	0.0	-	0.1		-		0.1	-	0.9		-							0.9	-	5.0

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Table 4: Test Pits summary table.

TEST PIT NUMBER	CHAINAGE	EXCAVATED TEST PIT SUMMARY			
		TRENCH DEPTH	SEEPAGE	SIDEWALLS	SAMPLES TAKEN
	(m)	(m)	(m)	Stability	Bulk (ModAASHTO & CBR); Disturbed (Atterberg Limits & pH-Conductivity)
TP1	263.5	3.0	none	stable	Disturbed and bulk samples taken at 0.15--0.5m & 0.5--1.0m & 1.0--2.2m & 2.2--2.6m.
TP2	369.1	3.0	none	stable	none
TP3	499.9	3.0	none	stable	none
TP4	647.0	3.0	none	stable	Disturbed soils samples taken at 0.1--1.0m.
TP5	957.0	3.0	none	stable	Disturbed soils samples taken at 2.2--3.0m.
TP6	1076.5	3.0	none	stable	none
TP7	1310.5	2.8	none	stable	Disturbed samples taken at 0.2--1.0m & 1.0--2.8m.
TP8	1538.0	3.0	none	stable	none
TP9	1703.2	3.0	none	stable	none
TP10	1951.2	2.8	none	stable	Disturbed and bulk samples taken at 0.2--1.2m & 1.2--3.0m.
TP11	2240.2	3.0	none	stable	none
TP12	2590.4	3.0	none	stable	Disturbed soils samples taken at 0.4--1.0m & 1.0--3.0m.
TP13	2855.6	3.0	none	stable	Bulk and disturbed soils samples taken 0.1--0.6m & 0.6--1.0m & 1.0--1.8 & 1.8--3.0m.
TP14	3077.0	3.0	3.0	1.2-1.8	Disturbed soils samples taken 0.1--1.2m & 1.8--3.0m.
TP15	3254.0	3.0	3.0	1.2-1.8	none
TP16	3333.2	2.0	none	stable	none
TP17	3411.2	3.0	2.0	stable	none
TP18	3510.4	3.0	1.3	stable	Bulk and disturbed soils samples taken 2.0--3.0m.
TP19	3602.1	3.0	1.3	stable	none
TP20	3704.3	3.0	none	stable	none

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TEST PIT NUMBER	CHAINAGE	EXCAVATED TEST PIT SUMMARY			
		TRENCH DEPTH	SEEPAGE	SIDEWALLS	SAMPLES TAKEN
	(m)	(m)	(m)	Stability	Bulk (ModAASHTO & CBR); Disturbed (Atterberg Limits & pH-Conductivity)
TP21	3798.3	3.0	none	stable	none
TP22	3913.5	3.0	none	stable	none
TP23	3999.5	3.0	none	stable	Bulk and disturbed soils samples taken at 1.0--3.0m.
TP24	4116.1	3.0	none	stable	none
TP25	4209.1	3.0	none	stable	none
TP26	4345.3	3.0	none	stable	Disturbed soils samples taken at 1.0--3.0m.
TP27	4428.3	3.0	none	stable	none
TP28	4535.8	3.0	none	stable	Bulk and disturbed soils samples taken 0.8--3.0m.
TP29	4601.8	3.0	none	stable	Bulk and disturbed soils samples taken 0.7--3.0m.
TP30	4728.4	3.0	none	stable	Bulk and disturbed soils samples taken 0.5--3.0m.
TP31	4945.7	2.8	none	stable	Bulk and disturbed soils samples taken at 0.8--2.8m.
TP32	5111.7	3.0	none	stable	Bulk and disturbed soils samples taken at 0.2--3.0m.
TP33	5419.7	3.0	none	stable	none
TP34	5686.7	3.0	none	stable	Bulk and disturbed soils samples taken at 0.4--1.2m & 1.2--2.6m & 2.6--3.0m.
TP35	6053.7	5.0	none	stable	Disturbed soils samples taken at 0.3--1.0m & 1.0--5.0m.

4.4 Laboratory tests

The following laboratory tests were performed by Geostrada which is a SANAS accredited laboratory;

- Foundation indicators (sieve analysis and Atterberg limits)
- California Bearing Ratio (CBR)
- Moisture/density relationship
- pH and conductivity

The detailed laboratory results are presented in **Annexure E** of this report.

It should be noted that not all the tests stated in the terms of reference were conducted. This is because the indicator tests showed that the in-situ material on which the pipe will be laid was poor (G9 or worse in accordance with TRH14 classification) due to high content of silt and clay therefore should be removed and replaced with imported material. Hence, further testing of this material would be a futile exercise.

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Table 5: Laboratory test results summary table.

TEST PIT NO	CHAINAGE (m)	DEPTH (m) From - to	%GRAVEL	%SAND	%SILT	%CLAY	ACTIVITY	% SOIL MORTAR	COARSE SAND RATIO	LL%	PL%	PI%	PIw%	LS%	GM	UC	CC	US CS	TRB	VAN DER MERWE'S SOIL ACTIVITY CHART
TP1	263.5	0.15 - 0.5	1	46	14	39	0.8	99	0.08	52	21	31	28.2	11	0.42	34	0	CH	A-7-6	High
TP1	263.5	0.5 - 1	10	40	13	37	1.2	90	0.1	67	22	45	36.5	12	0.67	36	0	CH	A-7-6	Very High
TP1	263.5	1 - 2.2	2	27	20	51	0.7	98	0.05	60	26	34	31.6	15.5	0.35	10	0.1	CH	A-7-6	High
TP1	263.5	2.2 - 2.6	17	20	17	46	0.8	83	0.05	62	26	36	28.4	15.5	0.75	23	23	CH	A-7-6	High
TP4	647.0	0.1 - 1	36	41	20	3	2	64	0.31	36	30	6	2.6	2	1.67	157	0.9	SM	A-1-b	Low
TP5	957.0	2.2 - 3	27	39	13	21	0.8	73	0.14	35	19	16	10.1	8.5	1.19	147	0.4	SC	A-6	Low
TP7	1310.5	0.2 - 1	0	41	19	40	0.8	100	0.05	54	21	33	31.4	13	0.31	31	0	CH	A-7-6	Very High
TP10	1951.2	0.2 - 1.2	9	45	17	29	0.8	91	0.11	41	18	23	18.6	10	0.75	61	0	CL	A-7-6	Medium
TP10	1951.2	1.2 - 3	5	42	16	37	0.7	95	0.08	45	18	27	23.5	11	0.57	37	0	CL	A-7-6	High
TP13	2855.6	0.1 - 0.6	36	41	18	5	1.2	64	0.22	37	31	6	3	3.5	1.56	221	0.5	SM	A-2-4	Low
TP13	2855.6	0.6 - 1	24	53	18	5	1.6	76	0.33	33	25	8	4.1	3.5	1.47	256	3.7	SM	A-2-4	Low
TP13	2855.6	1 - 1.8	2	62	17	19	0.6	98	0.18	26	14	12	9.6	5.5	0.77	89	0.9	SC	A-6	Low
TP13	2855.6	1.8 - 3	7	45	16	32	0.6	93	0.16	36	18	18	14	8	0.74	61	0	CL	A-6	Low
TP14	3077.0	0.1 - 1.2	53	33	11	3	1	47	0.32	38	35	3	1	1.5	2.03	286	1.1	GM	A-1-b	Low
TP14	3077.0	1.8 - 3	6	40	29	25	0.6	94	0.14	31	17	14	11.3	7	0.57	33	0.1	CL	A-6	Low - Medium
TP16	3333.2	0.2 - 3	17	47	24	12	0.9	83	0.18	30	19	11	7.5	5.5	1.09	111	1	SC	A-6	Low
TP18	3510.4	2 - 3	0	33	28	39	1	100	0.01	58	20	38	37.6	10.5	0.12	19	0.1	CH	A-7-6	Very High
TP20	3704.3	0.8 - 1.8	4	43	29	24	0.7	96	0.05	34	18	16	14.6	6	0.46	34	0.1	CL	A-6	Medium
TP20	3704.3	1.8 - 3	0	24	35	41	0.8	100	0.01	52	21	31	30.7	13	0.14	9	0.1	CH	A-7-6	High - Very High
TP21	3798.3	0.6 - 1	1	34	35	30	0.7	99	0.02	35	15	20	19.4	5.5	0.22	18	0.1	CL	A-6	Medium

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TEST PIT NO	CHAINAGE (m)	DEPTH (m) From - to	%GRAVEL	%SAND	%SILT	%CLAY	ACTIVITY	% SOIL MORTAR	COARSE SAND RATIO	LL%	PL%	PI%	Plw%	LS%	GM	UC	CC	US CS	TRB	VAN DER MERWE'S SOIL ACTIVITY CHART
TP21	3798.3	1.8 - 3	0	27	42	31	0.7	100	0.02	38	17	21	20.6	9.5	0.19	9	0.1	CL	A-6	Medium - High
TP23	3999.5	1 - 3	2	51	24	23	0.5	98	0	26	14	12	11.8	5.5	0.39	35	0.9	CL	A-6	Low - Medium
TP26	4345.5	1 - 3	1	31	38	30	0.8	99	0.04	41	17	24	22.8	9.5	0.22	10	0.1	CL	A-7-6	Medium - High
TP29	4601.8	0.7 - 3	1	88	6	5	1.8	99	0.64	19	10	9	3.2	2.5	1.52	47	5.3	SC	A-2-4	Low
TP30	4728.4	0.5 - 3	1	73	13	13	0.7	99	0.31	22	13	9	6.1	3.5	1	154	8.1	SC	A-2-4	Low
TP31	4945.7	0.8 - 2.8	0	60	25	15	0.8	100	0.15	25	13	12	10.2	3.5	0.67	61	1.7	SC	A-6	Low - Medium
TP34	5686.7	0.4 - 1.2	34	41	21	4	0	66	0.29	N/A	N/A	N/A	N/A	0	1.58	154	0.5	SM	A-2-7	Low
TP34	5686.7	1.2 - 2.6	41	36	10	13	0.8	59	0.17	21	11	10	4.9	4.5	1.66	1172	3.1	SC	A-2-4	Low
TP34	5686.7	2.6 - 3	52	30	8	10	1	48	0.17	21	11	10	4	4.5	1.9	5213	1.6	GC	A-2-4	Low
TP35	6053.7	0.3 - 1	2	30	35	33	0.7	98	0.03	39	16	23	21.9	8.5	0.34	17	0.1	CL	A-6	Medium - High
TP35	6053.7	1 - 3	22	38	19	21	0.6	78	0.12	27	14	13	9	7.5	1.09	108	0.4	SC	A-6	Low
Description of symbols																				
LL = Liquid Limit					PL = Plasticity Limit					PI = Plasticity Index					Plw = Weighted PI					
LS = Linear Shrinkage					GM = Grading Modulus					UC = Uniformity Coefficient					CC = Coefficient of Curvature					
USCS = Unified Soil Classification System																				
TRB = Committee on Classification of Materials for Subgrades and Granular Type Roads of the Highway Research Board / AASHTO Classification system																				

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Table 6: Laboratory test results summary table.

TEST PIT NO	CHAINAGE (m)	DEPTH (m) From - to	MODAASHTO		CALIFORNIA BEARING RATIO							TRH14
			Max.dry density kg/m3	Optimum moisture %	%swell	90%	93%	95%	97%	98%	100%	G-CLASS
TP1	263.5	1.0 - 2.6	1622	13.9	13.4	0.6	0.7	0.8	0.8	0.8	0.9	Less than G9
TP10	1951.2	0.2 - 1.2	1814	12	3.1	0.4	0.5	0.6	0.7	0.8	1.1	Less than G9
TP10	1951.2	1.2 - 3.0	1730	14.5	4.9	0.3	0.4	0.4	0.4	0.4	0.4	Less than G9
TP13	2855.6	0.1 - 0.6	1613	11.7	0.6	4	7	10	14	18	26	G9
TP13	2855.6	0.6 - 1.0	1817	15.4	0.3	3	8	16	30	40	75	G9
TP18	3510.4	2.0 - 3.0	1744	12.2	5.9	0.8	0.8	0.9	0.9	1	1	Less than G9
TP23	3999.5	1.0 - 3.0	1788	9.8	2.9	0.6	1.4	2.2	3.8	5	8	Less than G9
TP29	4601.8	0.7 - 3.0	2005	6.8	0	6.6	12	20	30	38	60	G8
TP31	4945.7	0.8 - 2.8	2011	8.1	0.1	2.3	6	11	21	29	43	Less than G9
TP31	4945.7	0.8 - 2.8	1904	8.4	0.1	1.1	2.3	4	5.5	7	10	Less than G9
TP34	5686.7	1.2 - 2.6	2049	8.2	0.3	1	3.2	7.1	16	23	51	Less than G9

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Table 7: pH and Conductivity results for B19 Sludge Pipeline

Test pit	Chainage (m)	Depth From - to	pH	Conductivity
			value	(siemens/meter)
TP1	263.5	1 – 2.6	7.94	0.1046
TP10	1951.2	0.2 – 1.2	7.49	0.0758
TP10	1951.2	1.2 – 3.0	7.3	0.1217
TP13	2855.6	0.1 – 0.6	9.22	0.1228
TP13	2855.6	0.6 – 1.0	9.05	0.13
TP13	2855.6	1.0 – 1.8	6.57	0.1046
TP13	2855.6	1.8 – 3.0	6.41	0.1071
TP34	5686.7	1.2 – 2.6	7.74	0.0687
TP34	5686.7	2.6 – 3.0	7.9	0.0721

4.5 Existing Geological Conditions

4.6 Soil Cover, Quaternary Deposits

This soil cover consists of various soil materials of variable origin that has been transported and deposited by the adjacent river, in the last 1.8 Ma. These alluvial sediments are generally deep, non-cohesive (sandy) to cohesive (silty) deposits, with lateral variation across the flood plain.

Due to the nature of their deposition these soils have a loose consistency. The loose nature of the soils may lead to settlement of foundations and instability problems during excavation. The loose and voided nature also means that these soils are highly permeable.

4.7 Aeolian Sand, Quaternary Deposits

These deposits represent young (<1.8 Ma) sandy soils that have undergone transportation and deposition by wind. The deposits are derived from the weathering products of the Karoo Supergroup and typically have a loose, voided, non-cohesive structure susceptible to settlement and collapse potential. Due to their loose, voided nature these soils may be highly permeable.

4.8 Vryheid Formation, Ecca Group, Karoo Supergroup

The bedrock geology consists of sedimentary rock units of the Vryheid Formation of the Karoo Supergroup. The Karoo Supergroup represents a sedimentary sequence and makes up South Africa's largest occurring rock group at surface. The Vryheid Formation is a mixture of sandstone, conglomerates and shales derived from a deltaic ingress and regress in the Palaeozoic Era.

Dolerite intrusions are common within the Vryheid Formation, however no intrusions are indicated on the geological map in close proximity to the pipeline route. The Vryheid Formation is generally upward coarsening and in the western part of Gauteng Province it is thought to have been deposited by linear coastline cycles.

The deposition of the Vryheid Formation involved fluvial processes and these remnant structures can be seen in the rock, such as paleo-ripples and pebble immature sandstone sediments. Sandstone

(and less commonly shale) rock can lead to excavation difficulties for the pipeline trench in areas underlain at shallow depth by the Karoo Supergroup bedrock.

4.9 Groundwater Seepage

Much of the pipeline is located within close proximity to the Vaal River, and is at risk of inundation during major flooding of the river.

For period on which the trenches were open, there was groundwater seepage observed within the test pits summarised within **Table 21: Groundwater Seepage within B19 Sludge Pipeline.**

Table 8: Groundwater Seepage along B19 Sludge Pipeline

TEST PIT NUMBER	CHAINAGE (m)	DEPTH TRENCH (m)	SEEPAGE (m)	SIDEWALLS Stability
TP14	3077.0	3.0	3.0	1.2-1.8 (unstable)
TP15	3254.0	3.0	3.0	1.2-1.8 (unstable)
TP17	3411.2	3.0	2.0	stable
TP18	3510.4	3.0	1.3	stable
TP19	3602.1	3.0	1.3	stable
TP24	4116.1	3.0	3.0	stable
TP25	4209.1	3.0	3.0	stable
TP26	4345.3	3.0	3.0	stable
TP27	4428.3	3.0	3.0	stable
TP28	4535.8	3.0	3.0	stable

Free standing water was encountered in a few of the test pits presented in figure 5 and 6 below.

The water logged areas also overlain by dumped ash, presumably transported and dumped by previous contractors for the construction of haulage roads.

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The study was conducted in the month of July to August, which is a dry season and several test pits within the study only showed slightly moist conditions. Water seepage could be experienced in these test pits during rainy season.



Figure 4: Water Logged Areas along the B19 Sludge Pipeline _ TP 13 to TP 20

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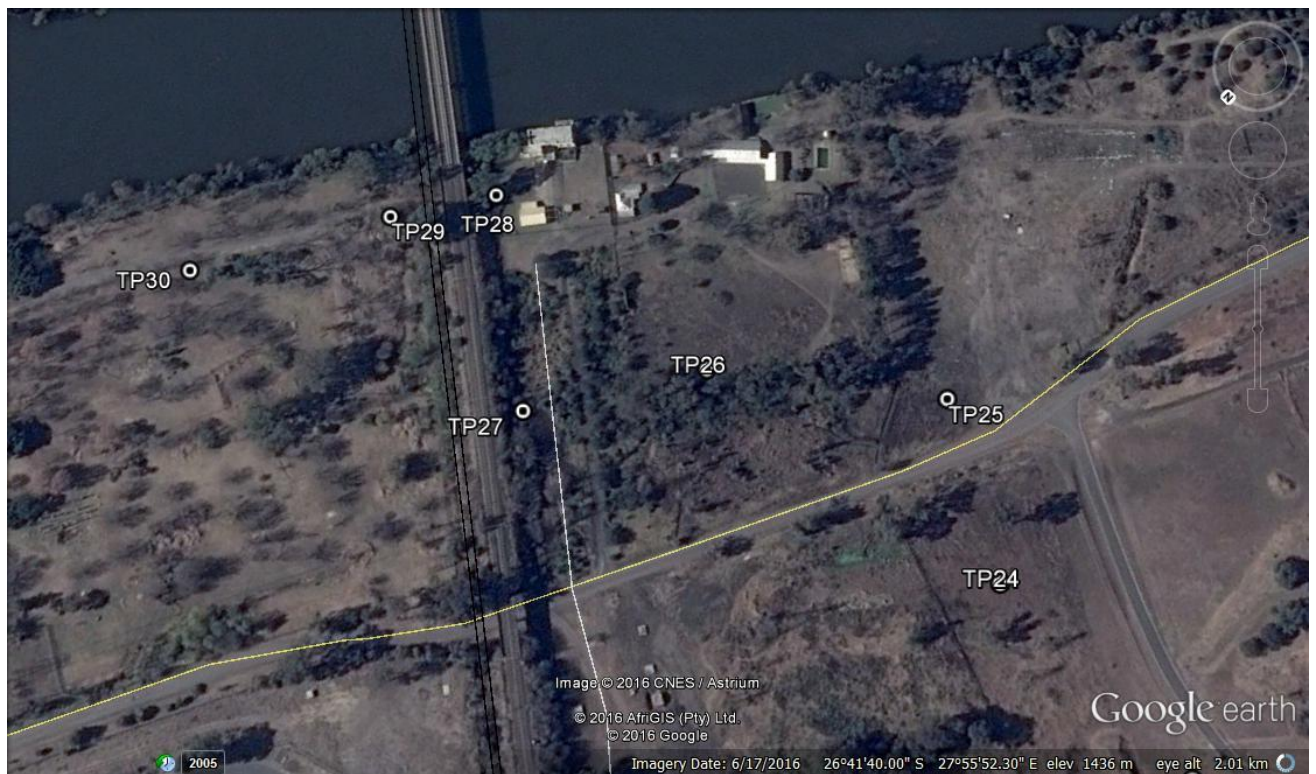


Figure 5: Water Logged Areas along the B19 Sludge Pipeline _ TP 24 to TP 28

The groundwater seepage encountered along the B19 Sludge Pipeline as well as surface runoff mean that the excavations will be flooded during construction. Dewatering of the pipe excavations shall therefore be required so that material and pipe placing can continue without disruption.

4.10 Excavatability

Excavation classification of the material along the pipeline route has been carried out in accordance with SANS 1200D in non-restricted areas as indicated below.

- Soft excavation
- Intermediate excavation
- Hard rock excavation
- Boulder excavation class A
- Boulder excavation class B

The excavation is classified as soft to at least 5m depth. Excavation of this would require manual excavation with shovels and picks or use of a TLB. The table below summarises the Classes of excavation anticipated along the pipeline route.

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Table 9: Classes of excavation for B19 Sludge pipeline

TEST PIT NUMBER	CHAINANGE (m)	DEPTH OF EXCAVATION (m)	CLASSES OF EXCAVATIONS B19 SLUDGE PIPELINE				
			Soft excavation	Intermediate excavation	Hard rock excavations	Boulder excavation Class A	Boulder excavation Class B
TP1 (RandWater Treatment Works)	263.5	3.0	yes	n/a	n/a	n/a	n/a
TP2	369.1	3.0	yes	n/a	n/a	n/a	n/a
TP3	499.9	3.0	yes	n/a	n/a	n/a	n/a
TP4	647.0	3.0	yes	n/a	n/a	n/a	n/a
TP5	957.0	3.0	yes	n/a	n/a	n/a	n/a
TP6	1076.5	3.0	yes	n/a	n/a	n/a	n/a
TP7	1310.5	2.8	yes	n/a	n/a	n/a	n/a
TP8	1538.0	3.0	yes	n/a	n/a	n/a	n/a
TP9	1703.2	3.0	yes	n/a	n/a	n/a	n/a
TP10	1951.2	2.8	yes	n/a	n/a	n/a	n/a
TP11	2240.2	3.0	yes	n/a	n/a	n/a	n/a
TP12	2590.4	3.0	yes	n/a	n/a	n/a	n/a
TP13	2855.6	3.0	yes	n/a	n/a	n/a	n/a
TP14	3077.0	3.0	yes	n/a	n/a	n/a	n/a
TP15	3254.0	3.0	yes	n/a	n/a	n/a	n/a
TP16	3333.2	3.0	yes	n/a	n/a	n/a	n/a
TP17	3411.2	3.0	yes	n/a	n/a	n/a	n/a
TP18	3510.4	3.0	yes	n/a	n/a	n/a	n/a
TP19	3602.1	3.0	yes	n/a	n/a	n/a	n/a
TP20	3704.3	3.0	yes	n/a	n/a	n/a	n/a
TP21	3798.3	3.0	yes	n/a	n/a	n/a	n/a

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TEST PIT NUMBER	CHAINANGE (m)	DEPTH OF EXCAVATION (m)	CLASSES OF EXCAVATIONS B19 SLUDGE PIPELINE				
			Soft excavation	Intermediate excavation	Hard rock excavations	Boulder excavation Class A	Boulder excavation Class B
TP22	3913.5	3.0	yes	n/a	n/a	n/a	n/a
TP23	3999.5	3.0	yes	n/a	n/a	n/a	n/a
TP24	4116.1	3.0	yes	n/a	n/a	n/a	n/a
TP25	4209.1	3.0	yes	n/a	n/a	n/a	n/a
TP26	4345.3	3.0	yes	n/a	n/a	n/a	n/a
TP27	4428.3	3.0	yes	n/a	n/a	n/a	n/a
TP28	4535.8	3.0	yes	n/a	n/a	n/a	n/a
TP29	4601.8	3.0	yes	n/a	n/a	n/a	n/a
TP30	4728.4	3.0	yes	n/a	n/a	n/a	n/a
TP31	4945.7	2.8	yes	n/a	n/a	n/a	n/a
TP32	5111.7	3.0	yes	n/a	n/a	n/a	n/a
TP33	5419.7	3.0	yes	n/a	n/a	n/a	n/a
TP34	5686.7	3.0	yes	n/a	n/a	n/a	n/a
TP35 (Vaal River Chamber Connection)	6053.7	5.0	yes	n/a	n/a	n/a	n/a
Percentage of excavation class			Soft excavation 100%	Intermediate excavation 0%	Hard rock excavation 0%	Boulder excavation Class A = 0%	Boulder excavation Class B = 0%

5 Engineering Behavior

5.1 Soil Classification and Behavior

According to the laboratory test results, the anticipated engineering properties and behavior of the soil along the pipeline is provided in the table below.

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Table 10: Soil properties along the pipe length

Test Pit No. /Chainage	Soil Classification	Expected behavior	Remarks
TP1 to TP4/Ch.264 to Ch.647	CH (Inorganic clays of high plasticity)	The high plasticity clay soil has high swell potential when exposed to moisture. This could result in excessive swell pressures on the pipe base as well as sidewalls.	The clay layer below at the bottom of the trench should be excavated and replaced with inert granular material. The distance between the pipe and trench wall should also be increased beyond what the standard requires, alternatively a rigid pipe should be used.
TP4 to TP5/ Ch.647 to Ch.957	SM (Silty sands, sand-silt mixtures), SC (Clayey sands, sand-clay mixtures)	The silty sand and clayey silt soil has low swell potential.	No precautions required on the pipe foundation. Side walls to be battered to avoid soil collapse into the excavation.
TP5 to TP10/Ch.957 to Ch.1951	CH (Inorganic clays of high plasticity), CL (Inorganic clays of low to medium plasticity)	The low to high plasticity clay soil exhibits medium low to high swell potential when exposed to moisture. This could result in excessive swell pressures on the pipe base as well as sidewalls.	The clay below at the bottom of the trench should be excavated and replaced with inert granular material. The distance between the pipe and trench wall should also be increased beyond what the standard

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			requires, alternatively a rigid pipe should be used.
TP10 to TP18/Ch.1951 to Ch.3510	SM (Silty sands, sand-silt mixtures), SC (Clayey sands, sand-clay mixtures), CL (Inorganic clays of low to medium plasticity)	The profile consists of silty sand, clayey sand and low plasticity clay. The swell potential is low to medium. Upper section between TP 13 to TP 19 consists of ASH fill. Groundwater was also encountered in this section.	The pipe foundation should be stable and only requires ripping and re-compaction before pipe bedding is placed. Dewatering of the trench will be required during excavation.
TP18 to TP28/Ch.3510 to Ch.4536	CH (Inorganic clays of high plasticity), CL (Inorganic clays of low to medium plasticity)	The medium to high plasticity clay soil exhibits medium low to high swell potential when exposed to moisture. This could result in excessive swell pressures on the pipe base as well as sidewalls. Groundwater was also encountered in this section (TP 24 to TP28).	The clayey material at the bottom of the trench should be excavated and replaced with inert granular material. The distance between the pipe and trench wall should also be increased beyond what the standard requires, alternatively a rigid pipe should be used. Dewatering of the trench will be required during excavation.
TP28 to TP34/Ch.4536 to Ch.5686.7	SC (Clayey sands, sand-clay mixture), SM (Silty sands, sand-silt mixtures)	The clayey and silty sand exhibits low swell potential.	Pipe foundation treatment will require reaping and re-compaction. Sidewalls will have to be battered to avoid soil collapse into the excavation.

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TP34 to TP35/Ch.5686.7 to Ch.6053.7	CL (Inorganic clays of low to medium plasticity)	The medium to high plasticity clay soil exhibits medium low to high swell potential when exposed to moisture. This could result in excessive swell pressures on the pipe base as well as sidewalls.	The clayey material at the bottom of the trench should be excavated and replaced with inert granular material. The distance between the pipe and trench wall should also be increased beyond what the standard requires, alternatively a rigid pipe should be used. Dewatering of the trench will be required during excavation.
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The geotechnical parameters of the soils along the pipeline have been estimated for the purposes of designing associated structures along the pipeline, e.g. valve chambers, etc. The soil on the pipe invert consists of low plasticity clays, high plasticity clays, silty sand and clayey sands as indicated in the table above. These values are provided in the table below;

5.2 Geotechnical parameters

Table 11: Geotechnical parameters for design

Soil Classification	Cohesion (kPa)	Angle of Friction (degrees)	Elastic Modulus E - MPa	Bearing Capacity (kPa)
CH and CL Inorganic clays	20 - 50	20 - 25	10 - 20	50 - 100

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SM and SC (Silty and clayey sands)	0	30 - 32	10 - 25	50 - 100
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5.3 Trench stability

Although the trenches were vertical and did not collapse during excavation, the trenches will be open for much longer during placing of the pipeline hence, it is recommended that the sidewalls of excavations be battered back to 1 (vertical) to 1.5 (horizontal) where wet conditions exist. In dry conditions, 1 (vertical) to 1 (horizontal) slopes can be used. If vertical slopes are excavated, shoring be used to ensure stability of the sides of the excavations.

5.4 Material Usability

The material obtained from the test pits indicate that it is unsuitable for use as backfill and general fill for the pipeline due to high content of silt and clay which results in low strength and high plasticity index. Even though Rand Water allows a higher value of 15 for plasticity index compared to SANS standards which allows 6 for selected backfill, it will be difficult to excavate and separate the material with PI of 15 or less as it appears in patches from different places along the pipeline route. Suitable material for backfill and selected fill will have to be obtained from commercial sources. The material is however suitable as general backfill provided oversized material, bigger than 150mm diameter is removed.

5.5 Compaction Next to Existing Structures

Stability next to existing pipeline and other adjacent structures should be ensured during compaction of the pipe bedding, selected and general backfill material.

Maximum size of equipment based on permissible vibration level is provided in the Table below.

Commercial and industrial buildings maybe able to tolerate a large vibration level of 20 mm/second.

Conversely, historical buildings and buildings with existing cracks would typically be tolerate significant less vibration 2 to 4 mm/second.

The table is based on a permissible peak particle velocity of 10 mm/second.

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Table 12: Minimum recommended distance from vibrating rollers (adopted after Tynan, 1973).

Roller class	Weight range	Minimum distance to nearest building
Very light	<1.25 tonne	Not restricted for normal road use, 3.0 m
Light	1 – 2 tonnes	Not restricted for normal road use, 5.0 m
Light to medium	2 – 4 tonnes	5 – 10 m
Medium to heavy	4 – 6 tonnes	Not advised for city and suburban streets 10 – 20m
Heavy	7 – 11 tonnes	Not advised for built up areas 20 – 40 m

5.6 Corrosivity

Corrosion potential of the soils on which the pipeline will be laid was assessed from the results obtained from pH and conductivity testing. The results indicate that the material is moderately corrosive and therefore corrosion protection would have to be provided for the steel pipe and concrete structures mix designs of concrete structures should allow for the corrosive nature of the soil.

Engineering and Exploration Geophysical Services, a geophysical exploration specialist company, were appointed to conduct soil resistivity survey to determine the corrosivity of the soil.

The report indicates that the soil corrosion potential is mostly moderate with a few areas being either severe or mild.

The results of the resistivity survey are included in Annexure G of this report.

6 Crossings

Crossings along the pipeline route are indicated below. Two test pits were excavated on either side of major road crossings while one test pit was excavated for minor roads.

Crossing	Chainage (m)	Test Pit No.	Description
Road crossing 1	263.5	1	Minor road crossing
Road crossing 2	369.1 and 499.9	2 and 3	Major road crossing
Road crossing 3	3913.5	22	Minor road crossing
Road crossing 4	3999.5	23	Minor road crossing
Road crossing 5	4209.1	25	Minor road crossing
Railway bridge crossing 1	4535.8 and 4601.8	28, 29	Difficult access

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Road crossing 6	5111.7	32	Minor road crossing
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Major road means a lot with a high traffic volume and a minor road has low traffic volume.

The geotechnical conditions at the crossings are described below.

Road crossing 1 – Test Pit No. 1

The soil profile down the test pit is as follows;

Depth below ground level	Material description
0m to 0.2m	Clayey sandy silt with roots. Topsoil.
0.2m to 0.5m	Sandy clayey silt with roots. Hillwash
0.5m to 1.0m	Firm, pinhole, clayey sandy silt with calcrete concretion. Ferruginous Hillwash.
1.0m to 2.2m	Firm, weakly cemented, sandy clayey silt with ferricrete concretion. Ferruginous Residual Mudstone.
2.2m to 2.6m	Firm, weakly cemented, sandy clayey silt with ferricrete concretion. Calcareous Residual Mudstone.
2.6m to 3.0m	Firm, weakly cemented, clayey sandy silt with ferricrete concretion. Calcareous Residual Mudstone.

Road crossing 2 – Test Pit No.2

Depth below ground level	Material description
0m to 0.2m	Clayey sandy silt with roots. Topsoil.
0.2m to 0.5m	Soft, pinhole, sandy clayey silt with roots. Fill.

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0.5m to 1.0m	Soft, intact, clayey sandy silt with subrounded fine ferricrete concretions. Ferruginous Hillwash.
1.0m to 2.0m	Firm, weakly cemented, sandy clayey silt with ferricrete concretion. Ferruginous Residual Mudstone.
2.0m to 2.7m	Firm, weakly cemented, sandy clayey silt with ferricrete concretion. Calcareous Residual Mudstone.
2.7m to 3.0m	Firm, weakly cemented, clayey sandy silt with ferricrete concretion. Calcareous Residual Mudstone.

Road crossing 2 – Test Pit No.3

Depth below ground level	Material description
0m to 0.2m	Firm, pinhole voided, clayey sandy silt with roots. Topsoil.
0.2m to 0.5m	Soft, pinhole, sandy clayey silt with roots. Fill.
0.5m to 1.1m	Soft, intact, clayey sandy silt with subrounded fine ferricrete concretions. Ferruginous Hillwash.
1.1m to 2.0m	Firm, weakly cemented, sandy clayey silt with ferricrete concretion. Ferruginous Residual Mudstone.
2.0m to 2.5m	Firm, weakly cemented, sandy clayey silt with ferricrete concretion. Calcareous Residual Mudstone.
2.5m to 3.0m	Firm, weakly cemented, clayey sandy silt with ferricrete concretion. Calcareous Residual Mudstone.

Road crossing 3 – Test Pit No.22

Depth below ground level	Material description
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0m to 0.2m	Firm, pinhole voided, clayey sandy silt with roots. Topsoil.
0.2m to 0.8m	Soft, weakly cemented, clayey sandy silt with ferricrete concretion. Hillwash.
0.8m to 2.5m	Firm, weakly cemented, clayey sandy silt with ferricrete concretion. Ferruginous Sandy Shale.
2.5m to 3.0m	Firm, weakly cemented, clayey sandy silt with ferricrete concretion. Ferruginous Sandy Shale.

Road crossing 4 – Test Pit No.23

Depth below ground level	Material description
0m to 0.2m	Firm, pinhole voided, clayey sandy silt with roots. Topsoil.
0.2m to 0.4m	Soft, weakly cemented, clayey sandy silt with ferricrete concretion. Hillwash.
0.4m to 1.0m	Soft, weakly cemented, clayey sandy silt with ferricrete concretion. Hillwash.
1.0m to 2.2m	Firm, weakly cemented, clayey sandy silt with ferricrete concretion. Ferruginous Sandy Shale.
2.2m to 3.0m	Firm, weakly cemented, clayey sandy silt with ferricrete concretion. Ferruginous Sandy Shale.

Road crossing 5 – Test Pit No.25

Depth below ground level	Material description
0m to 0.2m	Slightly moist, brown, firm, pinhole voided, clayey sandy silt with roots. Topsoil.

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0.2m to 0.4m	Moist, black, soft, clayey sandy silt. Alluvium.
0.4m to 0.9m	Moist, black, soft, weakly cemented, clayey sandy silt with ferricrete concretion. Alluvium.
0.9m to 2.0m	Slightly moist, khaki brown mottled orange, firm, weakly cemented, clayey sandy silt with ferricrete concretion. Ferruginous Sandy Shale.
2.0m to 3.0m	Slightly moist, khaki mottled olive, firm, weakly cemented, clayey sandy silt with ferricrete concretion. Ferruginous Sandy Shale.

Railway Crossing No. 1 – Test Pit 28

Depth below ground level	Material description
0m to 0.2m	Firm, pinhole voided, clayey sandy silt with roots. Topsoil.
0.2m to 0.5m	Soft, clayey sandy silt. Alluvium.
0.6m to 0.8m	Soft, weakly cemented, clayey sandy silt with ferricrete concretions. Alluvium.
0.8m to 2.0m	Firm, weakly cemented, clayey sandy silt with ferricrete concretions. Ferruginous Sandy Shale.
2.0m to 3.0m	Firm, weakly cemented, clayey sandy silt with ferricrete concretions. Ferruginous Sandy Shale.

Railway Bridge Crossing No. 1 – Test Pit 29

Depth below ground level	Material description
0m to 0.2m	Firm, pinhole voided, clayey sandy silt with roots. Topsoil.

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0.2m to 0.5m	Soft, clayey sandy silt. Alluvium.
0.6m to 0.7m	Soft, weakly cemented, clayey sandy silt with ferricrete concretion. Alluvium.
0.7m to 2.2m	Firm, weakly cemented, clayey sandy silt with ferricrete concretion. Ferruginous Sandy Shale.
2.2m to 3.0m	Firm, weakly cemented, clayey sandy silt with ferricrete concretion. Ferruginous Sandy Shale.

Road crossing 6 – Test Pit No.32

Depth below ground level	Material description
0m to 0.2m	Firm, pinhole voided, clayey sandy silt with roots. Topsoil.
0.2m to 0.6m	Soft, weakly cemented, clayey sandy silt with ferricrete concretion. Hillwash.
0.6m to 1.1m	Loose, pinhole, sandy ash. Fill.
1.1m to 2.0m	Firm, weakly cemented, clayey sandy silt with ferricrete concretion. Ferruginous Sandy Shale.
2.0m to 3.0m	Firm, weakly cemented, clayey sandy silt with ferricrete concretion. Ferruginous Sandy Shale.

It is anticipated that soft ground conditions will be encountered at all the crossings above and trenchless technology, e.g. horizontal directional drilling or pipe jacking can be used for construction of the pipeline across major road crossings. This trenchless technology can also be used for minor road crossings. Alternatively, open trenches can be used for minor crossing roads with low traffic volumes but these will require temporary road diversion during construction.

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Access under the railway bridge crossing is difficult for machine excavation. Excavation might have to be done using picks and shovels in this area.

There was no groundwater seepage encountered in any of the test pits excavated at the crossings and no test pit sidewall collapse was encountered.

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7 Material Quantities

The total length of the pipeline is 6.054 km and the diameter is 600mm according to the information provided by the Client. Average depth of the pipeline excavation is 2m below ground level. Pipe bedding, selected fill and general fill volume requirement calculations are based on SANS 1200LB.

Table 13: Excavation Classes

SECTION	AVERAGE TRENCH DEPTH (m)	SOFT EXCAVATION (VOLUME m ³) (percentage)
Pipeline Route	2.2	60,884

Battering of trench walls will vary between:

- 1V:1.5H in wet soil conditions
- 1V:1H in dry soil conditions.

60° battering of trench walls was assumed for the calculation of required bedding volumes along the entire pipe length.

The pipe bedding volume requirements are summarised in the Table below.

PIPELINE LENGTH (m)	BEDDING		SELECTED FILL		BACKFILL	
	PER METRE PIPE LENGTH (m ³ /m)	TOTAL (m ³)	PER METRE PIPE LENGTH (m ³ /m)	TOTAL (m ³)	PER METRE PIPE LENGTH (m ³ /m)	TOTAL (m ³)
6054	2.0	12,053	0.85	5,146	7.2	43,685

8 Materials Sources

As stated previously, the material encountered along the pipeline route are unsuitable for use as bedding and selected fill, hence these materials should be obtained from commercial sources. Such source was identified within 10km radius of the site and it is called Afrimat.

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Test results were requested to confirm suitability of the material for backfill and the material is indeed suitable. The test results from Afrimat are attached in Annexure E of this report.

9 Conclusions and Recommendations

The regional geology along the B19 Sludge Pipeline has been retrieved from the geology map Sheet 2626 West-Rand at scale of 1:250 000, it is therefore underlined by predominately mudrock that have been indurated into khaki brown blotch orange and calcareous white sandy clayey silts of firm soil consistency forming concealed geological contacts with sandy shales that are weakly cemented light brown speckled yellow firm clayey sandy silt soils, interpreted as Quaternary soils deposits Aeolian / Hillwash soils.

The material along the pipeline route is classified as soft and can be excavated manually using picks and shovels or mechanically using a tractor-loader-backhoe (TLB).

The material along the pipeline route is generally classified as G9 or worse due to high content of silt and clay hence it is unsuitable for use as pipe bedding and selected fill and can only be used as general backfill. Suitable material will have to be obtained from commercial sources.

Unstable trench or test pit side walls were encountered. It is thus recommended that the sidewalls of excavations be either battered back to 1 (vertical) to 1.5 (horizontal) where wet conditions exist. In dry conditions, 1 (vertical) to 1 (horizontal) slopes can be used. If vertical slopes are excavated, shoring be used to ensure stability of the sides of the excavations.

De-watering of the trenches during construction will be required in some sections along the pipeline route where groundwater was encountered.

pH and conductivity test results indicated that the soils are moderately corrosive hence measures against corrosion protection of steel elements should be provided. However, resistivity testing shall also be carried out to confirm the corrosive potential of the material.

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REPORT ANNEXURES

Annexure A - Google Earth Test Pit Positions

Annexure B - Test Pits Profiles

Annexure C - Test Pits Photographs

Annexure D - Laboratory Test Results

Annexure E - Material Sources Test Results

Annexure F - Longitudinal Sections

Annexure G - Soil Resistivity Survey Report

RESPONSE TO THE SCOPE OF WORK

- Conduct field investigations for ± 8 km pipeline for 600 mm diameter steel pipeline by means of exploration trial pits excavated by an excavator. The depth of trial pits shall be 4m and at every 250m along the proposed pipeline. *The test pits have been excavated at 250m spacing, to 3.0m depth which is the maximum reach of a TLB. The depth is considered adequate since the pipe will be placed at an average depth of 2m depth. 4m deep test pits would require use of an excavator which would increase the cost significantly because of higher hourly rate as well as slower travelling speed compared to a TLB.*
- Geotechnical survey to be conducted on either side of all the pipe jacking crossings required. *Discussed in section 6 (Crossings) of the report.*
- Provide the detailed description and recommendation on the stratigraphy at all anticipated pipe jack crossings. *Discussed in section 6 (Crossings) of the report.*
- Study the usability of trench materials as bedding and selected backfill. *Discussed in section 5.4 (Material Usability) of the report.*
- Identify the alternative potential sources of construction materials, either in areas adjacent to the pipeline or commercial sources. *Discussed in section 8 (Material Sources) of the report.*
- Conduct soil Resistivity surveys for the entire route of the pipeline. *Included as Annexure G in the report.*
- Provide recommendations on areas where the pipeline route pass through the soils with high collapsible grain structure, expansive and sink hole characteristics. *No soils with high collapsible grain were encountered. Sinkholes are encountered on dolomitic ground and the pipeline route is not situated on dolomites. There are areas where potentially expansive clay is encountered and these would require replacing the expansive material with inert granular material. See section 5.1 (Soil Classification and behavior) of the report.*

- *Demarcate all areas of high water table, and provide recommendations and procedures for dewatering. These areas have been identified and indicated in section 4.9 (Groundwater seepage) of the report and also shown on the long section drawing.*

- *Carry out laboratory tests on the Grading (sieve analysis and Atterberg limits), Chemical Analysis (soil and ground Water including Acid Mine Drainage (AMD)) , Moisture sensitivity analysis(Mod AASHTO), Oedometer test (stress deformation analysis) and compatibility factor tests etc. Sieve analysis including Atterberg Limits have been conducted. These indicated that the material along the pipeline route was unsuitable to use as bedding and selected fill and should therefore be excavated and replaced with suitable material. As a results, there was no need to do further testing on these materials as it would be a fruitless exercise. See section 4.4 (Laboratory Testing) of the report.*

- *Provide excavation and backfill quantities (soft and rock) and quantities for soiled and imported materials as per SANS 1200 DB and SANS 1200 LB. Discussed in section 7 (Material Quantities) of the report.*

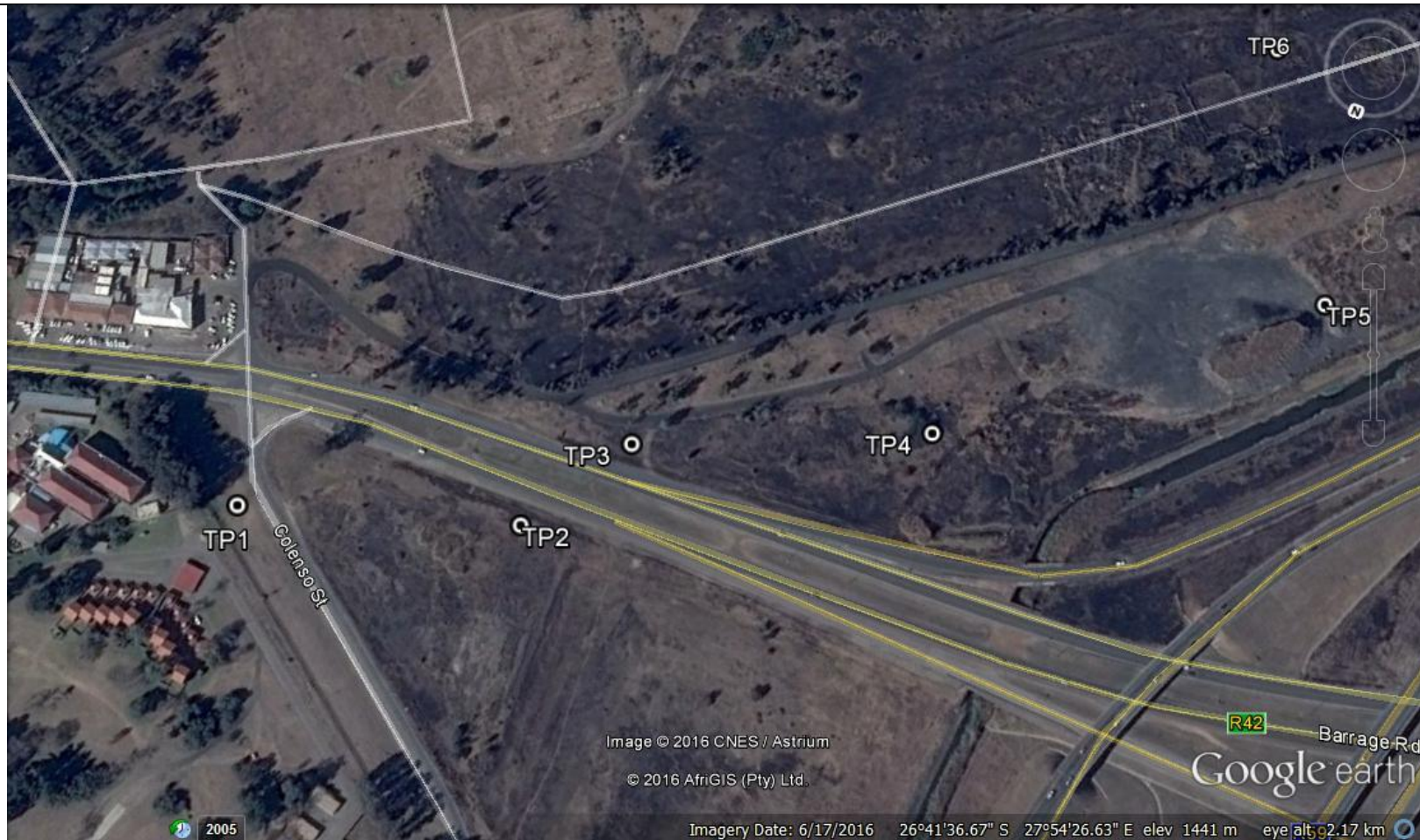
- *Provide soil profile on the final long section that will be given to by Rand Water (RW) in DWG format. Included as Annexure F in the report.*

- *Provide trench excavations and backfilling details specific for the project. Provided in section 5.3 (Trench stability) and section 5.4 (Material Usability) of the report.*

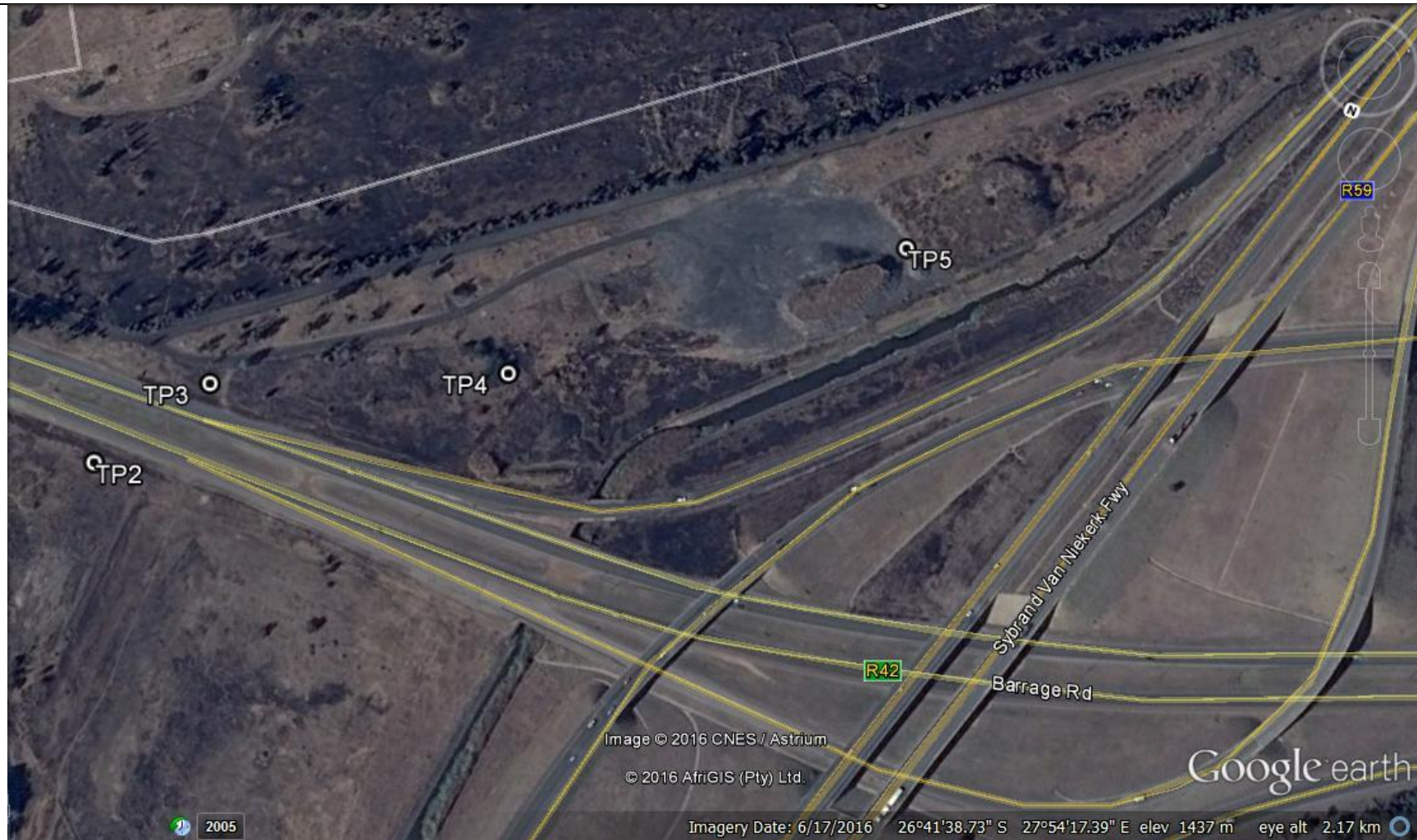
ANNEXURE A

GOOGLE EARTH TEST PITS POSTIONS

TP1 – TP6



TP2 – TP5



TP5 - TP7



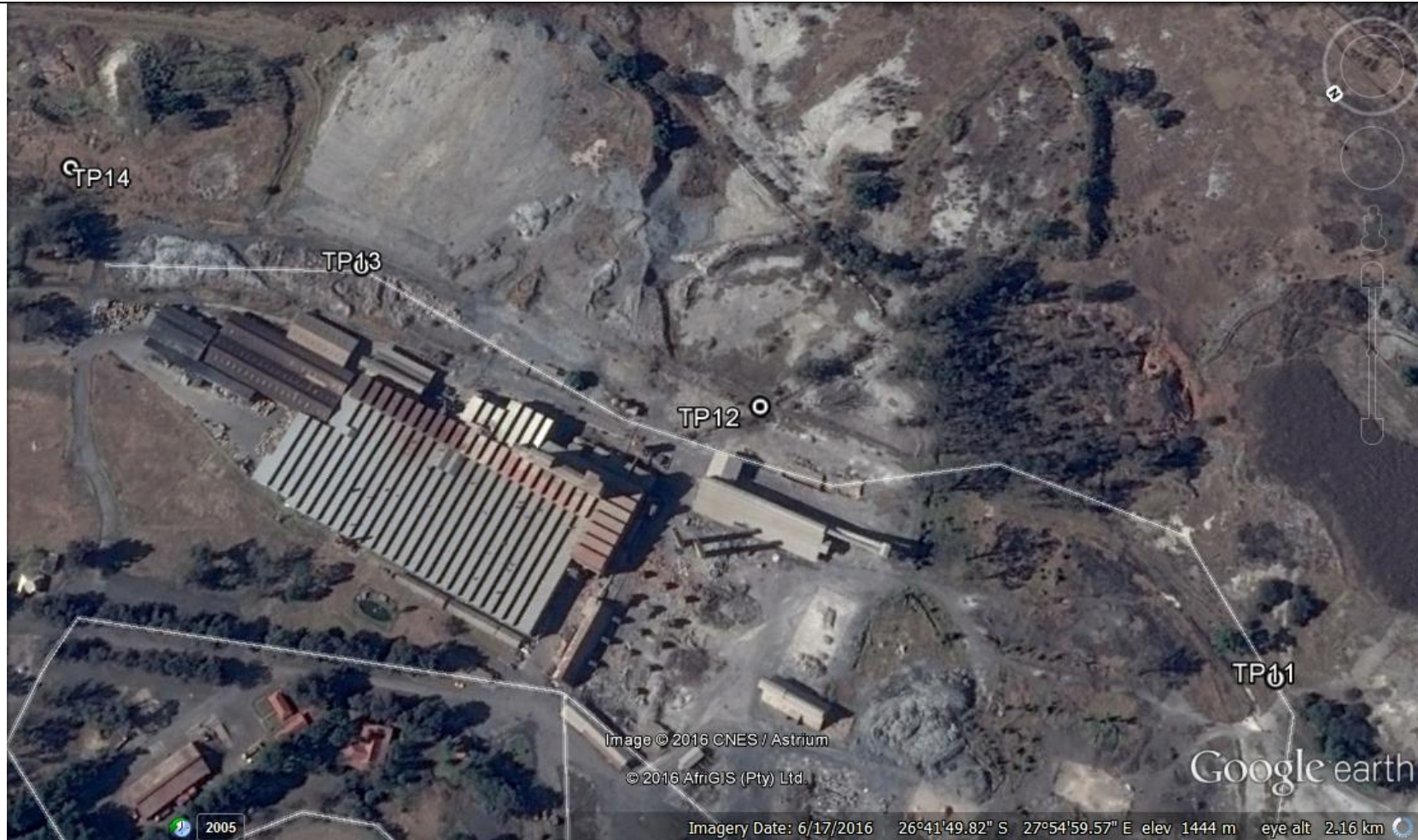
TP7 – TP9



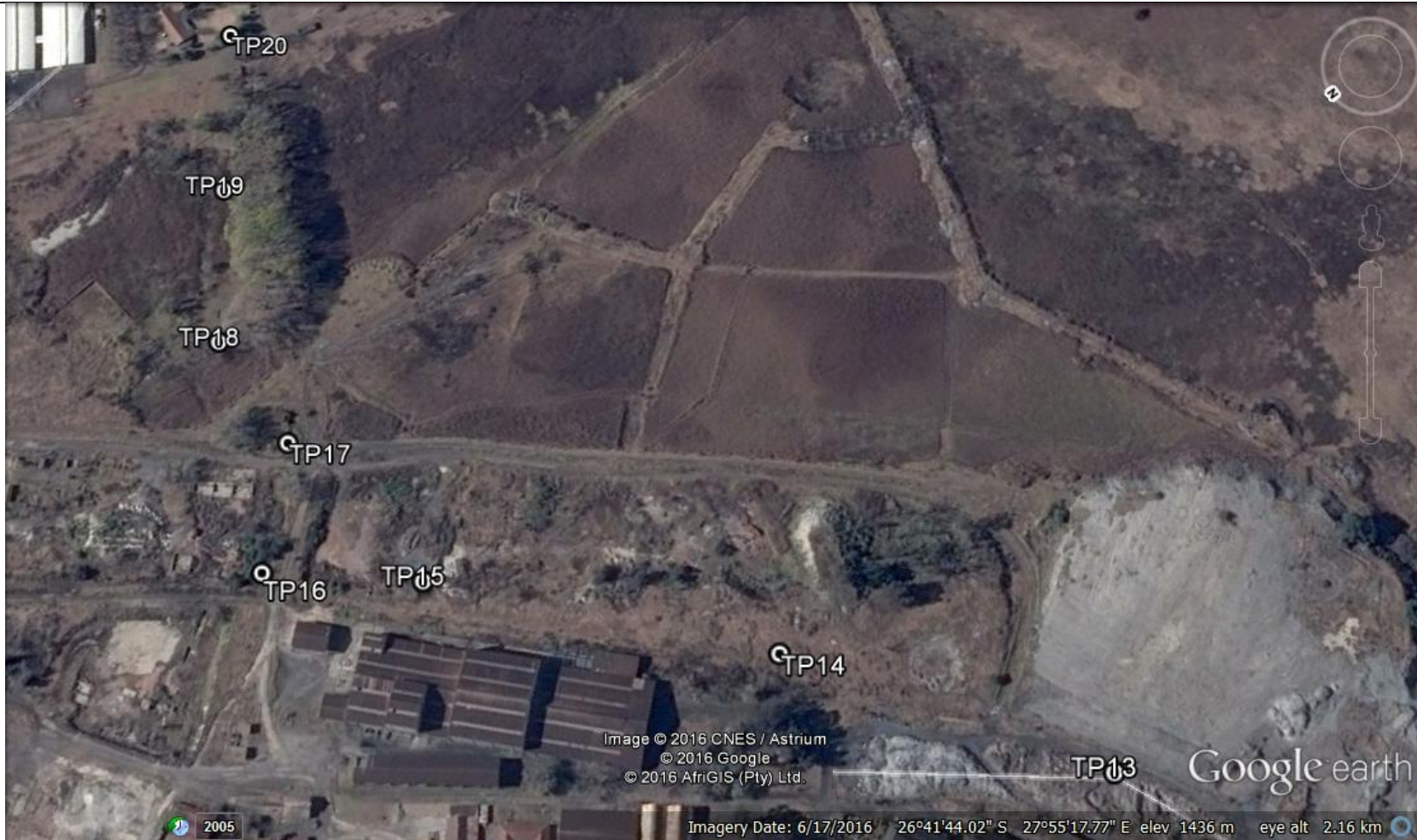
TP9 – TP11



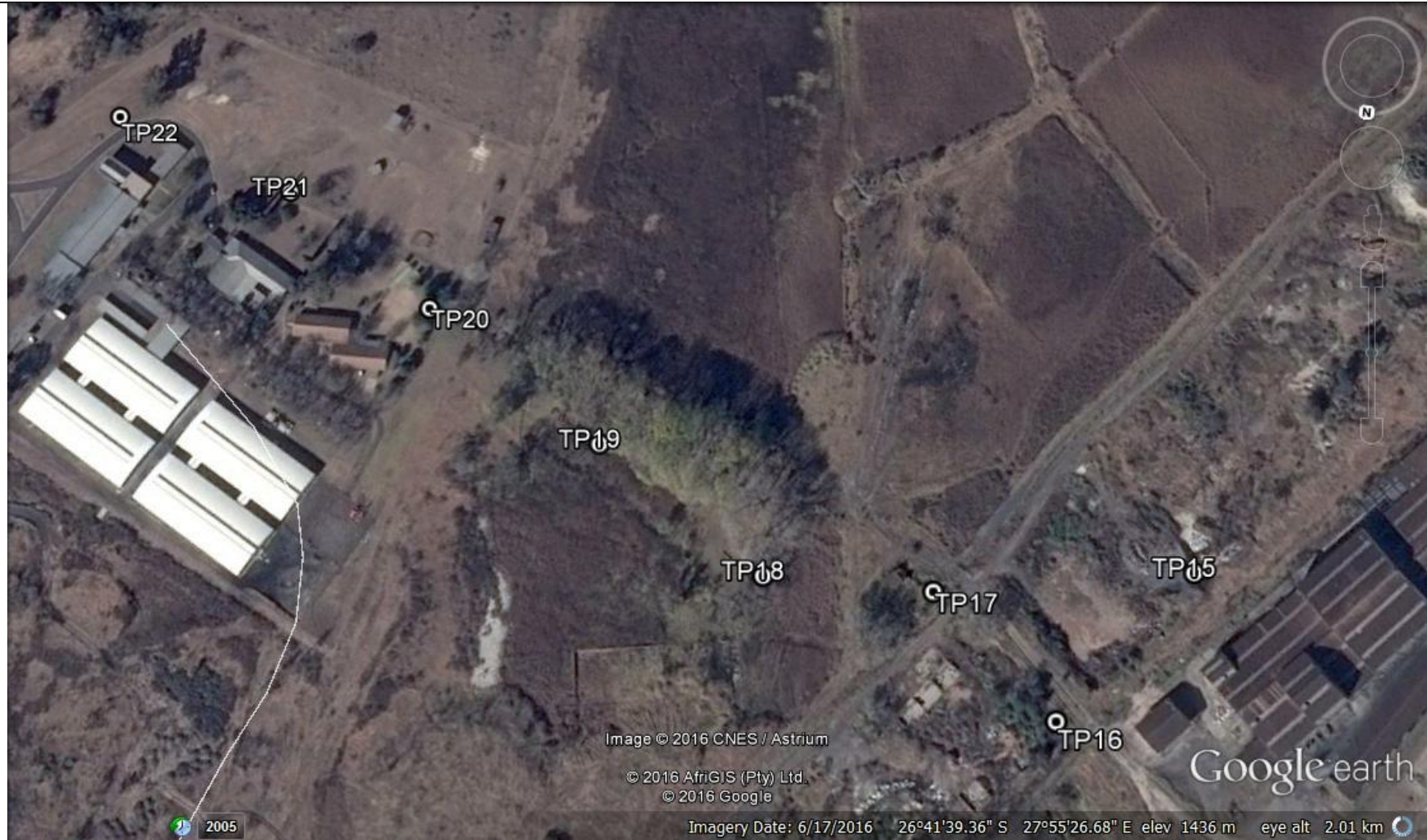
TP11 – TP14



TP13 – TP20



TP15 – TP22



TP20 – TP26



TP24 – TP30



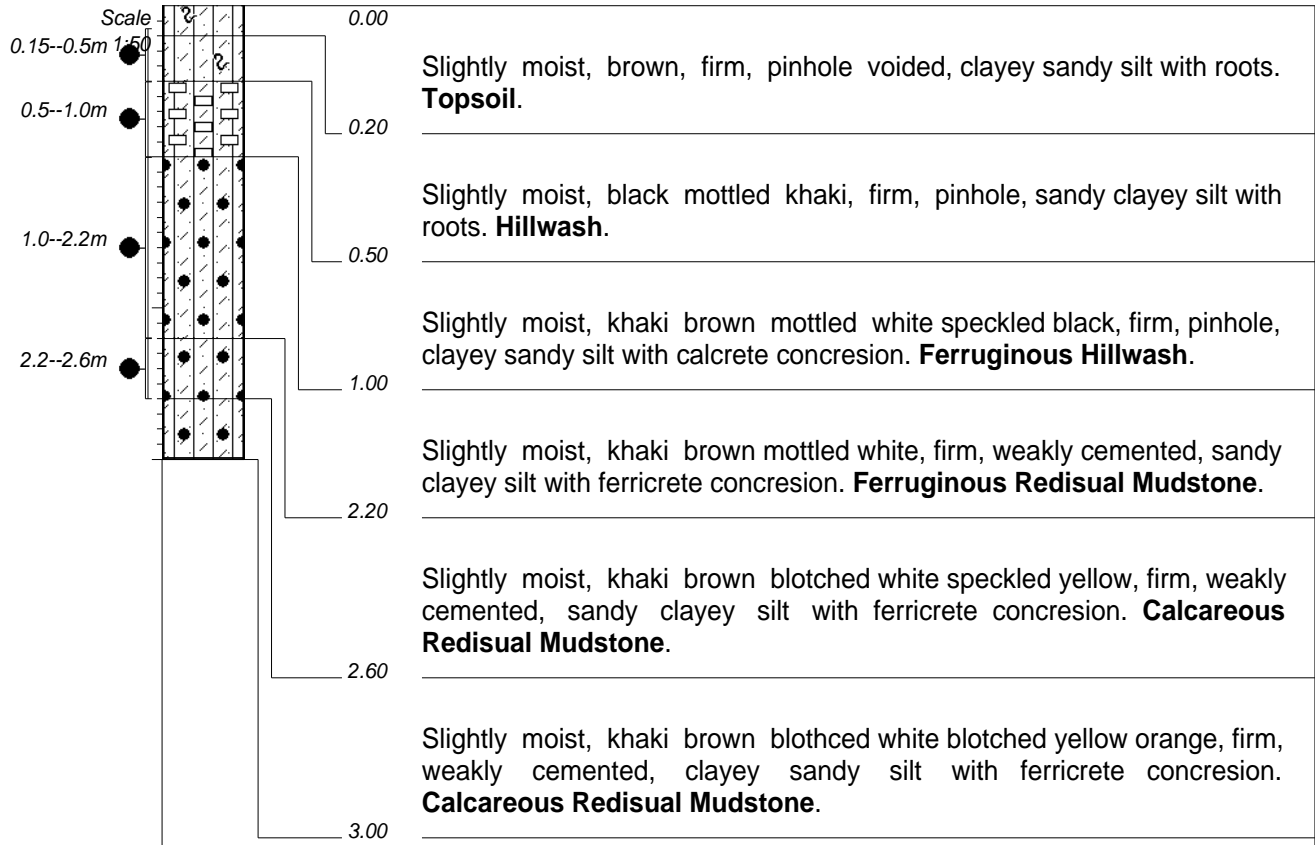
TP26 – TP32



TP31 – TP35



ANNEXURE B
TESTS PITS PROFILES



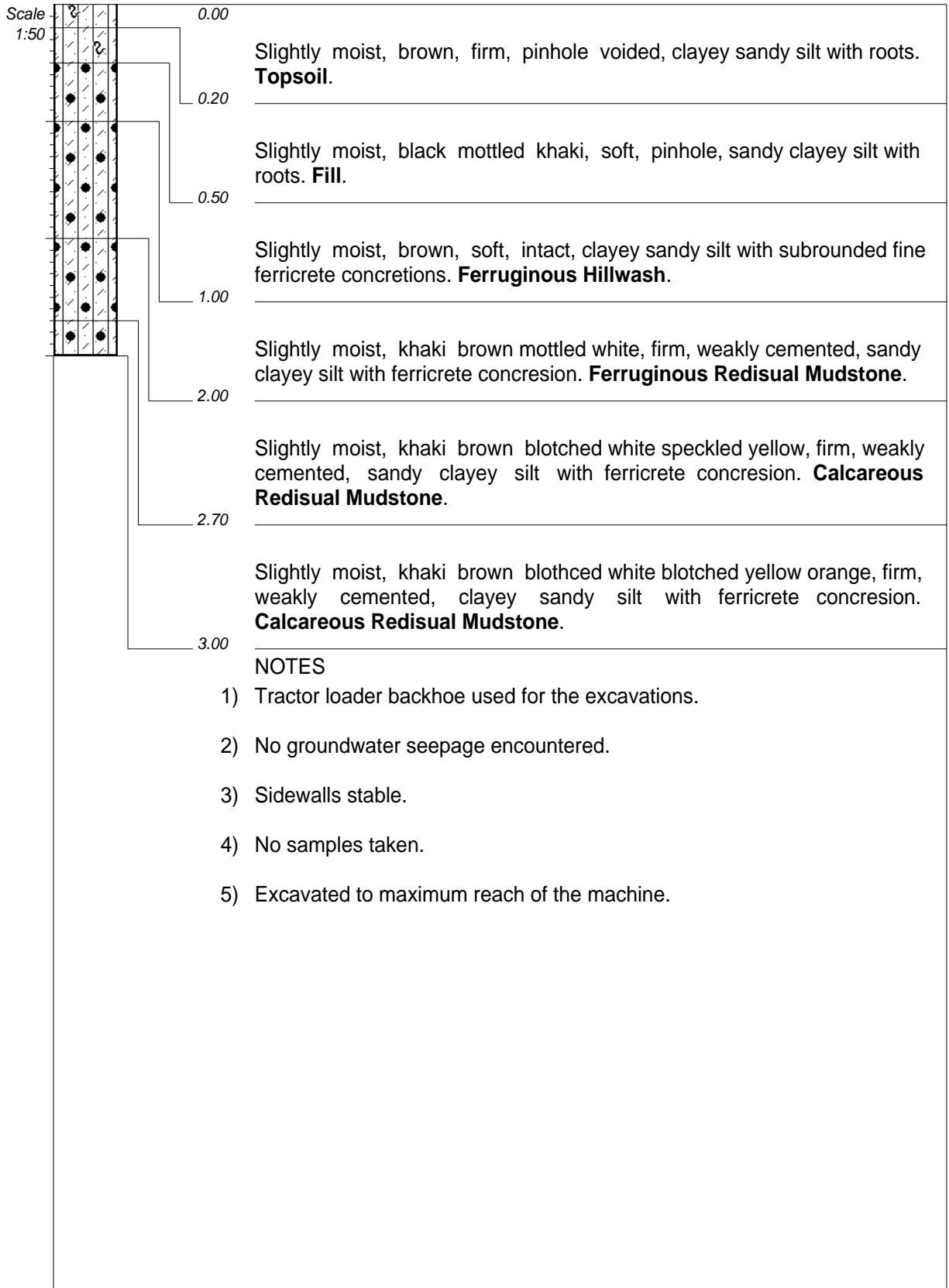
NOTES

- 1) Tractor loader backhoe used for the excavations.
- 2) No groundwater seepage encountered.
- 3) Sidewalls stable.
- 4) Disturbed and bulk samples taken at 0.15--0.5m & 0.5--1.0m & 1.0--2.2m & 2.2--2.6m.
- 5) Excavated to maximum reach of the machine.

CONTRACTOR :
 MACHINE : TLB
 DRILLED BY :
 PROFILED BY : G. Rabodiba Pr.Sci.Nat
 TYPE SET BY :
 SETUP FILE : STANDARD.SET

INCLINATION :
 DIAM : Trench
 DATE :
 DATE : 2016/07/29
 DATE : 27/09/2016 19:57
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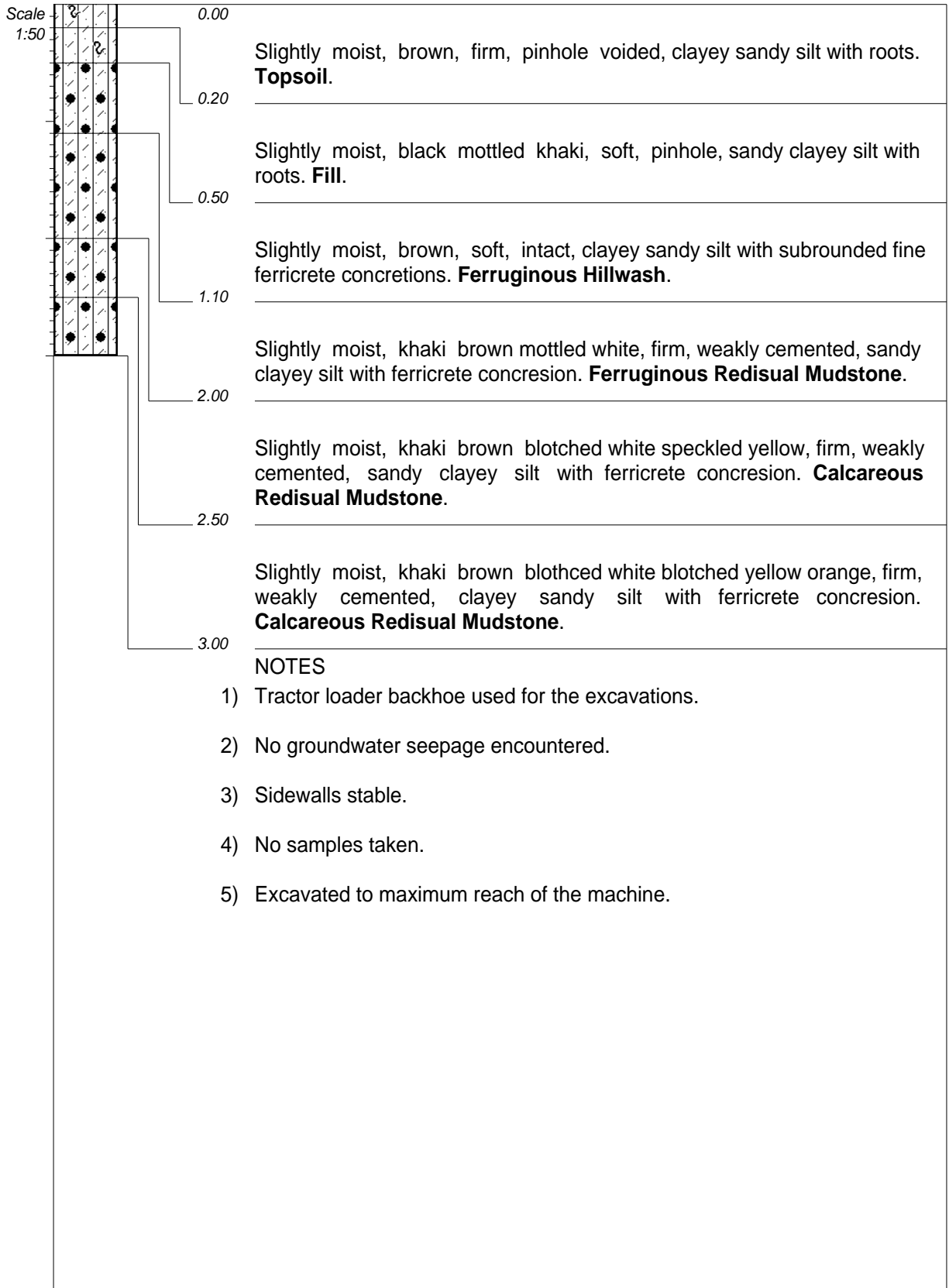
ELEVATION :
 X-COORD :
 Y-COORD :



CONTRACTOR :
 MACHINE : TLB
 DRILLED BY :
 PROFILED BY : G. Rabodiba Pr.Sci.Nat
 TYPE SET BY :
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 DATE :
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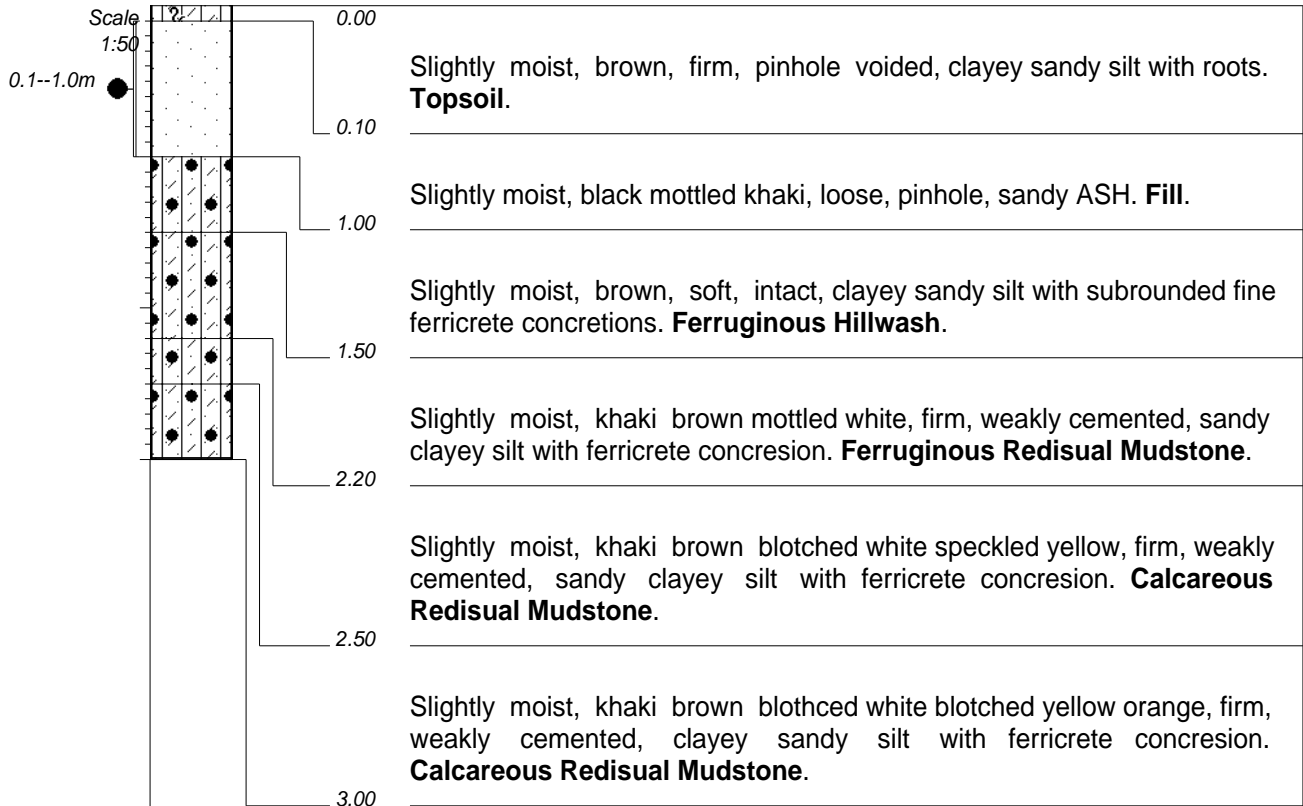
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 X-COORD :
 Y-COORD :



CONTRACTOR :
 MACHINE : TLB
 DRILLED BY :
 PROFILED BY : G. Rabodiba Pr.Sci.Nat
 TYPE SET BY :
 SETUP FILE : STANDARD.SET

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 Y-COORD :



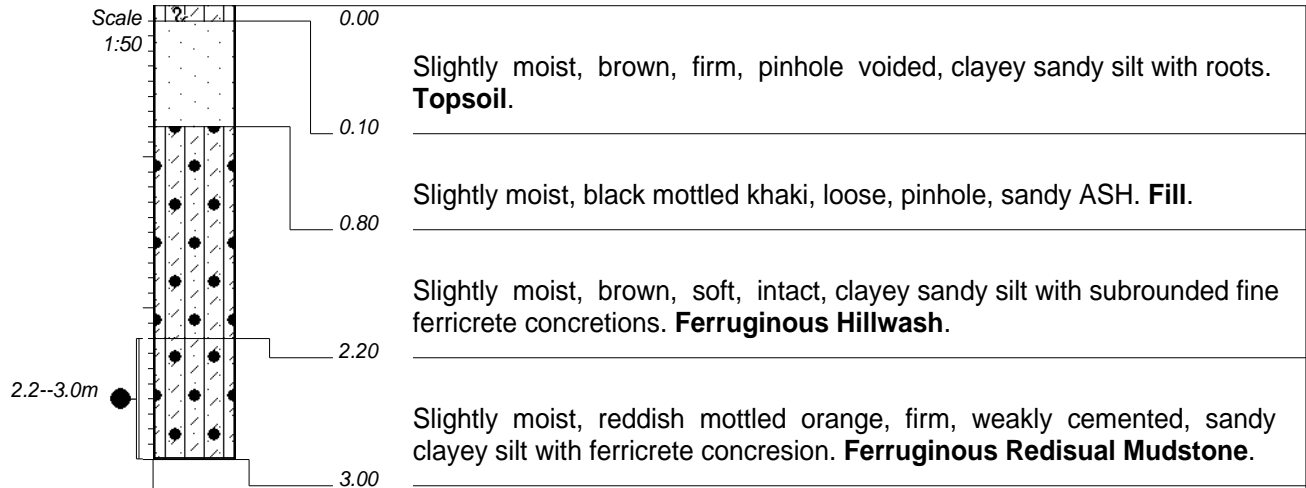
NOTES

- 1) Tractor loader backhoe used for the excavations.
- 2) No groundwater seepage encountered.
- 3) Sidewalls stable.
- 4) Disturbed soils samples taken at 0.1--1.0m.
- 5) Excavated to maximum reach of the machine.

CONTRACTOR :
 MACHINE : TLB
 DRILLED BY :
 PROFILED BY : G. Rabodiba Pr.Sci.Nat
 TYPE SET BY :
 SETUP FILE : STANDARD.SET

INCLINATION :
 DIAM : Trench
 DATE :
 DATE : 2016/07/29
 DATE : 27/09/2016 19:57
 TEXT : ..TPITS\RandWaterTP110.txt

ELEVATION :
 X-COORD :
 Y-COORD :



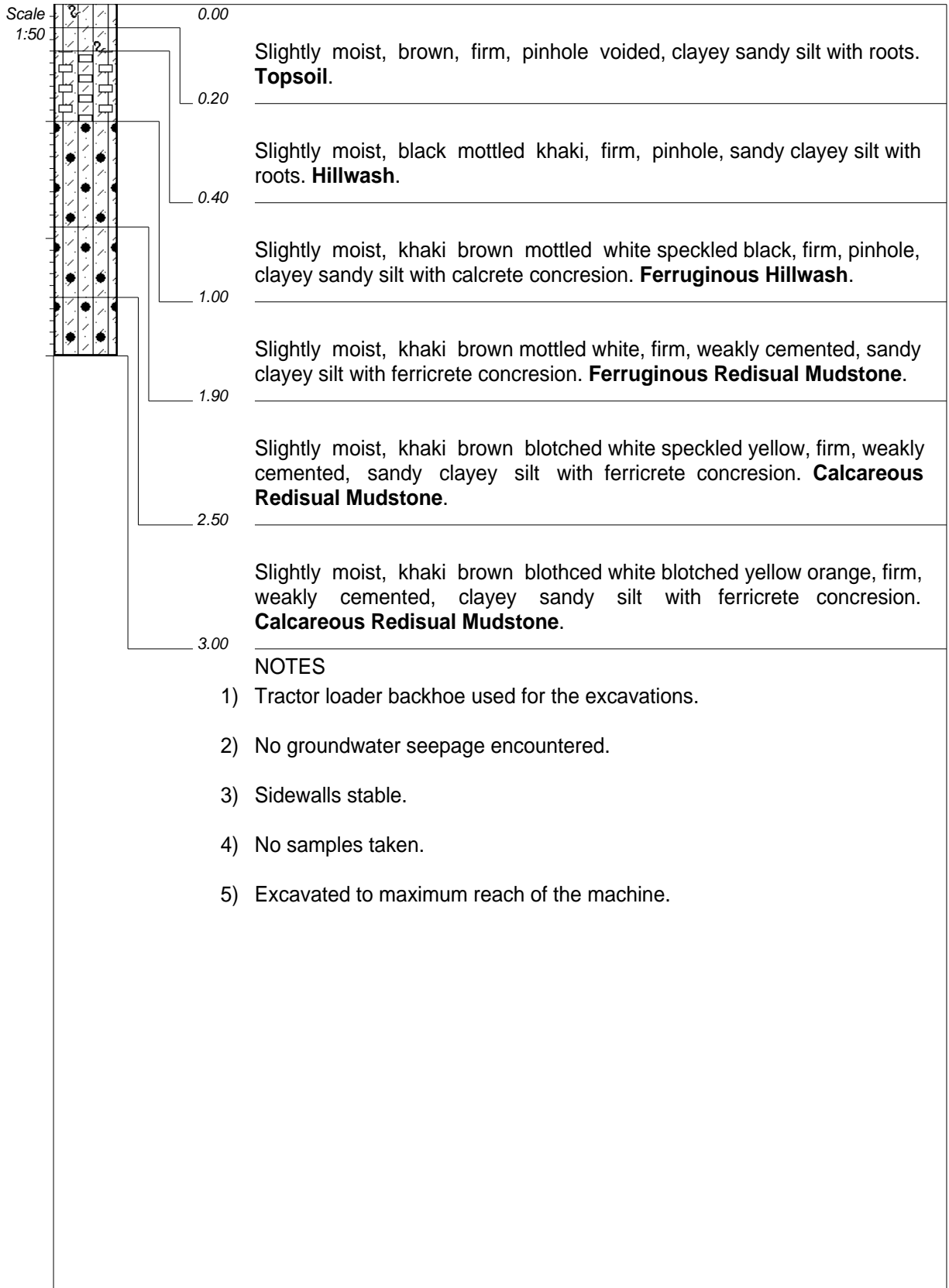
NOTES

- 1) Tractor loader backhoe used for the excavations.
- 2) No groundwater seepage encountered.
- 3) Sidewalls stable.
- 4) Disturbed soils samples taken at 2.2--3.0m.
- 5) Excavated to maximum reach of the machine.

CONTRACTOR :
 MACHINE : TLB
 DRILLED BY :
 PROFILED BY : G. Rabodiba Pr.Sci.Nat
 TYPE SET BY :
 SETUP FILE : STANDARD.SET

INCLINATION :
 DIAM : Trench
 DATE :
 DATE : 2016/07/29
 DATE : 27/09/2016 19:57
 TEXT : ..TPITS\RandWaterTP110.txt

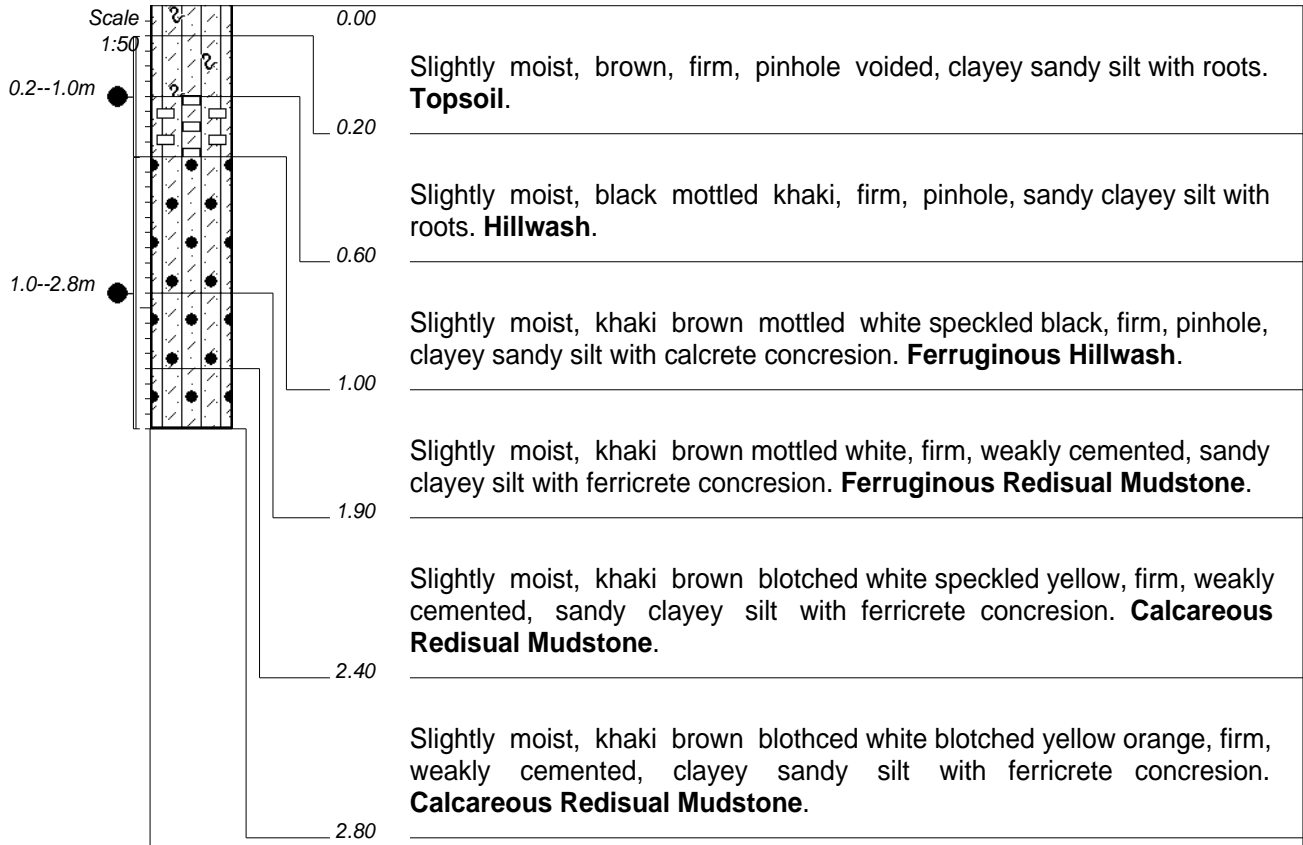
ELEVATION :
 X-COORD :
 Y-COORD :



CONTRACTOR :
 MACHINE : TLB
 DRILLED BY :
 PROFILED BY : G. Rabodiba Pr.Sci.Nat
 TYPE SET BY :
 SETUP FILE : STANDARD.SET

INCLINATION :
 DIAM : Trench
 DATE :
 DATE : 2016/07/29
 DATE : 27/09/2016 19:57
 TEXT : ..TPITS\RandWaterTP110.txt

ELEVATION :
 X-COORD :
 Y-COORD :



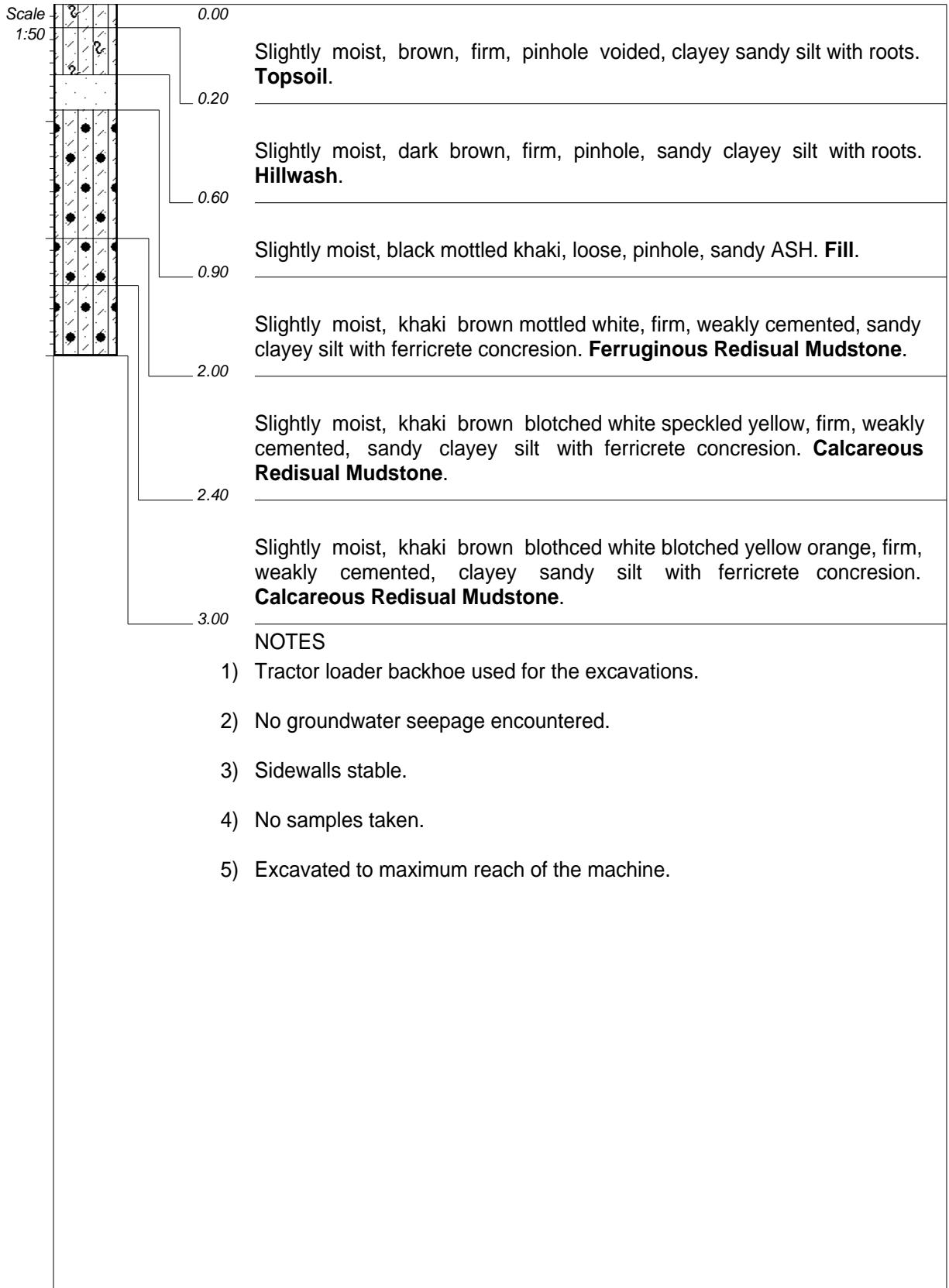
NOTES

- 1) Tractor loader backhoe used for the excavations.
- 2) No groundwater seepage encountered.
- 3) Sidewalls stable.
- 4) Disturbed samples taken at 0.2--1.0m & 1.0--2.8m.
- 5) Excavated to maximum reach of the machine.

CONTRACTOR :
 MACHINE : TLB
 DRILLED BY :
 PROFILED BY : G. Rabodiba Pr.Sci.Nat
 TYPE SET BY :
 SETUP FILE : STANDARD.SET

INCLINATION :
 DIAM : Trench
 DATE :
 DATE : 2016/07/29
 DATE : 27/09/2016 19:57
 TEXT : ..TPITS\RandWaterTP110.txt

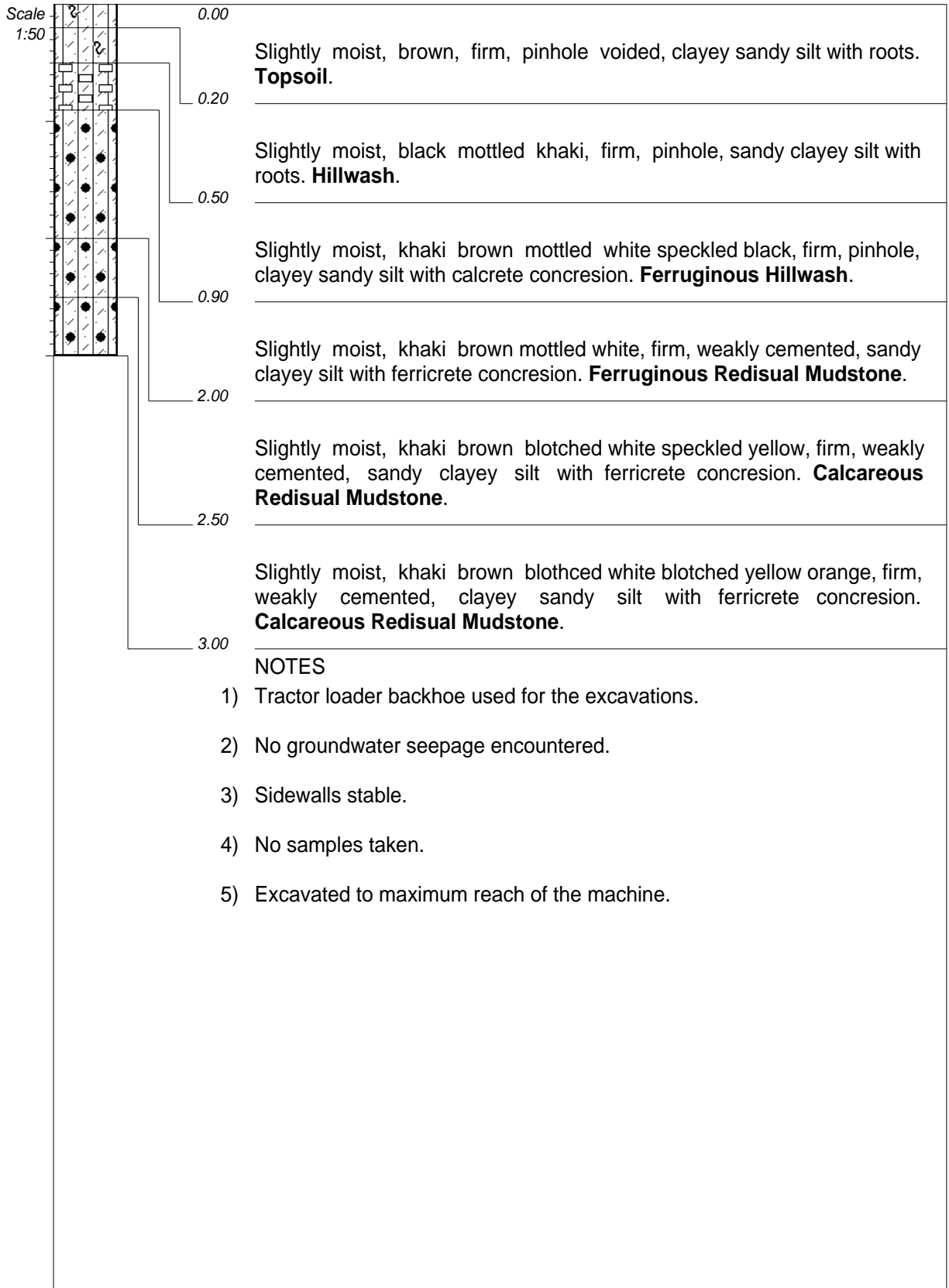
ELEVATION :
 X-COORD :
 Y-COORD :



CONTRACTOR :
 MACHINE : TLB
 DRILLED BY :
 PROFILED BY : G. Rabodiba Pr.Sci.Nat
 TYPE SET BY :
 SETUP FILE : STANDARD.SET

INCLINATION :
 DIAM : Trench
 DATE :
 DATE : 2016/07/29
 DATE : 27/09/2016 19:57
 TEXT : ..TPITS\RandWaterTP110.txt

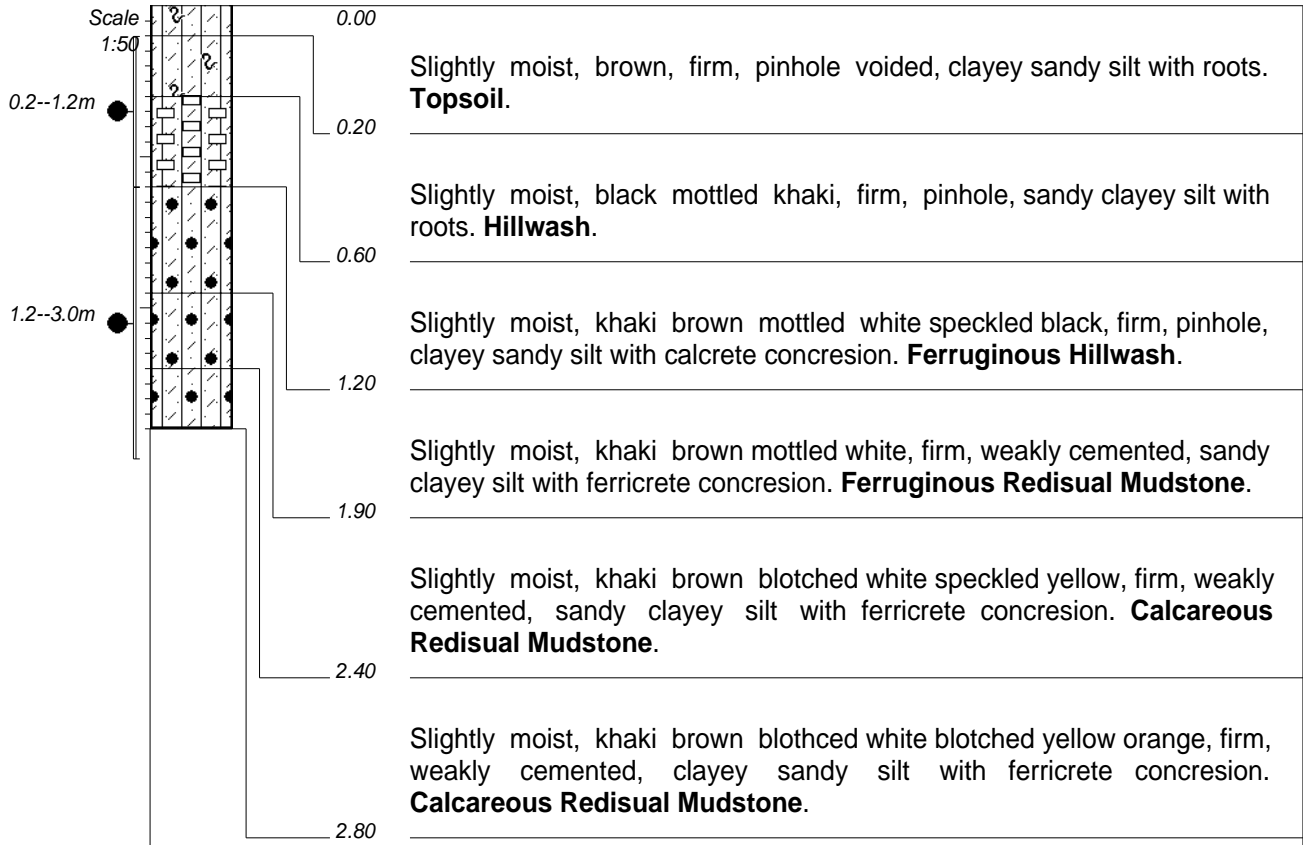
ELEVATION :
 X-COORD :
 Y-COORD :



CONTRACTOR :
 MACHINE : TLB
 DRILLED BY :
 PROFILED BY : G. Rabodiba Pr.Sci.Nat
 TYPE SET BY :
 SETUP FILE : STANDARD.SET

INCLINATION :
 DIAM : Trench
 DATE :
 DATE : 2016/07/29
 DATE : 27/09/2016 19:57
 TEXT : ..TPITS\RandWaterTP110.txt

ELEVATION :
 X-COORD :
 Y-COORD :



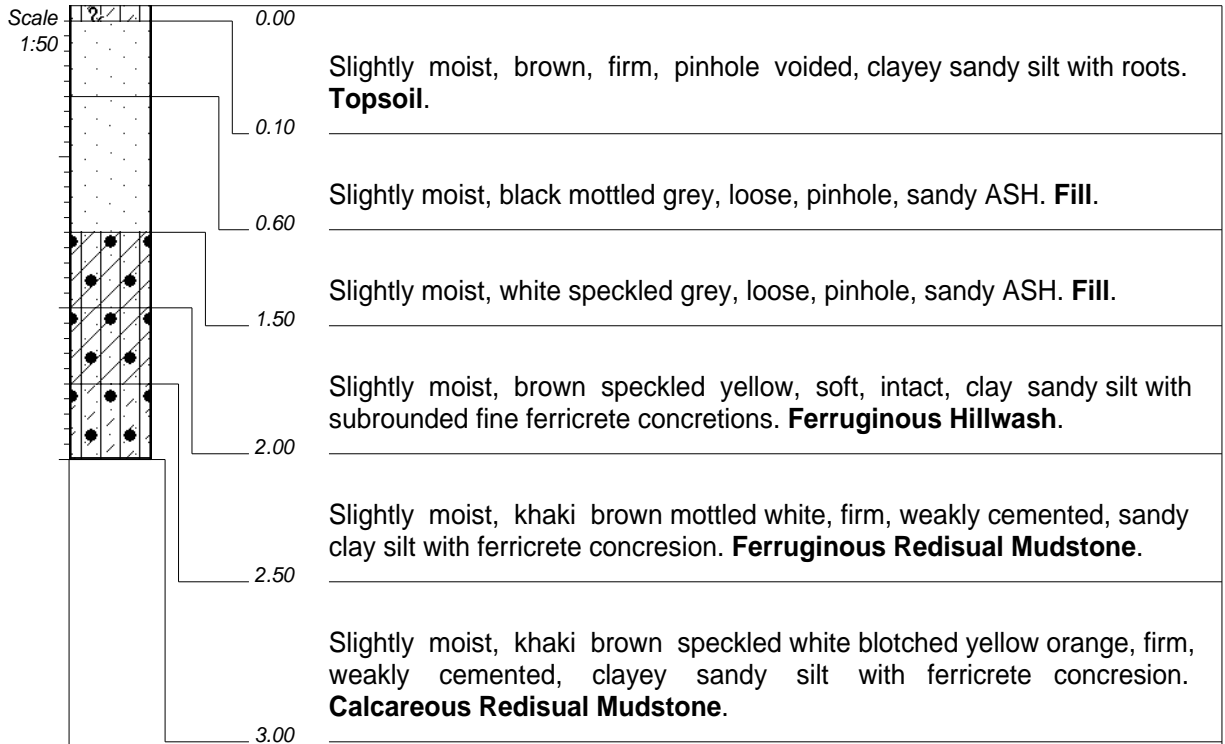
NOTES

- 1) Tractor loader backhoe used for the excavations.
- 2) No groundwater seepage encountered.
- 3) Sidewalls stable.
- 4) Disturbed and bulk samples taken at 0.2--1.2m & 1.2--3.0m.
- 5) Excavated to maximum reach of the machine.

CONTRACTOR :
 MACHINE : TLB
 DRILLED BY :
 PROFILED BY : G. Rabodiba Pr.Sci.Nat
 TYPE SET BY :
 SETUP FILE : STANDARD.SET

INCLINATION :
 DIAM : Trench
 DATE :
 DATE : 2016/07/29
 DATE : 27/09/2016 19:57
 TEXT : ..TPITS\RandWaterTP110.txt

ELEVATION :
 X-COORD :
 Y-COORD :



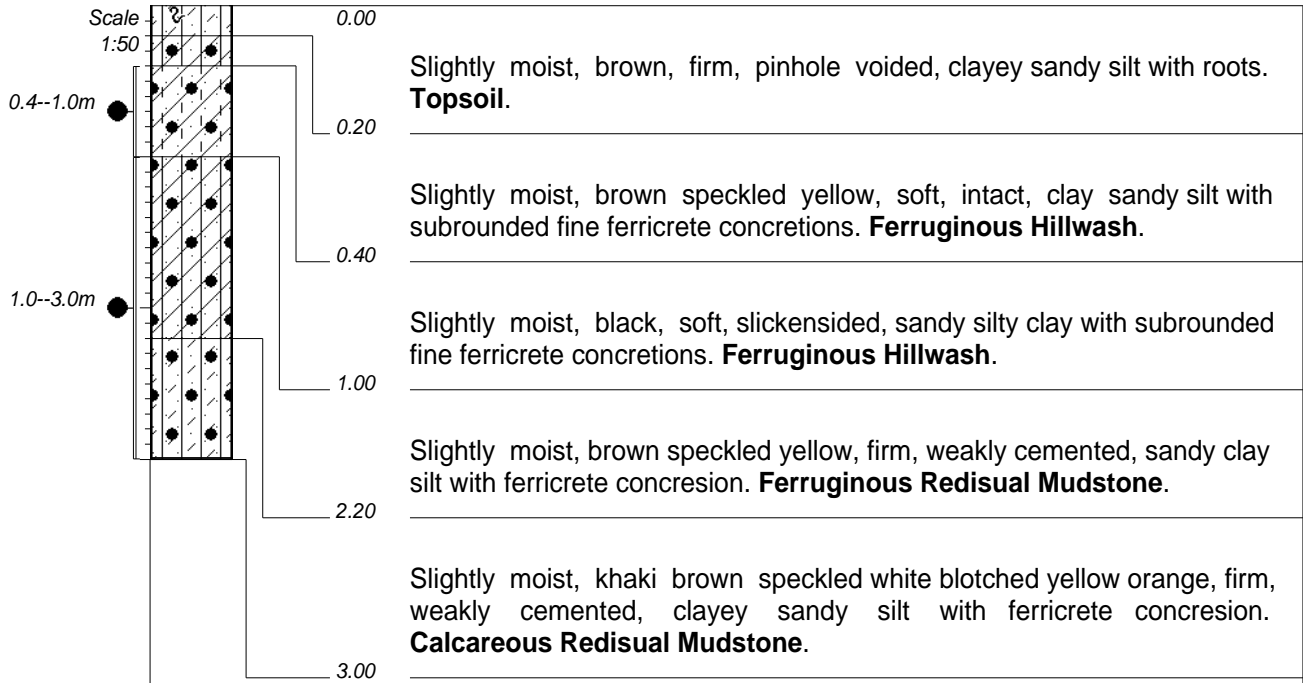
NOTES

- 1) Tractor loader backhoe used for the excavations.
- 2) No groundwater seepage encountered.
- 3) Sidewalls stable.
- 4) No soils samples taken.
- 5) Excavated to maximum reach of the machine.

CONTRACTOR :
 MACHINE : TLB
 DRILLED BY :
 PROFILED BY : G. Rabodiba Pr.Sci.Nat
 TYPE SET BY :
 SETUP FILE : STANDARD.SET

INCLINATION :
 DIAM : Trench
 DATE :
 DATE : 2016/07/29
 DATE : 27/09/2016 19:58
 TEXT : ..PITS\RandWaterTP1120.txt

ELEVATION :
 X-COORD :
 Y-COORD :



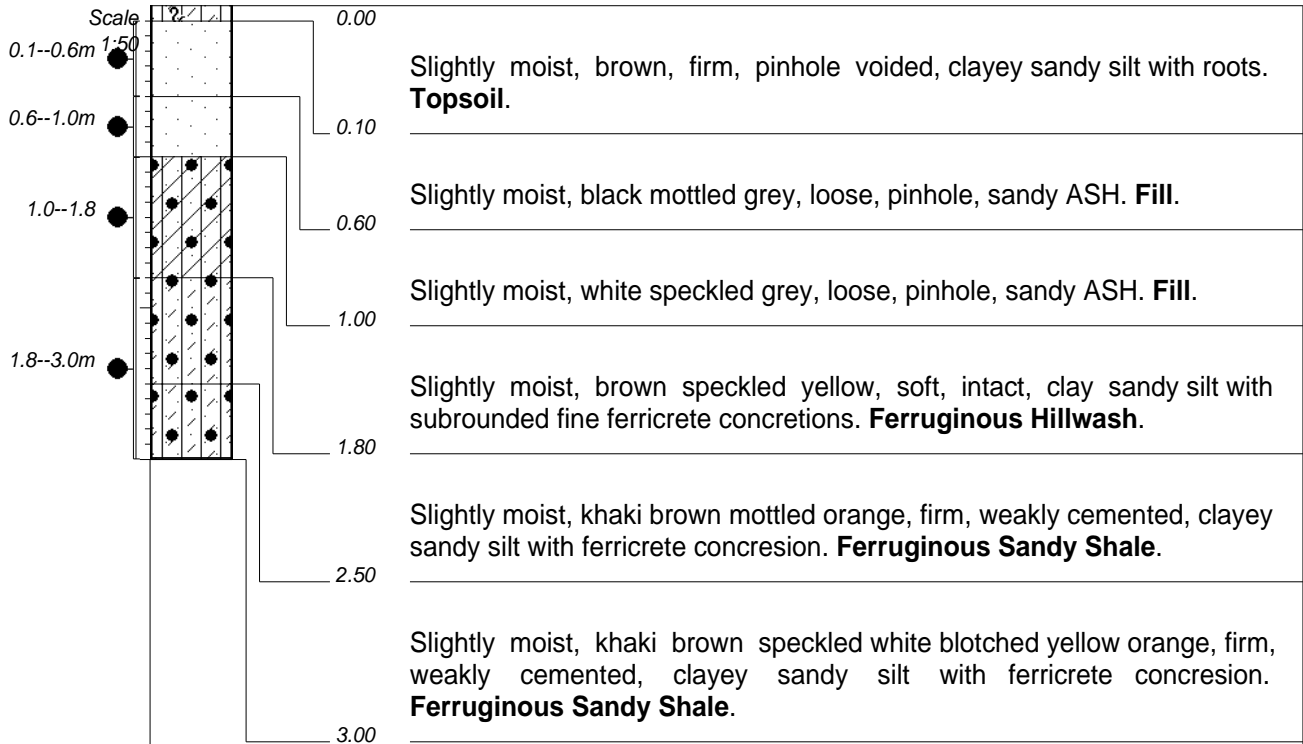
NOTES

- 1) Tractor loader backhoe used for the excavations.
- 2) No groundwater seepage encountered.
- 3) Sidewalls stable.
- 4) Disturbed soils samples taken at 0.4--1.0m & 1.0--3.0m.
- 5) Excavated to maximum reach of the machine.

CONTRACTOR :
 MACHINE : TLB
 DRILLED BY :
 PROFILED BY : G. Rabodiba Pr.Sci.Nat
 TYPE SET BY :
 SETUP FILE : STANDARD.SET

INCLINATION :
 DIAM : Trench
 DATE :
 DATE : 2016/07/29
 DATE : 27/09/2016 19:58
 TEXT : ..PITS\RandWaterTP1120.txt

ELEVATION :
 X-COORD :
 Y-COORD :



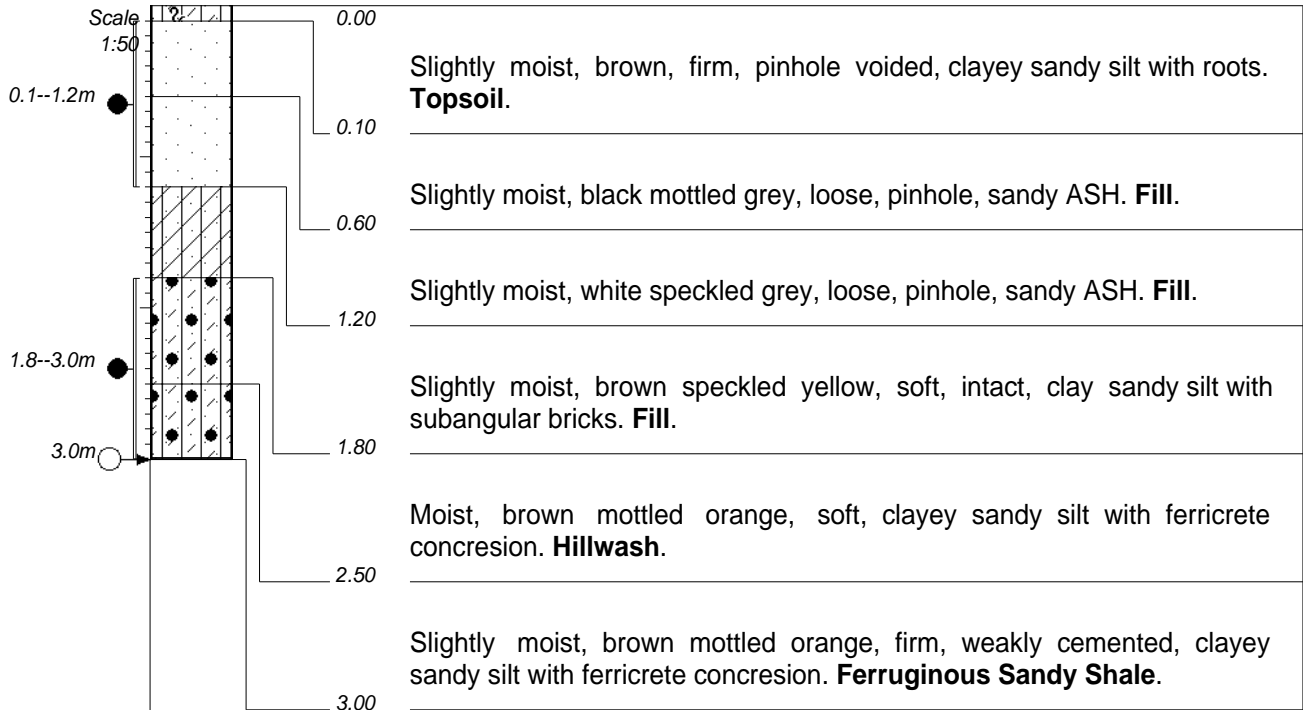
NOTES

- 1) Tractor loader backhoe used for the excavations.
- 2) No groundwater seepage encountered.
- 3) Sidewalls stable.
- 4) Bulk and disturbed soils samples taken 0.1--0.6m & 0.6--1.0m & 1.0--1.8 & 1.8--3.0m.
- 5) Excavated to maximum reach of the machine.

CONTRACTOR :
 MACHINE : TLB
 DRILLED BY :
 PROFILED BY : G. Rabodiba Pr.Sci.Nat
 TYPE SET BY :
 SETUP FILE : STANDARD.SET

INCLINATION :
 DIAM : Trench
 DATE :
 DATE : 2016/08/07
 DATE : 27/09/2016 19:58
 TEXT : ..PITS\RandWaterTP1120.txt

ELEVATION :
 X-COORD :
 Y-COORD :



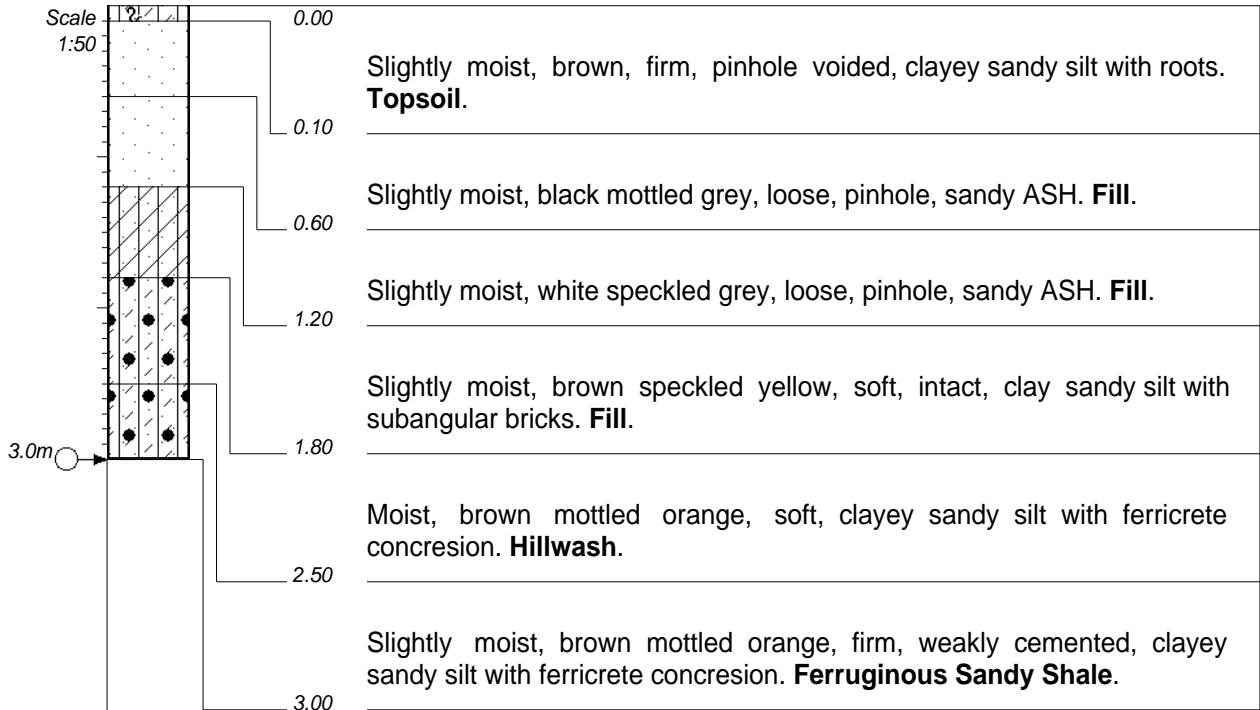
NOTES

- 1) Tractor loader backhoe used for the excavations.
- 2) Groundwater seepage encountered at 3.0m.
- 3) Sidewalls unstable at 1.2-1.8m.
- 4) Disturbed soils samples taken 0.1--1.2m & 1.8--3.0m.
- 5) Excavated to maximum reach of the machine.

CONTRACTOR :
 MACHINE : TLB
 DRILLED BY :
 PROFILED BY : G. Rabodiba Pr.Sci.Nat
 TYPE SET BY :
 SETUP FILE : STANDARD.SET

INCLINATION :
 DIAM : Trench
 DATE :
 DATE : 2016/08/07
 DATE : 27/09/2016 19:58
 TEXT : ..PITS\RandWaterTP1120.txt

ELEVATION :
 X-COORD :
 Y-COORD :



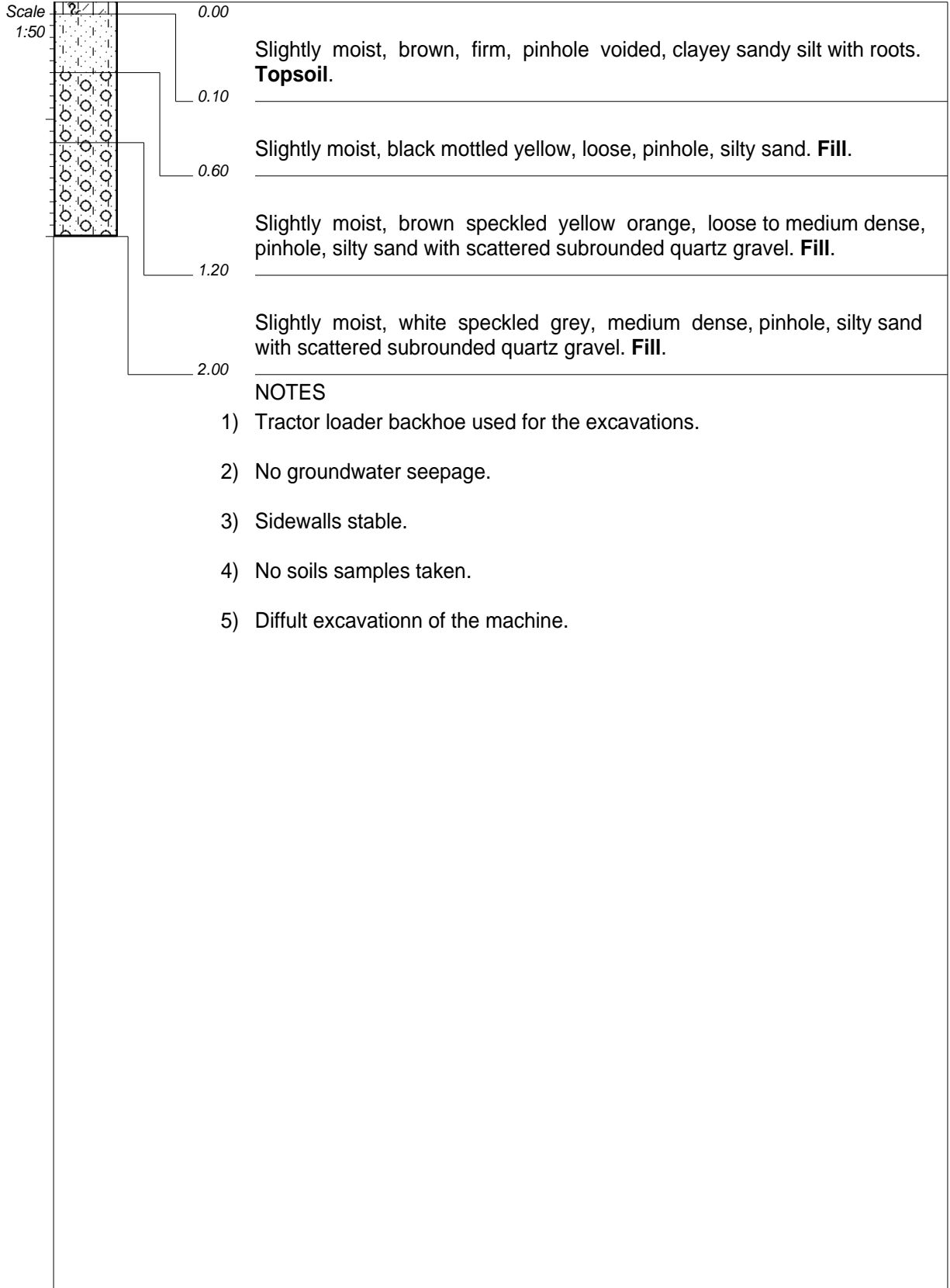
NOTES

- 1) Tractor loader backhoe used for the excavations.
- 2) Groundwater seepage encountered at 3.0m.
- 3) Sidewalls unstable at 1.2-1.8m.
- 4) No soils samples taken.
- 5) Excavated to maximum reach of the machine.

CONTRACTOR :
 MACHINE : TLB
 DRILLED BY :
 PROFILED BY : G. Rabodiba Pr.Sci.Nat
 TYPE SET BY :
 SETUP FILE : STANDARD.SET

INCLINATION :
 DIAM : Trench
 DATE :
 DATE : 2016/08/07
 DATE : 27/09/2016 19:58
 TEXT : ..PITS\RandWaterTP1120.txt

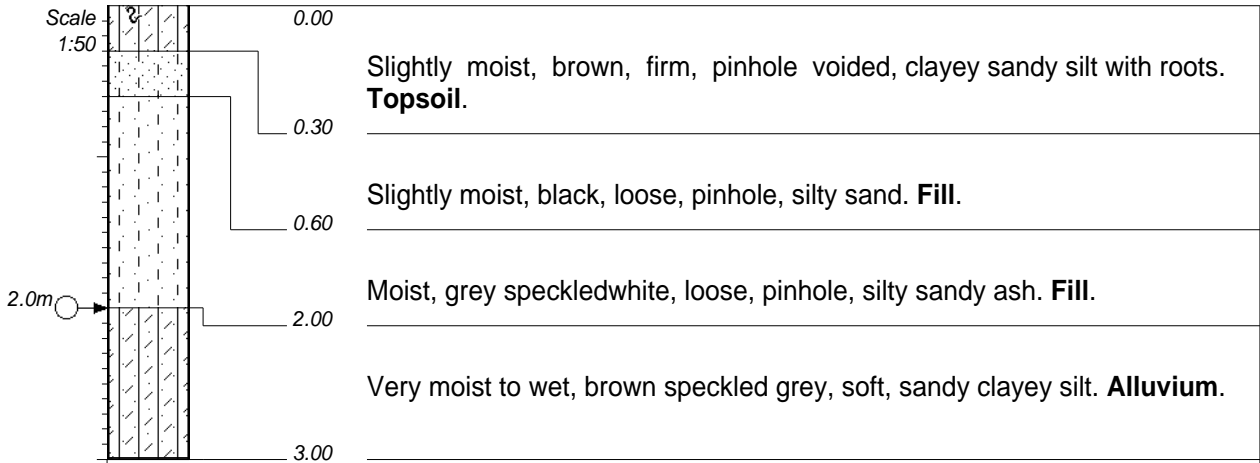
ELEVATION :
 X-COORD :
 Y-COORD :



CONTRACTOR :
MACHINE : TLB
DRILLED BY :
PROFILED BY : G. Rabodiba Pr.Sci.Nat
TYPE SET BY :
SETUP FILE : STANDARD.SET

INCLINATION :
DIAM : Trench
DATE :
DATE : 2016/08/07
DATE : 27/09/2016 19:58
TEXT : ..PITS\RandWaterTP1120.txt

ELEVATION :
X-COORD :
Y-COORD :



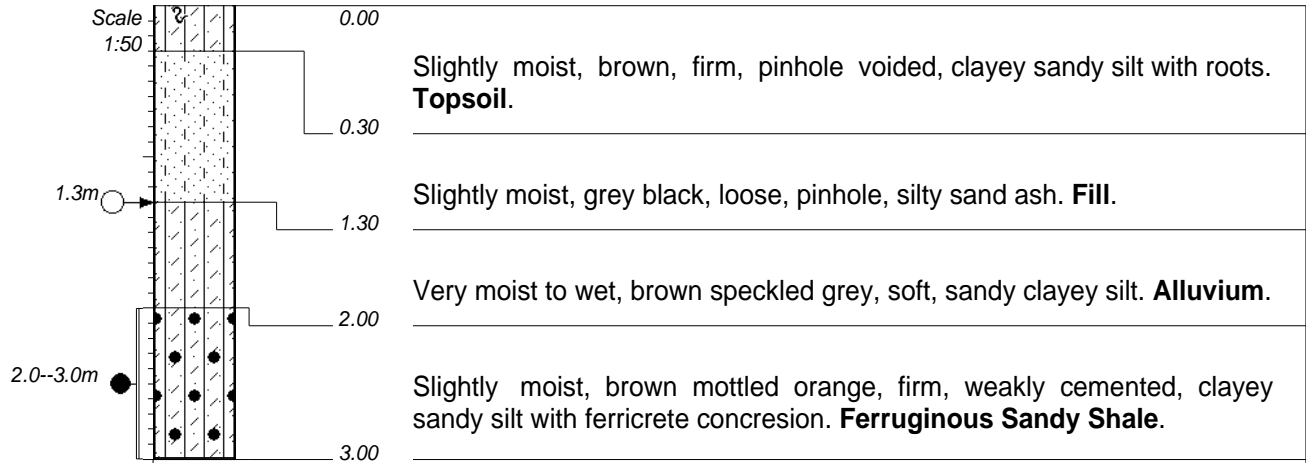
NOTES

- 1) Tractor loader backhoe used for the excavations.
- 2) Groundwater seepage at 2.0m.
- 3) Sidewalls stable.
- 4) No soils samples taken.
- 5) Excavated to maximum reach of the machine.

CONTRACTOR :
 MACHINE : TLB
 DRILLED BY :
 PROFILED BY : G. Rabodiba Pr.Sci.Nat
 TYPE SET BY :
 SETUP FILE : STANDARD.SET

INCLINATION :
 DIAM : Trench
 DATE :
 DATE : 2016/08/07
 DATE : 27/09/2016 19:58
 TEXT : ..PITS\RandWaterTP1120.txt

ELEVATION :
 X-COORD :
 Y-COORD :



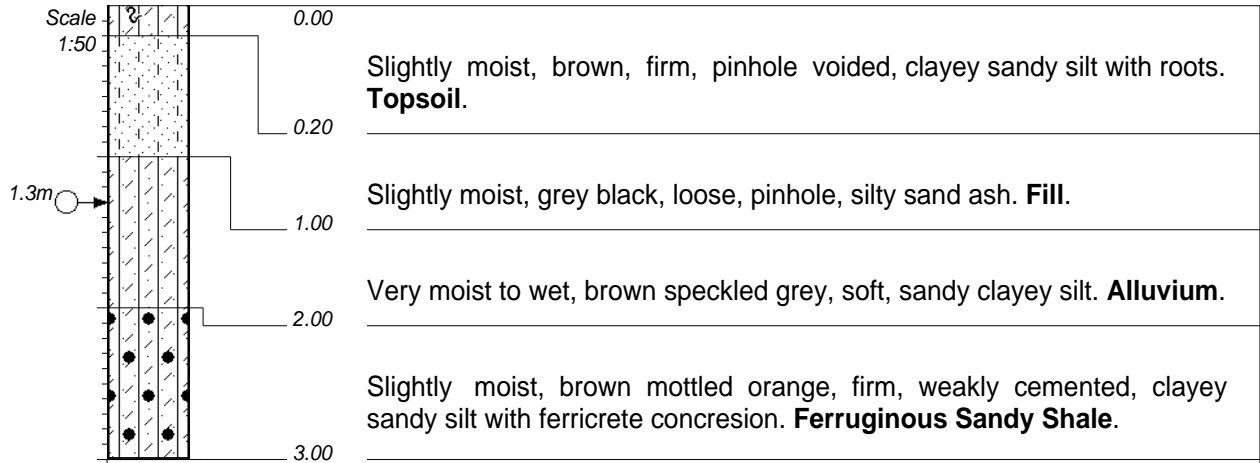
NOTES

- 1) Tractor loader backhoe used for the excavations.
- 2) Groundwater seepage at 1.3m.
- 3) Sidewalls stable.
- 4) Bulk and disturbed soils samples taken 2.0--3.0m.
- 5) Excavated to maximum reach of the machine.

CONTRACTOR :
 MACHINE : TLB
 DRILLED BY :
 PROFILED BY : G. Rabodiba Pr.Sci.Nat
 TYPE SET BY :
 SETUP FILE : STANDARD.SET

INCLINATION :
 DIAM : Trench
 DATE :
 DATE : 2016/08/07
 DATE : 27/09/2016 19:58
 TEXT : ..PITS\RandWaterTP1120.txt

ELEVATION :
 X-COORD :
 Y-COORD :



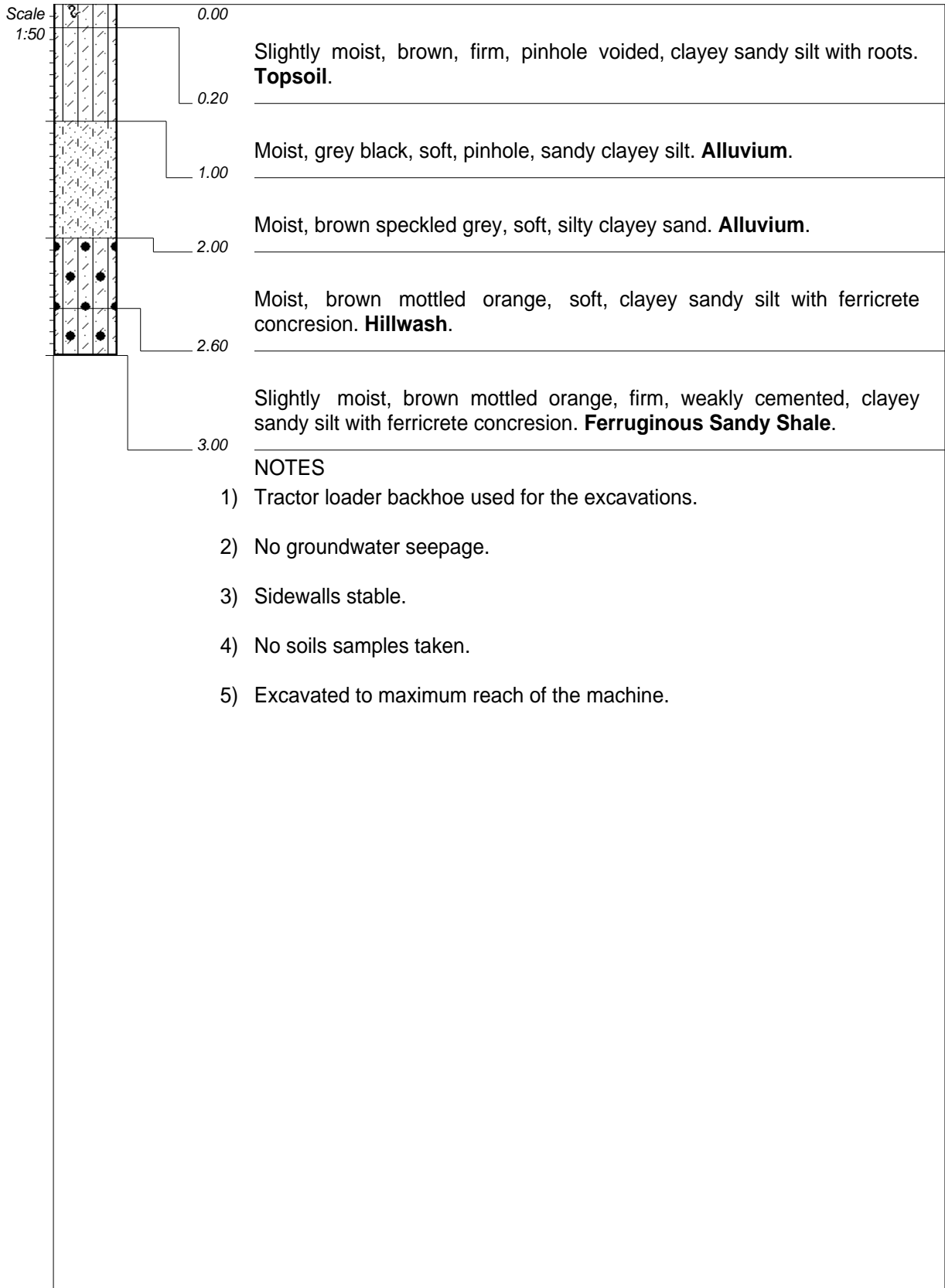
NOTES

- 1) Tractor loader backhoe used for the excavations.
- 2) Groundwater seepage at 1.3m.
- 3) Sidewalls stable.
- 4) No soils samples taken.
- 5) Excavated to maximum reach of the machine.

CONTRACTOR :
 MACHINE : TLB
 DRILLED BY :
 PROFILED BY : G. Rabodiba Pr.Sci.Nat
 TYPE SET BY :
 SETUP FILE : STANDARD.SET

INCLINATION :
 DIAM : Trench
 DATE :
 DATE : 2016/08/07
 DATE : 27/09/2016 19:58
 TEXT : ..PITS\RandWaterTP1120.txt

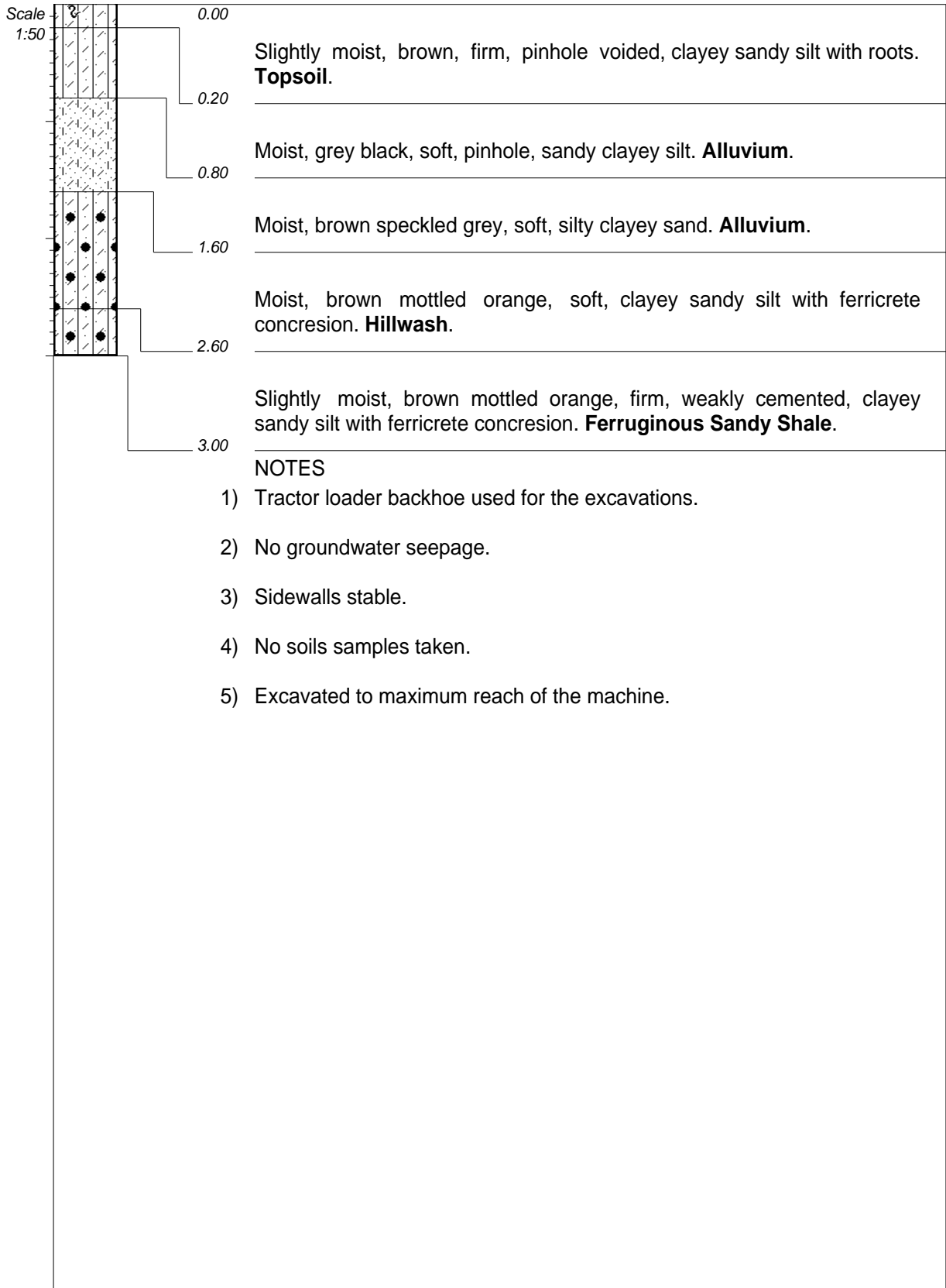
ELEVATION :
 X-COORD :
 Y-COORD :



CONTRACTOR :
MACHINE : TLB
DRILLED BY :
PROFILED BY : G. Rabodiba Pr.Sci.Nat
TYPE SET BY :
SETUP FILE : STANDARD.SET

INCLINATION :
DIAM : Trench
DATE :
DATE : 2016/08/07
DATE : 27/09/2016 19:58
TEXT : ..PITS\RandWaterTP1120.txt

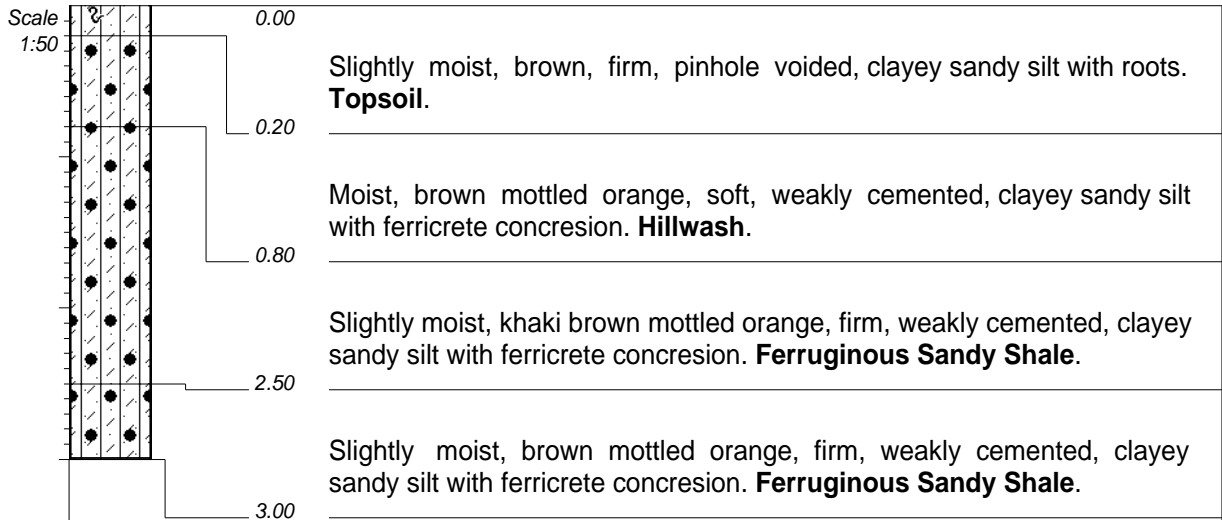
ELEVATION :
X-COORD :
Y-COORD :



CONTRACTOR :
MACHINE : TLB
DRILLED BY :
PROFILED BY : G. Rabodiba Pr.Sci.Nat
TYPE SET BY :
SETUP FILE : STANDARD.SET

INCLINATION :
DIAM : Trench
DATE :
DATE : 2016/08/07
DATE : 27/09/2016 19:59
TEXT : ..PITS\RandwaterTP2130.txt

ELEVATION :
X-COORD :
Y-COORD :



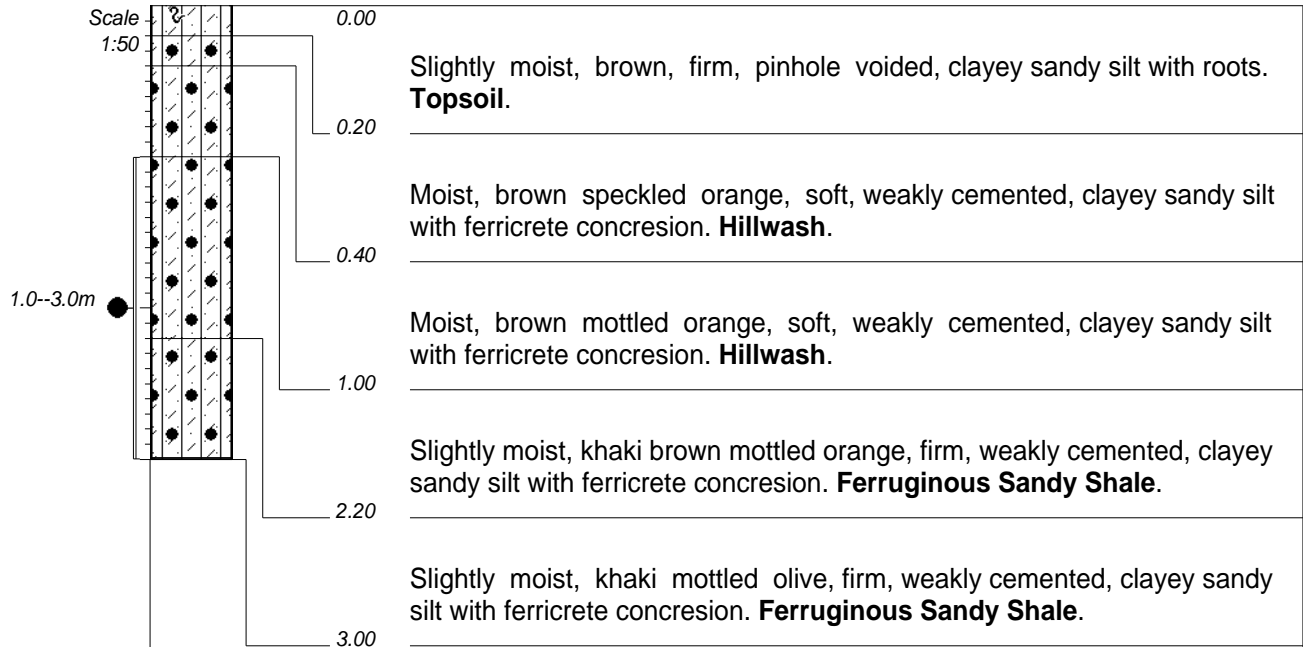
NOTES

- 1) Tractor loader backhoe used for the excavations.
- 2) No groundwater seepage.
- 3) Sidewalls stable.
- 4) No soils samples taken.
- 5) Excavated to maximum reach of the machine.

CONTRACTOR :
MACHINE : TLB
DRILLED BY :
PROFILED BY : G. Rabodiba Pr.Sci.Nat
TYPE SET BY :
SETUP FILE : STANDARD.SET

INCLINATION :
DIAM : Trench
DATE :
DATE : 2016/08/07
DATE : 27/09/2016 19:59
TEXT : ..PITS\RandwaterTP2130.txt

ELEVATION :
X-COORD :
Y-COORD :



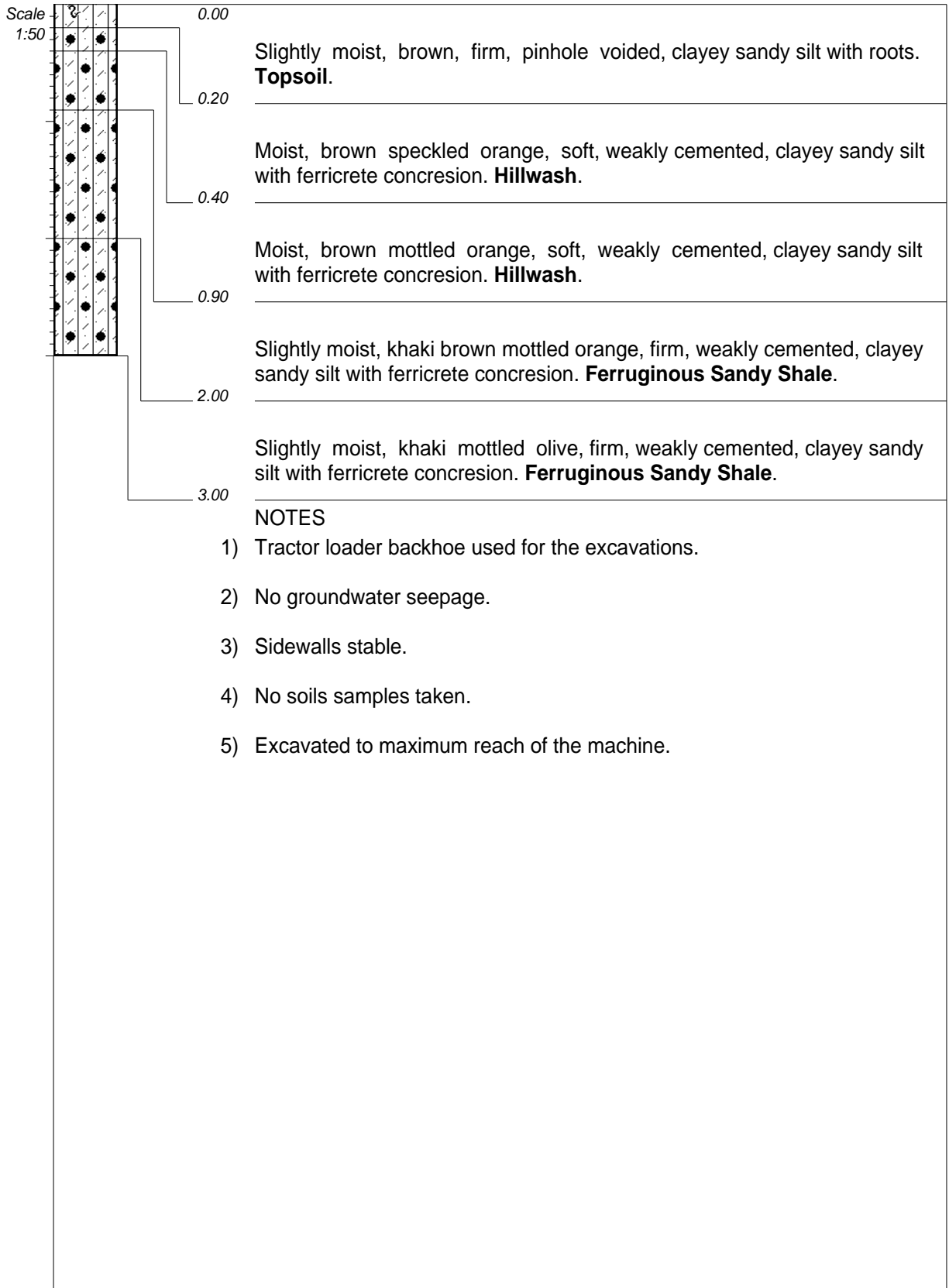
NOTES

- 1) Tractor loader backhoe used for the excavations.
- 2) No groundwater seepage.
- 3) Sidewalls stable.
- 4) Bulk and disturbed soils samples taken at 1.0--3.0m.
- 5) Excavated to maximum reach of the machine.

CONTRACTOR :
 MACHINE : TLB
 DRILLED BY :
 PROFILED BY : G. Rabodiba Pr.Sci.Nat
 TYPE SET BY :
 SETUP FILE : STANDARD.SET

INCLINATION :
 DIAM : Trench
 DATE :
 DATE : 2016/08/07
 DATE : 27/09/2016 19:59
 TEXT : ..PITS\RandwaterTP2130.txt

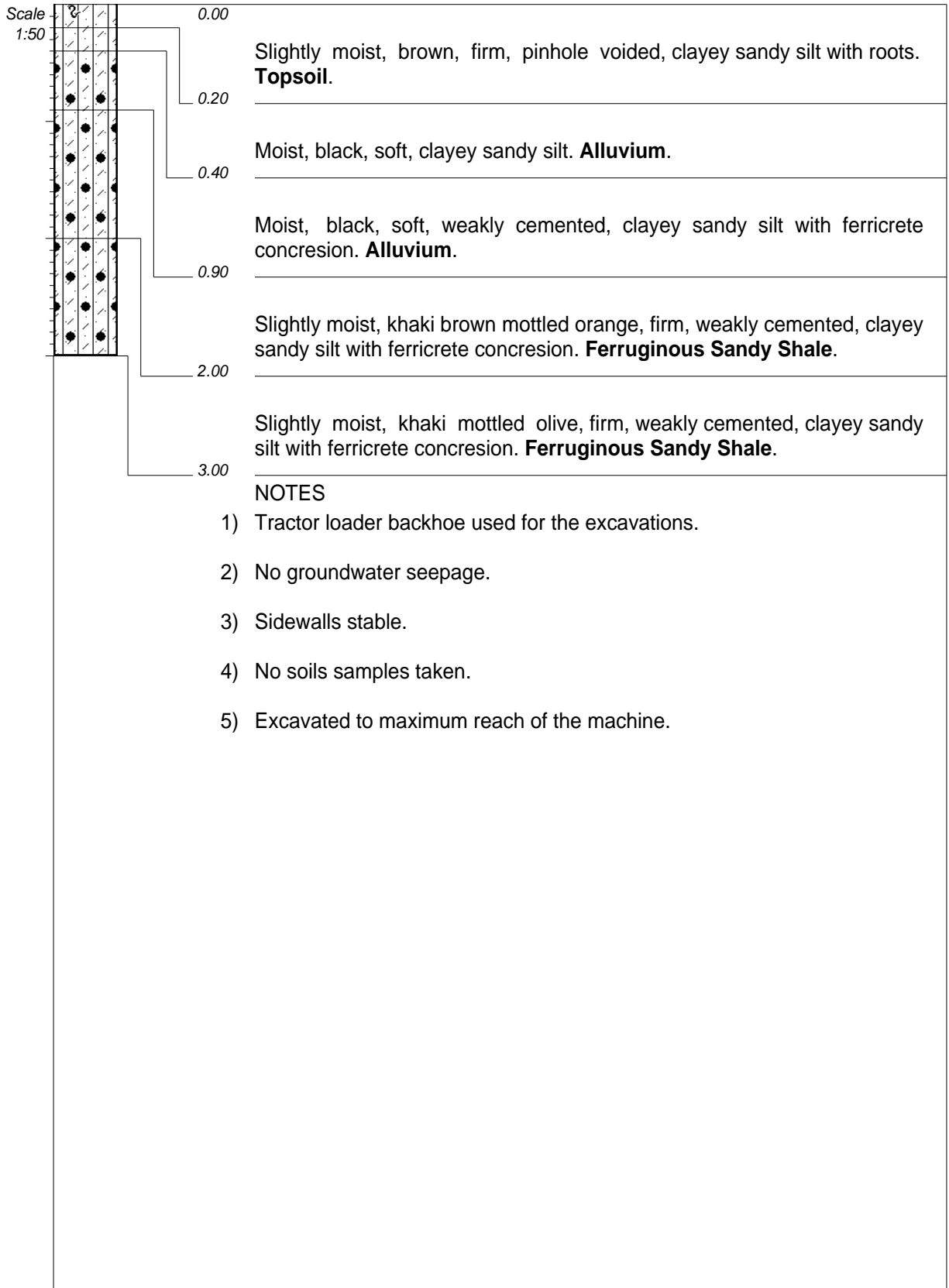
ELEVATION :
 X-COORD :
 Y-COORD :



CONTRACTOR :
 MACHINE : TLB
 DRILLED BY :
 PROFILED BY : G. Rabodiba Pr.Sci.Nat
 TYPE SET BY :
 SETUP FILE : STANDARD.SET

INCLINATION :
 DIAM : Trench
 DATE :
 DATE : 2016/08/07
 DATE : 27/09/2016 19:59
 TEXT : ..PITS\RandwaterTP2130.txt

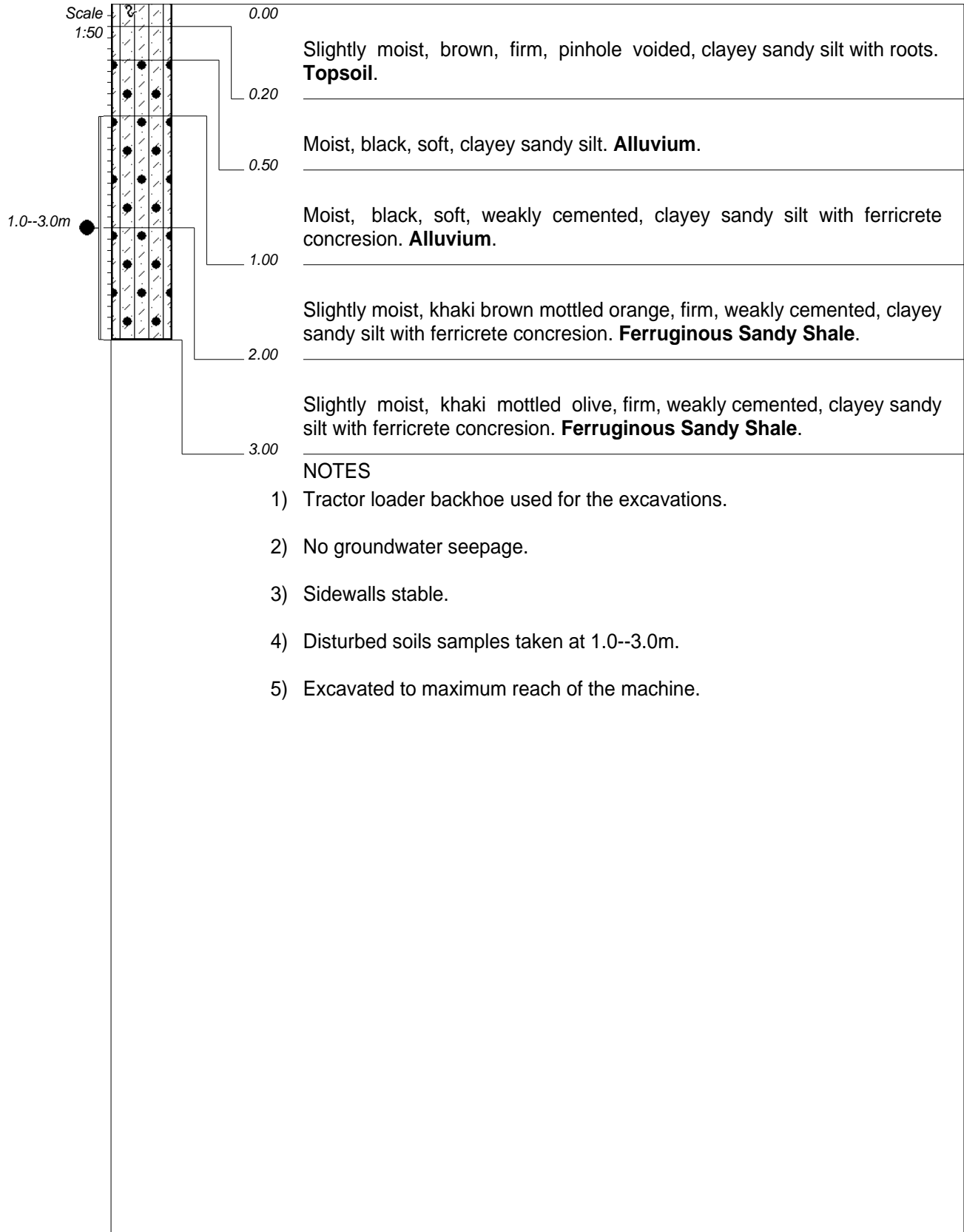
ELEVATION :
 X-COORD :
 Y-COORD :



CONTRACTOR :
 MACHINE : TLB
 DRILLED BY :
 PROFILED BY : G. Rabodiba Pr.Sci.Nat
 TYPE SET BY :
 SETUP FILE : STANDARD.SET

INCLINATION :
 DIAM : Trench
 DATE :
 DATE : 2016/08/07
 DATE : 27/09/2016 19:59
 TEXT : ..PITS\RandwaterTP2130.txt

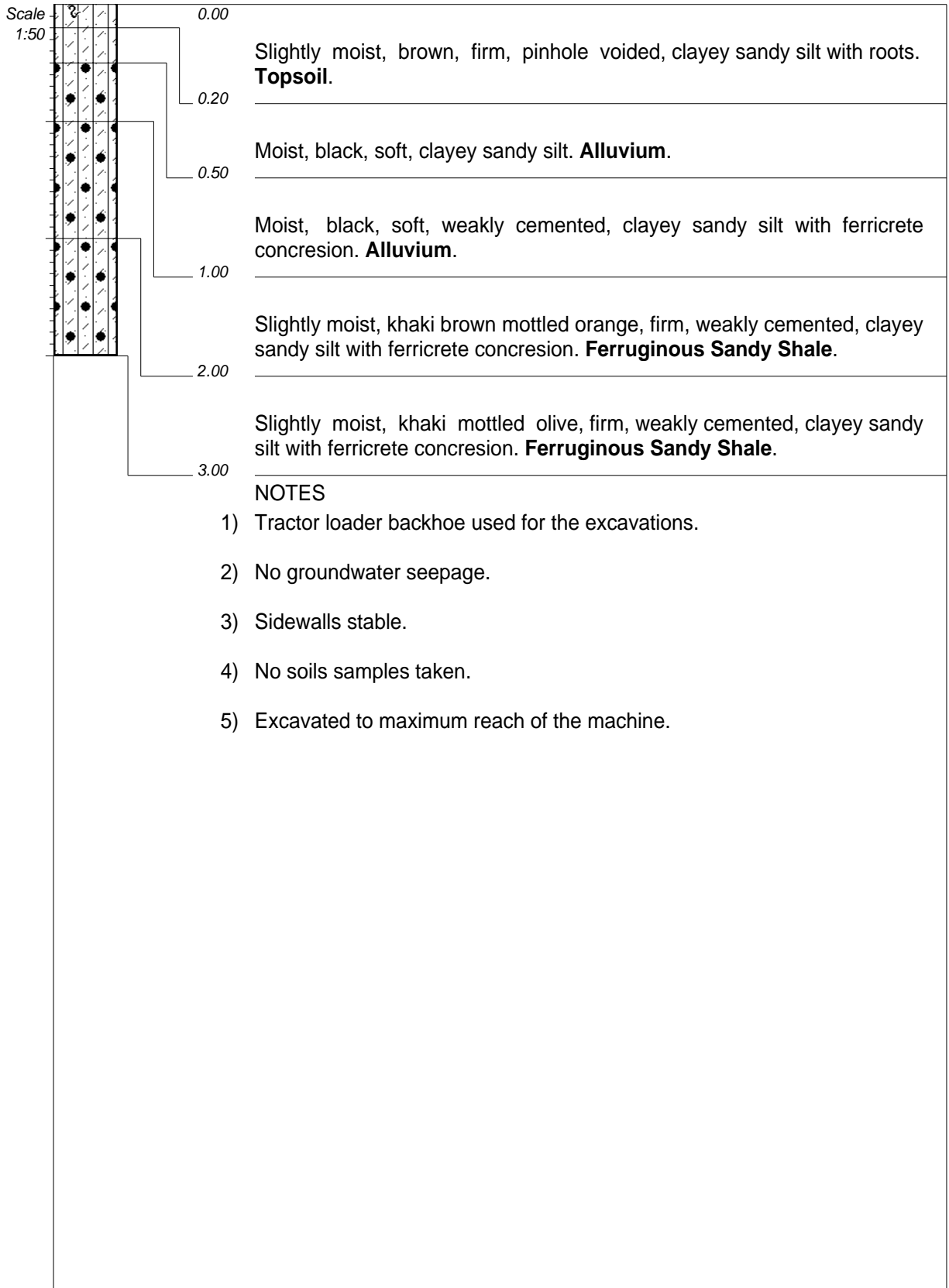
ELEVATION :
 X-COORD :
 Y-COORD :



CONTRACTOR :
 MACHINE : TLB
 DRILLED BY :
 PROFILED BY : G. Rabodiba Pr.Sci.Nat
 TYPE SET BY :
 SETUP FILE : STANDARD.SET

INCLINATION :
 DIAM : Trench
 DATE :
 DATE : 2016/08/07
 DATE : 27/09/2016 19:59
 TEXT : ..PITS\RandwaterTP2130.txt

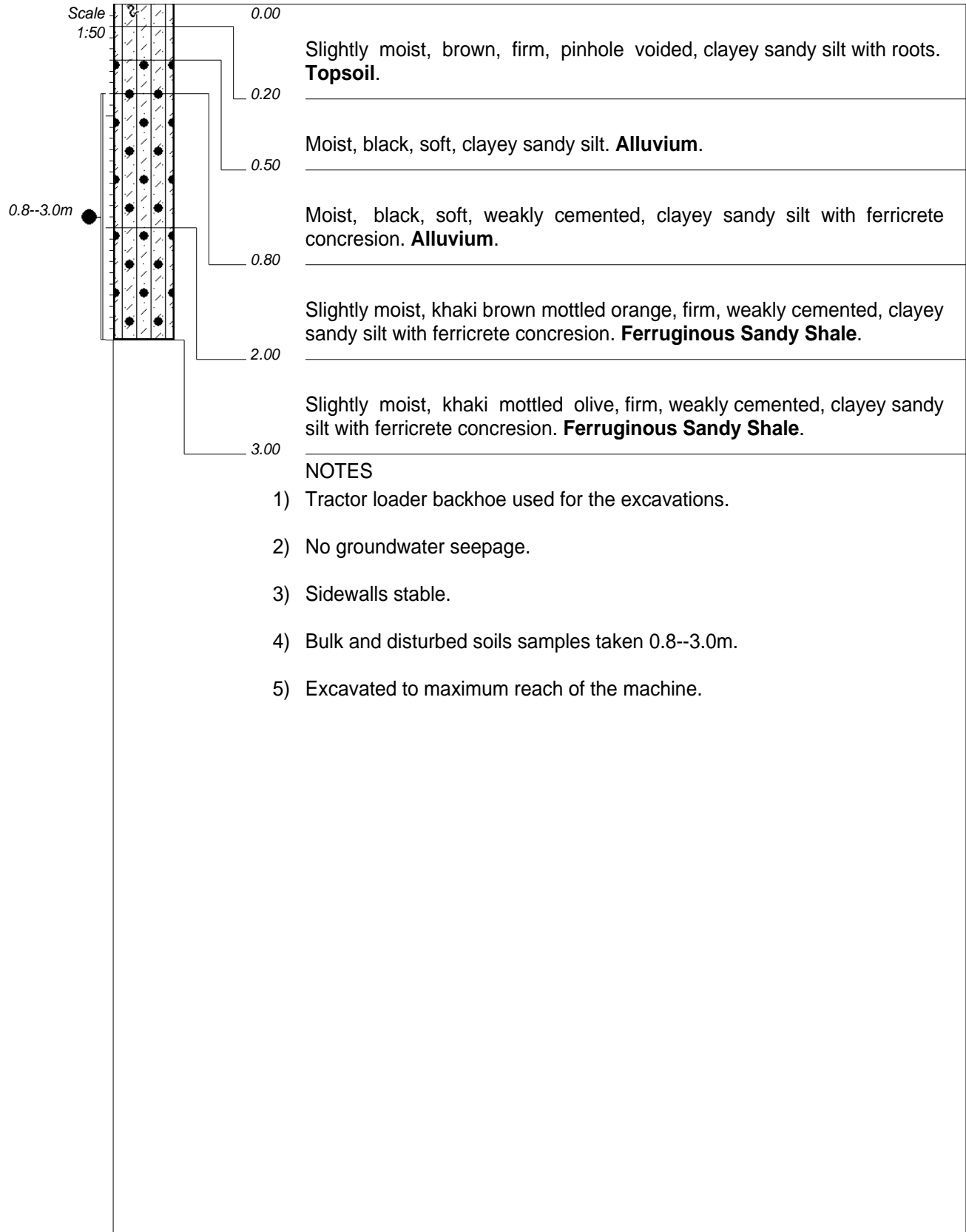
ELEVATION :
 X-COORD :
 Y-COORD :



CONTRACTOR :
 MACHINE : TLB
 DRILLED BY :
 PROFILED BY : G. Rabodiba Pr.Sci.Nat
 TYPE SET BY :
 SETUP FILE : STANDARD.SET

INCLINATION :
 DIAM : Trench
 DATE :
 DATE : 2016/08/07
 DATE : 27/09/2016 19:59
 TEXT : ..PITS\RandwaterTP2130.txt

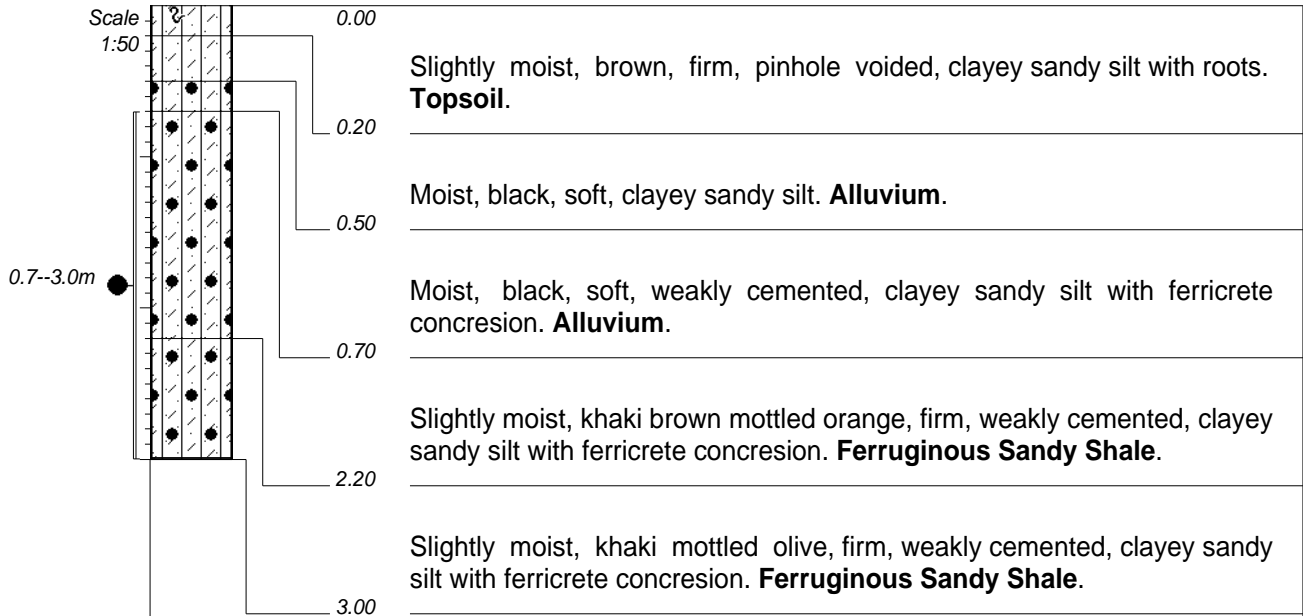
ELEVATION :
 X-COORD :
 Y-COORD :



CONTRACTOR :
 MACHINE : TLB
 DRILLED BY :
 PROFILED BY : G. Rabodiba Pr.Sci.Nat
 TYPE SET BY :
 SETUP FILE : STANDARD.SET

INCLINATION :
 DIAM : Trench
 DATE :
 DATE : 2016/08/07
 DATE : 27/09/2016 19:59
 TEXT : ..PITS\RandwaterTP2130.txt

ELEVATION :
 X-COORD :
 Y-COORD :



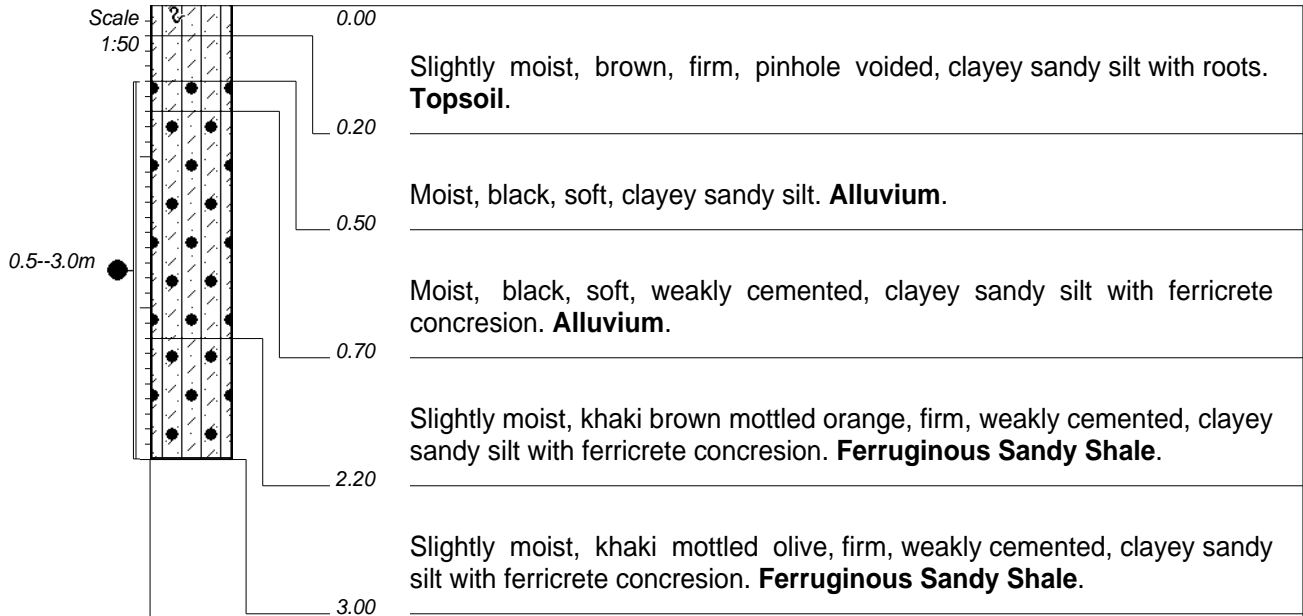
NOTES

- 1) Tractor loader backhoe used for the excavations.
- 2) No groundwater seepage.
- 3) Sidewalls stable.
- 4) Bulk and disturbed soils samples taken 0.7--3.0m.
- 5) Excavated to maximum reach of the machine.

CONTRACTOR :
 MACHINE : TLB
 DRILLED BY :
 PROFILED BY : G. Rabodiba Pr.Sci.Nat
 TYPE SET BY :
 SETUP FILE : STANDARD.SET

INCLINATION :
 DIAM : Trench
 DATE :
 DATE : 2016/08/07
 DATE : 27/09/2016 19:59
 TEXT : ..PITS\RandwaterTP2130.txt

ELEVATION :
 X-COORD :
 Y-COORD :



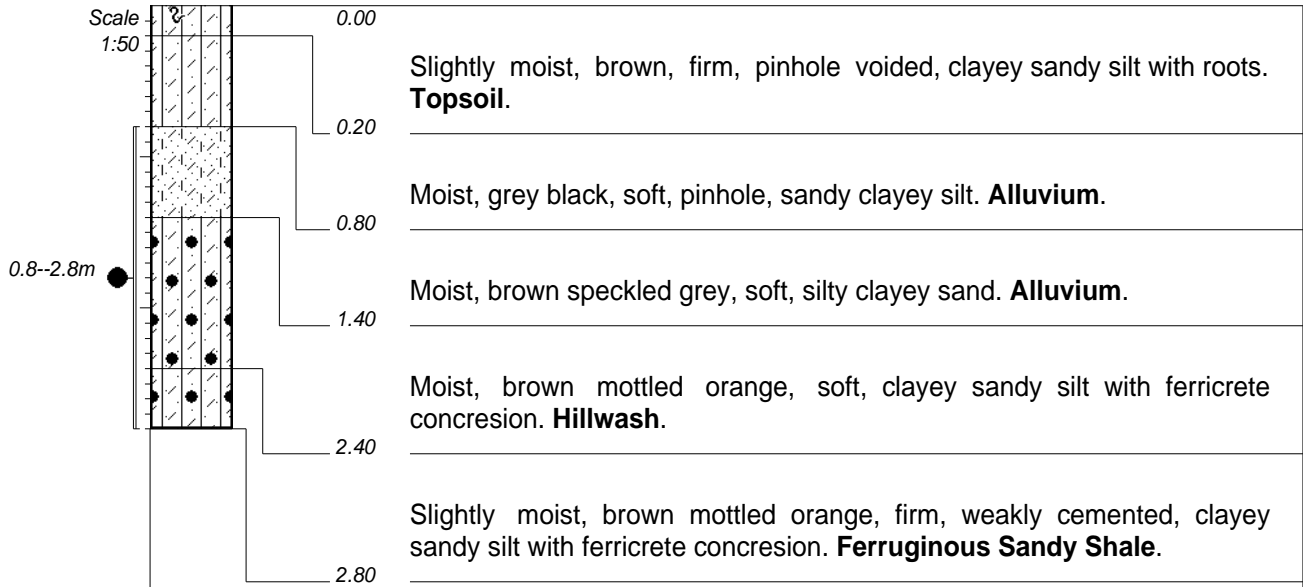
NOTES

- 1) Tractor loader backhoe used for the excavations.
- 2) No groundwater seepage.
- 3) Sidewalls stable.
- 4) Bulk and disturbed soils samples taken 0.5--3.0m.
- 5) Excavated to maximum reach of the machine.

CONTRACTOR :
 MACHINE : TLB
 DRILLED BY :
 PROFILED BY : G. Rabodiba Pr.Sci.Nat
 TYPE SET BY :
 SETUP FILE : STANDARD.SET

INCLINATION :
 DIAM : Trench
 DATE :
 DATE : 2016/08/07
 DATE : 27/09/2016 19:59
 TEXT : ..PITS\RandwaterTP2130.txt

ELEVATION :
 X-COORD :
 Y-COORD :



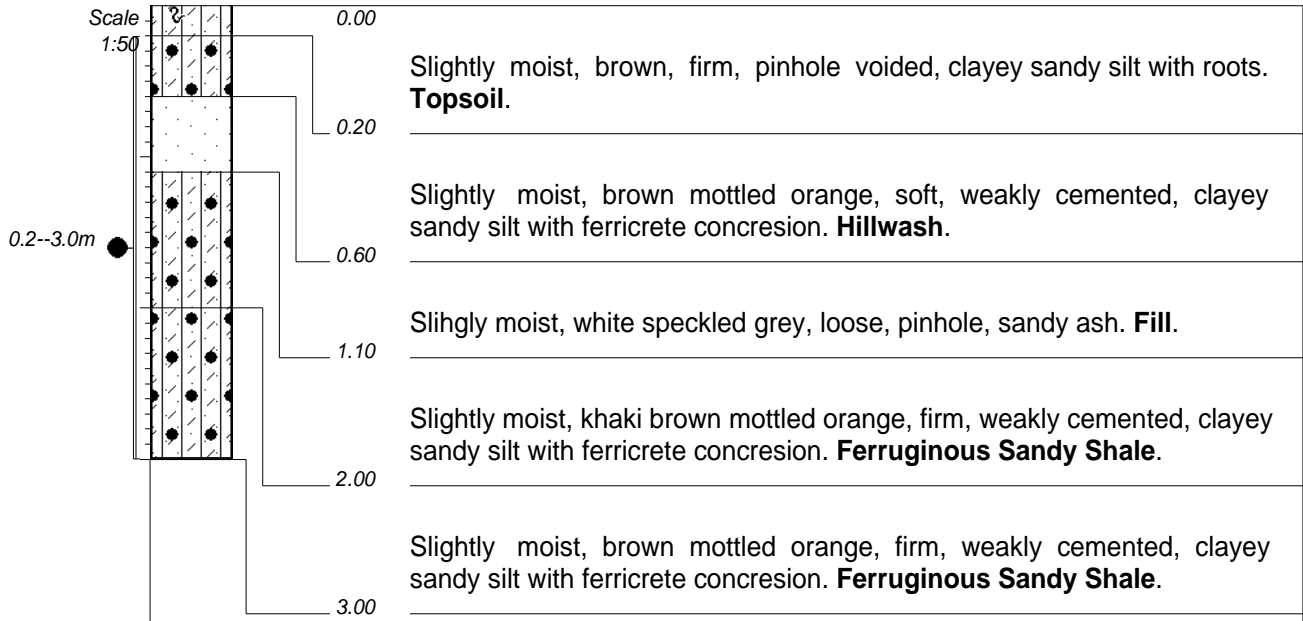
NOTES

- 1) Tractor loader backhoe used for the excavations.
- 2) No groundwater seepage.
- 3) Sidewalls stable.
- 4) Bulk and disturbed soils samples taken at 0.8--2.8m.
- 5) Excavated to maximum reach of the machine.

CONTRACTOR :
 MACHINE : TLB
 DRILLED BY :
 PROFILED BY : G. Rabodiba Pr.Sci.Nat
 TYPE SET BY :
 SETUP FILE : STANDARD.SET

INCLINATION :
 DIAM : Trench
 DATE :
 DATE : 2016/08/07
 DATE : 27/09/2016 20:00
 TEXT : ..PITS\RandwaterTP3135.txt

ELEVATION :
 X-COORD :
 Y-COORD :



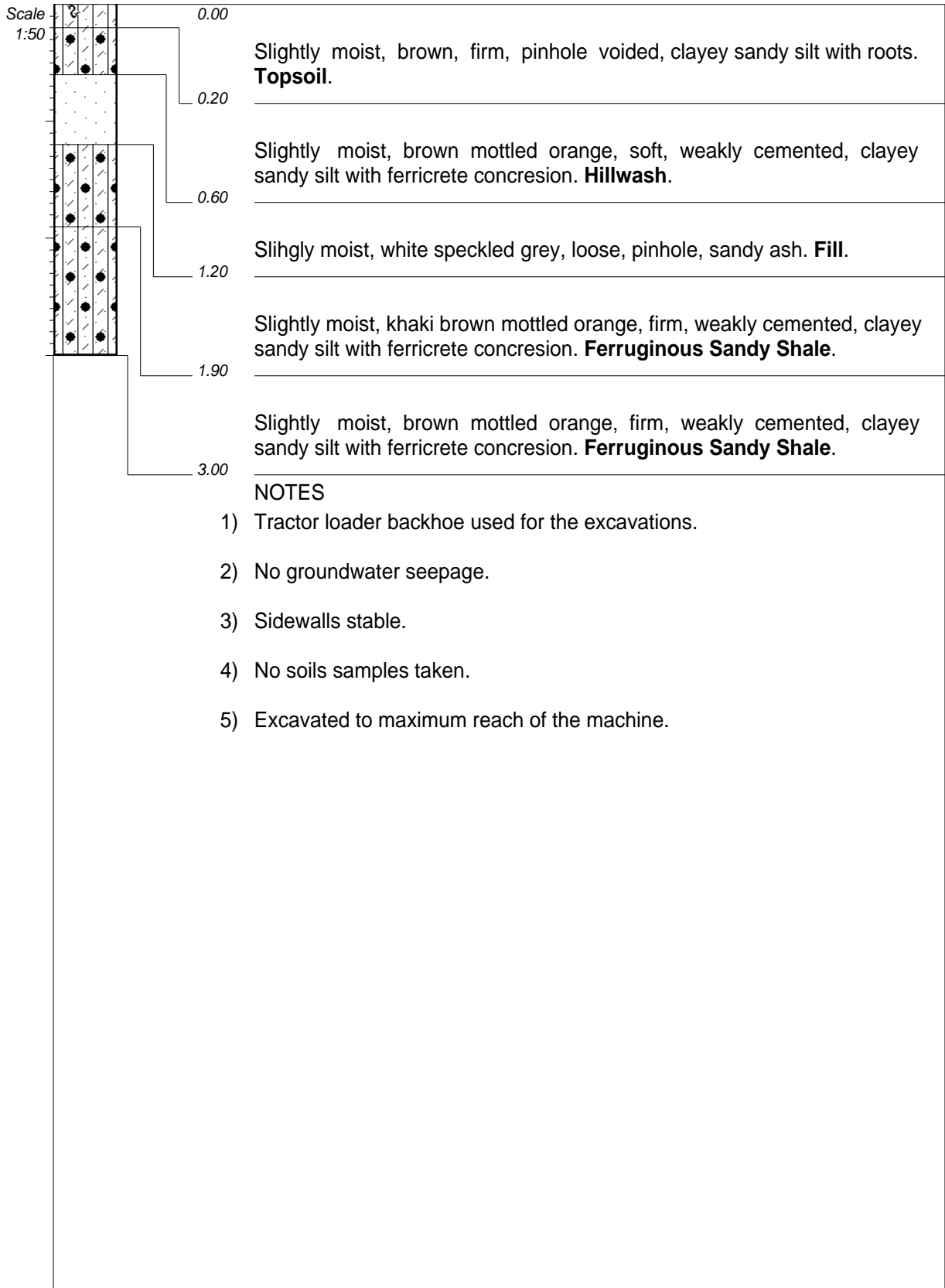
NOTES

- 1) Tractor loader backhoe used for the excavations.
- 2) No groundwater seepage.
- 3) Sidewalls stable.
- 4) Bulk and disturbed soils samples taken at 0.2--3.0m.
- 5) Excavated to maximum reach of the machine.

CONTRACTOR :
 MACHINE : TLB
 DRILLED BY :
 PROFILED BY : G. Rabodiba Pr.Sci.Nat
 TYPE SET BY :
 SETUP FILE : STANDARD.SET

INCLINATION :
 DIAM : Trench
 DATE :
 DATE : 2016/08/07
 DATE : 27/09/2016 20:00
 TEXT : ..PITS\RandwaterTP3135.txt

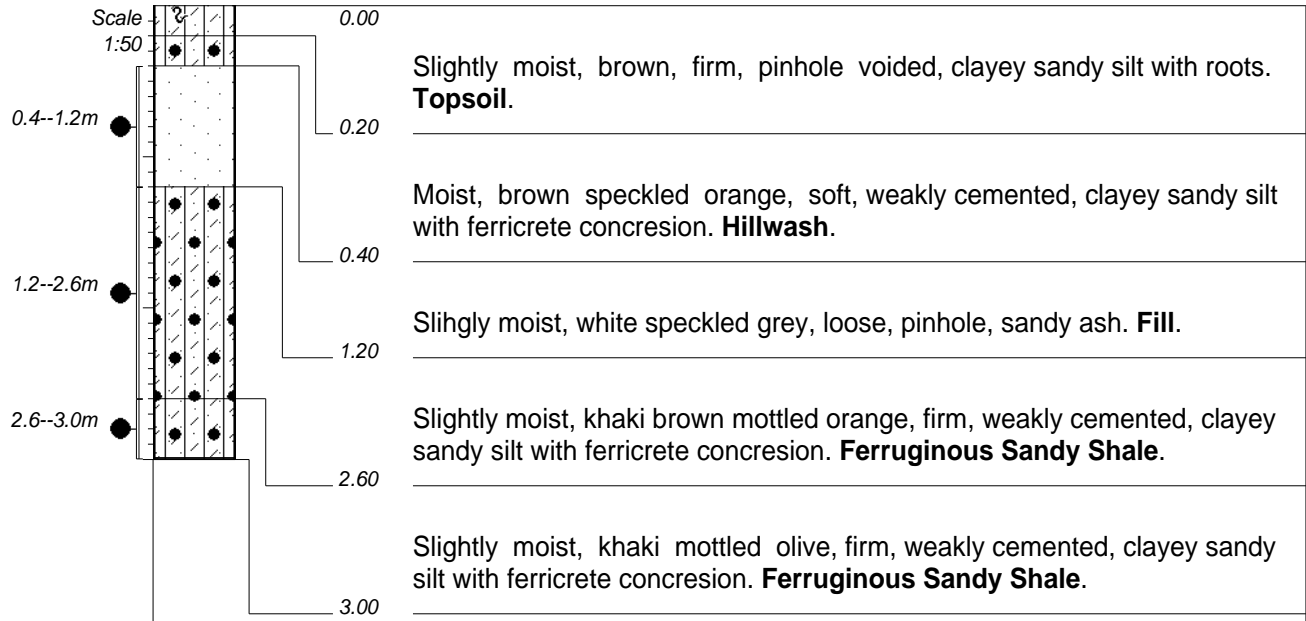
ELEVATION :
 X-COORD :
 Y-COORD :



CONTRACTOR :
 MACHINE : TLB
 DRILLED BY :
 PROFILED BY : G. Rabodiba Pr.Sci.Nat
 TYPE SET BY :
 SETUP FILE : STANDARD.SET

INCLINATION :
 DIAM : Trench
 DATE :
 DATE : 2016/08/07
 DATE : 27/09/2016 20:00
 TEXT : ..PITS\RandwaterTP3135.txt

ELEVATION :
 X-COORD :
 Y-COORD :



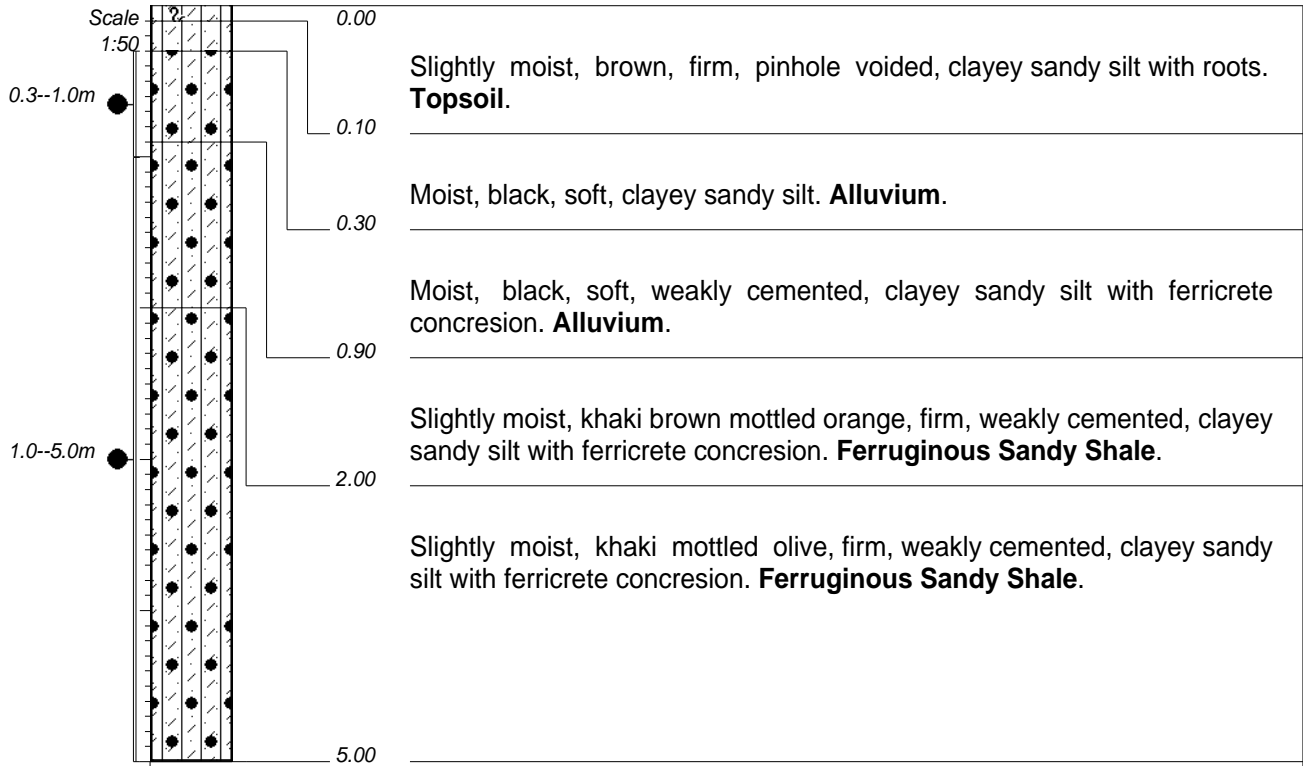
NOTES

- 1) Tractor loader backhoe used for the excavations.
- 2) No groundwater seepage.
- 3) Sidewalls stable.
- 4) Bulk and disturbed soils samples taken at 0.4--1.2m & 1.2--2.6m & 2.6--3.0m.
- 5) Excavated to maximum reach of the machine.

CONTRACTOR :
 MACHINE : TLB
 DRILLED BY :
 PROFILED BY : G. Rabodiba Pr.Sci.Nat
 TYPE SET BY :
 SETUP FILE : STANDARD.SET

INCLINATION :
 DIAM : Trench
 DATE :
 DATE : 2016/08/07
 DATE : 27/09/2016 20:00
 TEXT : ..PITS\RandwaterTP3135.txt

ELEVATION :
 X-COORD :
 Y-COORD :











NOTES

- 1) Tractor loader backhoe used for the excavations.
- 2) No groundwater seepage.
- 3) Sidewalls stable.
- 4) Disturbed soils samples taken at 0.3--1.0m & 1.0--5.0m.
- 5) Excavated to maximum reach of the machine.

CONTRACTOR :
 MACHINE : TLB
 DRILLED BY :
 PROFILED BY : G. Rabodiba Pr.Sci.Nat
 TYPE SET BY :
 SETUP FILE : STANDARD.SET

INCLINATION :
 DIAM : Trench
 DATE :
 DATE : 2016/08/07
 DATE : 27/09/2016 20:00
 TEXT : ..PITS\RandwaterTP3135.txt

ELEVATION :
 X-COORD :
 Y-COORD :

	SAND	{SA04}
	SANDY	{SA05}
	SILT	{SA06}
	SILTY	{SA07}
	CLAYEY	{SA09}
	FERRICRETE	{SA24}
	DISTURBED SAMPLE	{SA38}
	ROOTS	{SA40}

CONTRACTOR :
 MACHINE :
 DRILLED BY :
 PROFILED BY :

INCLINATION :
 DIAM :
 DATE :
 DATE :

ELEVATION :
 X-COORD :
 Y-COORD :

TYPE SET BY :
 SETUP FILE : STANDARD.SET

DATE : 27/09/2016 20:00
 TEXT : ..PITS\RandwaterTP3135.txt

ANNEXURE C
TEST PITS PHOTOGRAPHS

TP 1



TP 1

TP2



TP2

TP3



TP3

TP4



TP4

TP5



TP5

TP6



TP6

TP7



TP7

TP8



TP8

TP9



TP9

TP10





TP10

TP 11



TP 11

TP12



TP12

TP13



TP13

TP14



TP14

TP15



TP15

TP16



TP16

TP17



TP17

TP18



TP18

TP19



TP19

TP20



TP20

TP 21



TP 21

TP22



TP22

TP23



TP23

TP24



TP24

TP25



TP25

TP26



TP26

TP27



TP27

TP28



TP28

TP29



TP29

TP30



TP30

TP 31



TP 31

TP32



TP32

TP33



TP33

TP34



TP34

TP35



TP35

ANNEXURE D
LABORATORY TEST RESULTS

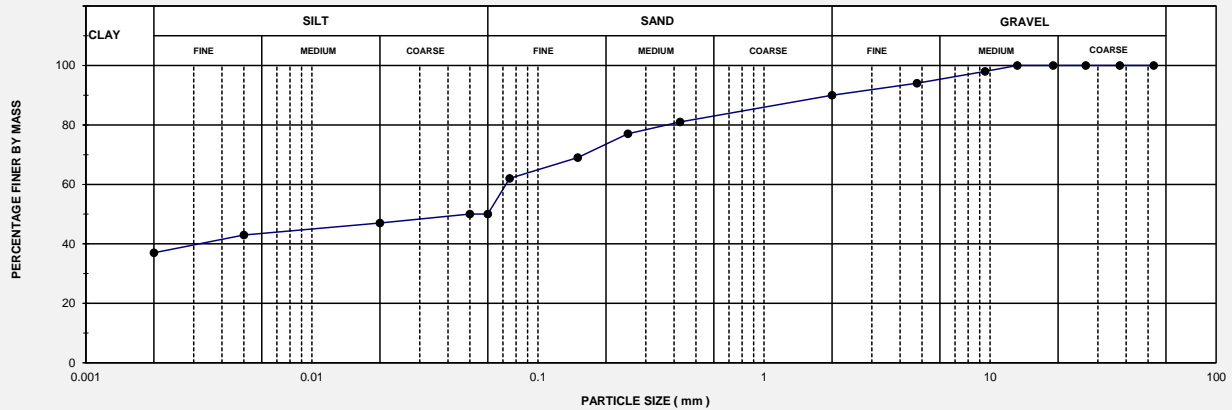


FOUNDATION INDICATOR TEST RESULTS - REP COM 7

Client: Nomfundo Exploration	Source/Location:	Job No: 2016-C-991
Project Name: Rand Water Pipeline Project in Vereenig. Layer:		Sample No: 6/7724
Project No: 0	Lane:	Date: 02/09/2016
Hole/TP No: TP1	Stabilizing Agent:	Test Method: TMH1 A1, A5 & ASTM D422
Depth (m): 0.5 - 1.0	Section:	Client Ref No:
Description: Black speckled khakhi brown sandy silty	Chainage:	GPS X:
Additional Info:	Offset:	GPS Y:

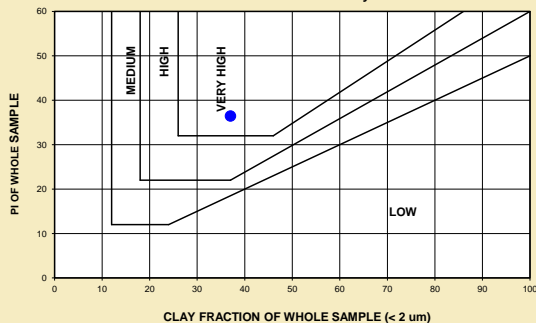
SIEVE ANALYSIS				ATTERBERG LIMITS		SOIL CLASSIFICATION	
Sieve (mm)	% Passing	Sieve (mm)	% Passing				
75.0	100	0.425	81	Liquid Limit (%)	67	% Gravel	10
63.0	100	0.250	77	Plastic Limit (%)	22	% Sand	40
53.0	100	0.150	69	Plasticity Index (%)	45	% Silt	13
37.5	100	0.075	62	Weighted PI (%)	36.5	% Clay	37
26.5	100	0.060	50	Linear Shrinkage (%)	12.0	Activity	1.2
19.0	100	0.050	50	Grading Modulus	0.67	% Soil Mortar	90
13.2	100	0.020	47	Uniformity coefficient	36	Coarse Sand Ratio	0.10
9.5	98	0.005	43	Coefficient of curvature	0.0	TRB Classification	A - 7 - 6
4.75	94	0.002	37			Unified Classification	CH
2.00	90			Remarks:			

PARTICLE SIZE DISTRIBUTION

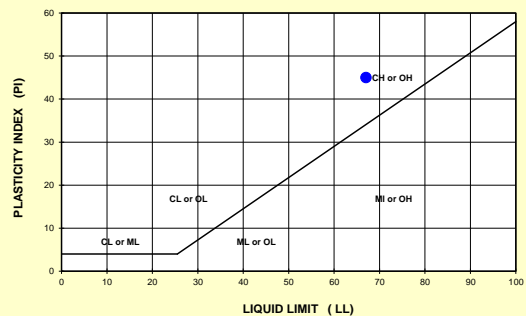


POTENTIAL EXPANSIVENESS

Van der Merwe's Activity Chart



CASAGRANDE 'A' LINE



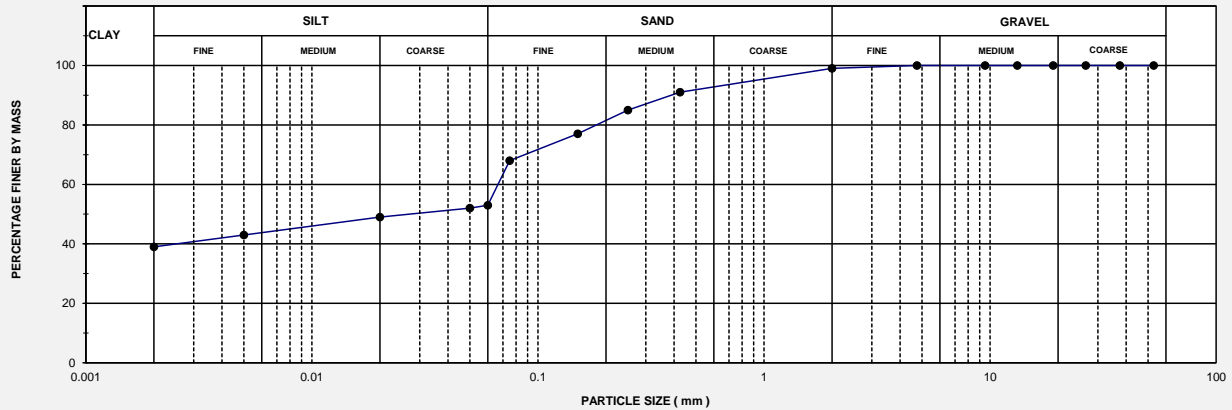


FOUNDATION INDICATOR TEST RESULTS - REP COM 7

Client:	Nomfundo Exploration	Source/Location:		Job No:	2016-C-991
Project Name:	Rand Water Pipeline Project in Vereenig. Layer:			Sample No:	6/7723
Project No:	0	Lane:		Date:	02/09/2016
Hole/TP No:	TP1	Stabilizing Agent:		Test Method:	TMH1 A1, A5 & ASTM D422
Depth (m):	0.15 - 0.5	Section:		Client Ref No:	
Description:	Black sandy silty clay (Hillwash)	Chainage:		GPS X:	
Additional Info:		Offset:		GPS Y:	

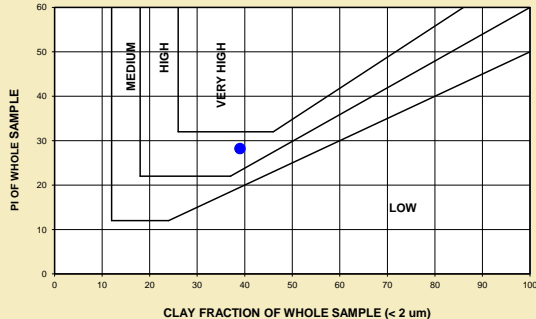
SIEVE ANALYSIS				ATTERBERG LIMITS		SOIL CLASSIFICATION	
Sieve (mm)	% Passing	Sieve (mm)	% Passing				
75.0	100	0.425	91	Liquid Limit (%)	52	% Gravel	1
63.0	100	0.250	85	Plastic Limit (%)	21	% Sand	46
53.0	100	0.150	77	Plasticity Index (%)	31	% Silt	14
37.5	100	0.075	68	Weighted PI (%)	28.2	% Clay	39
26.5	100	0.060	53	Linear Shrinkage (%)	11.0	Activity	0.8
19.0	100	0.050	52	Grading Modulus	0.42	% Soil Mortar	99
13.2	100	0.020	49	Uniformity coefficient	34	Coarse Sand Ratio	0.08
9.5	100	0.005	43	Coefficient of curvature	0.0	TRB Classification	A - 7 - 6
4.75	100	0.002	39			Unified Classification	CH
2.00	99			Remarks:			

PARTICLE SIZE DISTRIBUTION

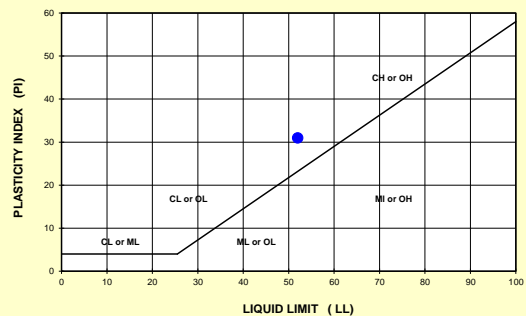


POTENTIAL EXPANSIVENESS

Van der Merwe's Activity Chart



CASAGRANDE 'A' LINE



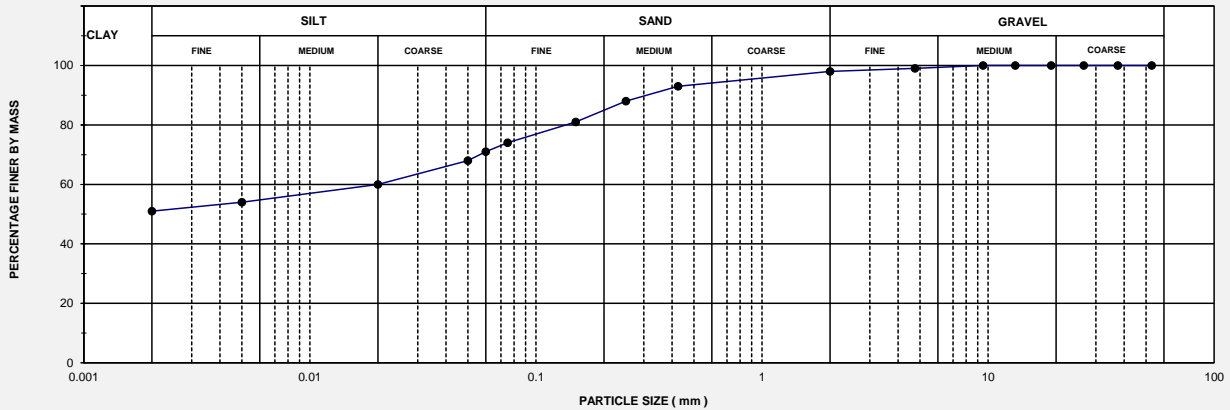


FOUNDATION INDICATOR TEST RESULTS - REP COM 7

Client: Nomfundo Exploration	Source/Location:	Job No: 2016-C-991
Project Name: Rand Water Pipeline Project in Vereenig. Layer:		Sample No: 6/7725
Project No: 0	Lane:	Date: 02/09/2016
Hole/TP No: TP1	Stabilizing Agent:	Test Method: TMH1 A1, A5 & ASTM D422
Depth (m): 1.0 - 2.2	Section:	Client Ref No:
Description: Khakhi blotched calcrete white sandy cla	Chainage:	GPS X:
Additional Info:	Offset:	GPS Y:

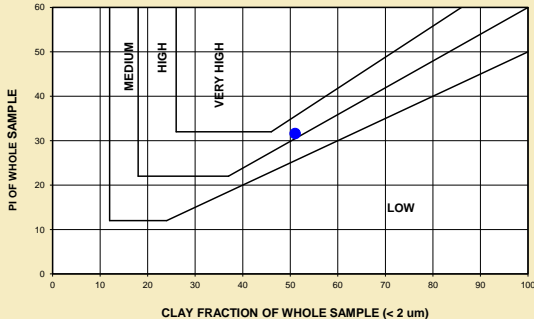
SIEVE ANALYSIS				ATTERBERG LIMITS		SOIL CLASSIFICATION	
Sieve (mm)	% Passing	Sieve (mm)	% Passing				
75.0	100	0.425	93	Liquid Limit (%)	60	% Gravel	2
63.0	100	0.250	88	Plastic Limit (%)	26	% Sand	27
53.0	100	0.150	81	Plasticity Index (%)	34	% Silt	20
37.5	100	0.075	74	Weighted PI (%)	31.6	% Clay	51
26.5	100	0.060	71	Linear Shrinkage (%)	15.5	Activity	0.7
19.0	100	0.050	68	Grading Modulus	0.35	% Soil Mortar	98
13.2	100	0.020	60	Uniformity coefficient	10	Coarse Sand Ratio	0.05
9.5	100	0.005	54	Coefficient of curvature	0.1	TRB Classification	A - 7 - 6
4.75	99	0.002	51			Unified Classification	CH
2.00	98			Remarks:			

PARTICLE SIZE DISTRIBUTION

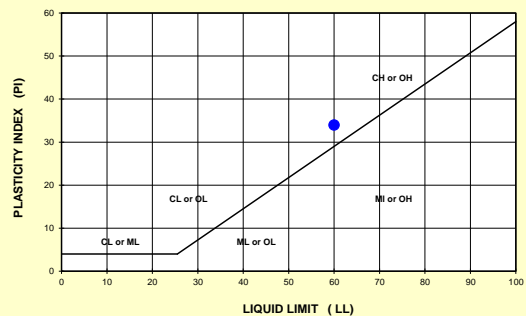


POTENTIAL EXPANSIVENESS

Van der Merwe's Activity Chart



CASAGRANDE 'A' LINE



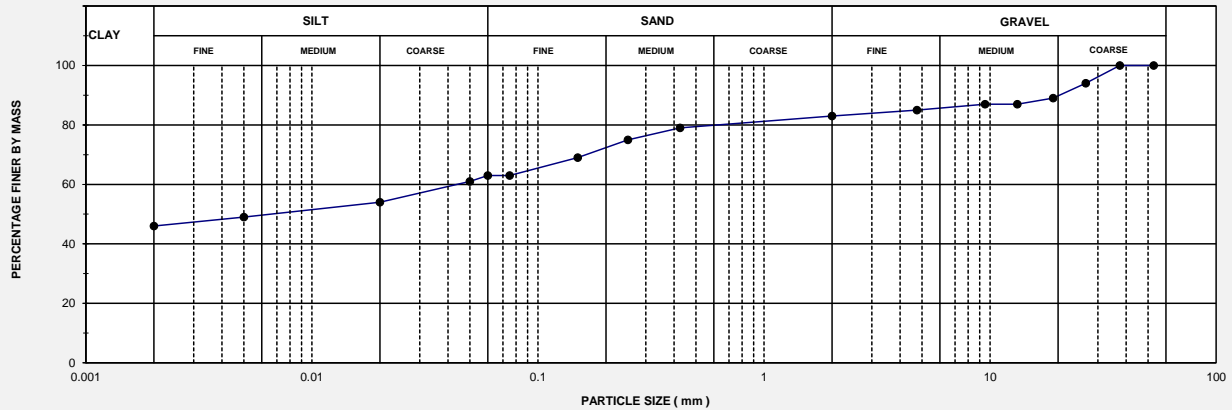


FOUNDATION INDICATOR TEST RESULTS - REP COM 7

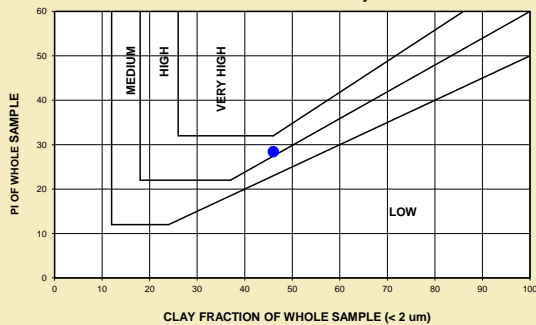
Client: Nomfundo Exploration	Source/Location:	Job No: 2016-C-991
Project Name: Rand Water Pipeline Project in Vereenig. Layer:		Sample No: 6/7726
Project No: 0	Lane:	Date: 02/09/2016
Hole/TP No: TP1	Stabilizing Agent:	Test Method: TMH1 A1, A5 & ASTM D422
Depth (m): 2.2 - 2.6	Section:	Client Ref No:
Description: Khakhi mottled grey & orange sandy clay Chainage:		GPS X:
Additional Info:	Offset:	GPS Y:

SIEVE ANALYSIS				ATTERBERG LIMITS		SOIL CLASSIFICATION	
Sieve (mm)	% Passing	Sieve (mm)	% Passing				
75.0	100	0.425	79	Liquid Limit (%)	62	% Gravel	17
63.0	100	0.250	75	Plastic Limit (%)	26	% Sand	20
53.0	100	0.150	69	Plasticity Index (%)	36	% Silt	17
37.5	100	0.075	63	Weighted PI (%)	28.4	% Clay	46
26.5	94	0.060	63	Linear Shrinkage (%)	15.5	Activity	0.8
19.0	89	0.050	61	Grading Modulus	0.75	% Soil Mortar	83
13.2	87	0.020	54	Uniformity coefficient	23	Coarse Sand Ratio	0.05
9.5	87	0.005	49	Coefficient of curvature	0.0	TRB Classification	A - 7 - 6
4.75	85	0.002	46			Unified Classification	CH
2.00	83			Remarks:			

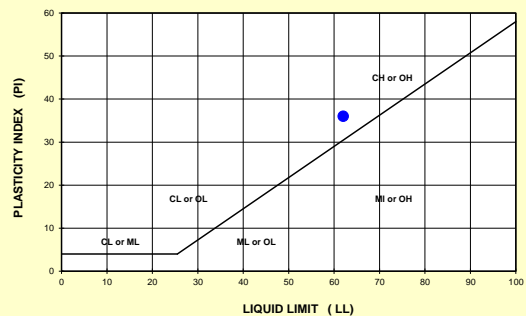
PARTICLE SIZE DISTRIBUTION



POTENTIAL EXPANSIVENESS Van der Merwe's Activity Chart



CASAGRANDE 'A' LINE



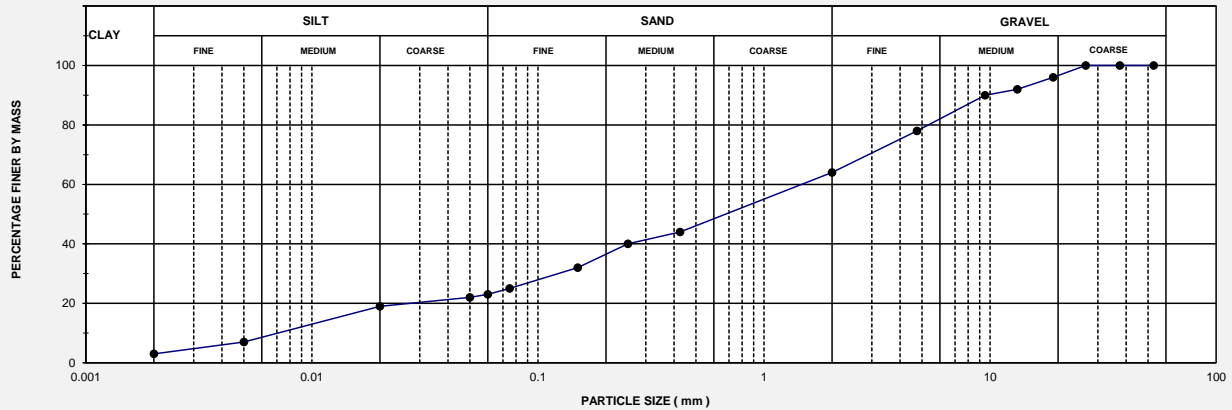


FOUNDATION INDICATOR TEST RESULTS - REP COM 7

Client: Nomfundo Exploration	Source/Location:	Job No: 2016-C-991
Project Name: Rand Water Pipeline Project in Vereenig. Layer:		Sample No: 6/7728
Project No: 0	Lane:	Date: 02/09/2016
Hole/TP No: TP4	Stabilizing Agent:	Test Method: TMH1 A1, A5 & ASTM D422
Depth (m): 0.1 - 1.0	Section:	Client Ref No:
Description: Grey speckled white silty sand ASH (Fill)	Chainage:	GPS X:
Additional Info:	Offset:	GPS Y:

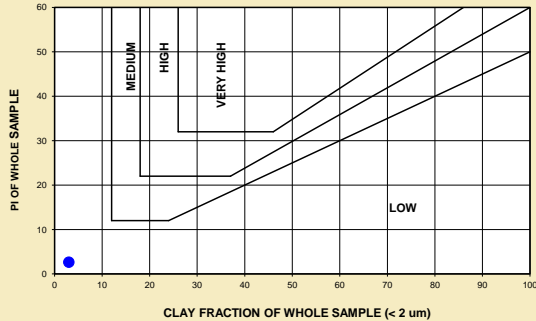
SIEVE ANALYSIS				ATTERBERG LIMITS		SOIL CLASSIFICATION	
Sieve (mm)	% Passing	Sieve (mm)	% Passing				
75.0	100	0.425	44	Liquid Limit (%)	36	% Gravel	36
63.0	100	0.250	40	Plastic Limit (%)	30	% Sand	41
53.0	100	0.150	32	Plasticity Index (%)	6	% Silt	20
37.5	100	0.075	25	Weighted PI (%)	2.6	% Clay	3
26.5	100	0.060	23	Linear Shrinkage (%)	2.0	Activity	2.0
19.0	96	0.050	22	Grading Modulus	1.67	% Soil Mortar	64
13.2	92	0.020	19	Uniformity coefficient	157	Coarse Sand Ratio	0.31
9.5	90	0.005	7	Coefficient of curvature	0.9	TRB Classification	A - 1 - b
4.75	78	0.002	3			Unified Classification	SM
2.00	64			Remarks:			

PARTICLE SIZE DISTRIBUTION

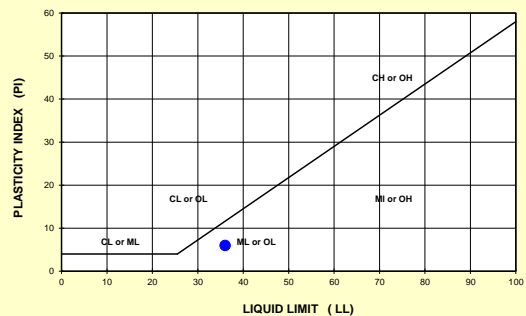


POTENTIAL EXPANSIVENESS

Van der Merwe's Activity Chart



CASAGRANDE 'A' LINE

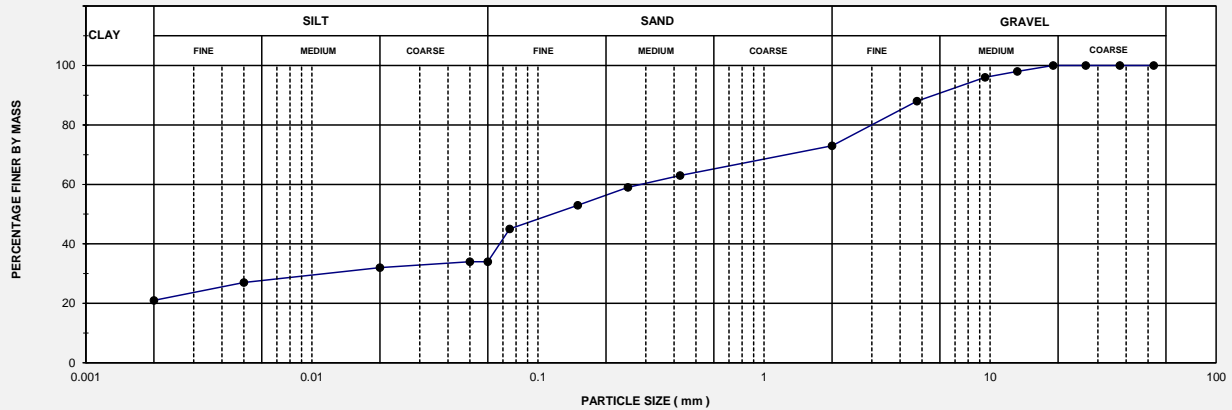


FOUNDATION INDICATOR TEST RESULTS - REP COM 7

Client: Nomfundo Exploration	Source/Location:	Job No: 2016-C-991
Project Name: Rand Water Pipeline Project in Vereenig. Layer:		Sample No: 6/7729
Project No: 0	Lane:	Date: 02/09/2016
Hole/TP No: TP5	Stabilizing Agent:	Test Method: TMH1 A1, A5 & ASTM D422
Depth (m): 2.2 - 3.0	Section:	Client Ref No:
Description: Red blotched black sandy silty clay (Resi Chainage:		GPS X:
Additional Info:	Offset:	GPS Y:

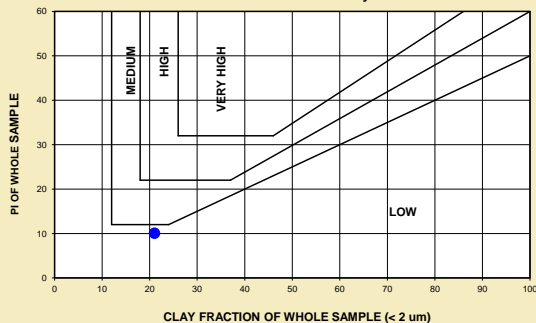
SIEVE ANALYSIS				ATTERBERG LIMITS		SOIL CLASSIFICATION	
Sieve (mm)	% Passing	Sieve (mm)	% Passing				
75.0	100	0.425	63	Liquid Limit (%)	35	% Gravel	27
63.0	100	0.250	59	Plastic Limit (%)	19	% Sand	39
53.0	100	0.150	53	Plasticity Index (%)	16	% Silt	13
37.5	100	0.075	45	Weighted PI (%)	10.1	% Clay	21
26.5	100	0.060	34	Linear Shrinkage (%)	8.5	Activity	0.8
19.0	100	0.050	34	Grading Modulus	1.19	% Soil Mortar	73
13.2	98	0.020	32	Uniformity coefficient	147	Coarse Sand Ratio	0.14
9.5	96	0.005	27	Coefficient of curvature	0.4	TRB Classification	A - 6
4.75	88	0.002	21			Unified Classification	SC
2.00	73			Remarks:			

PARTICLE SIZE DISTRIBUTION

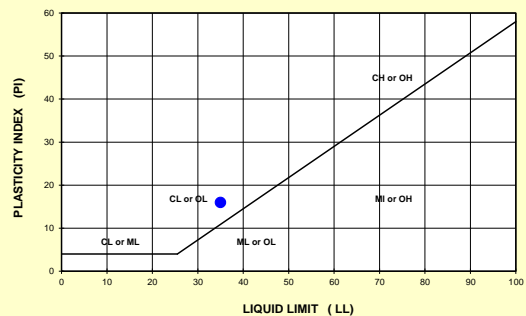


POTENTIAL EXPANSIVENESS

Van der Merwe's Activity Chart



CASAGRANDE 'A' LINE



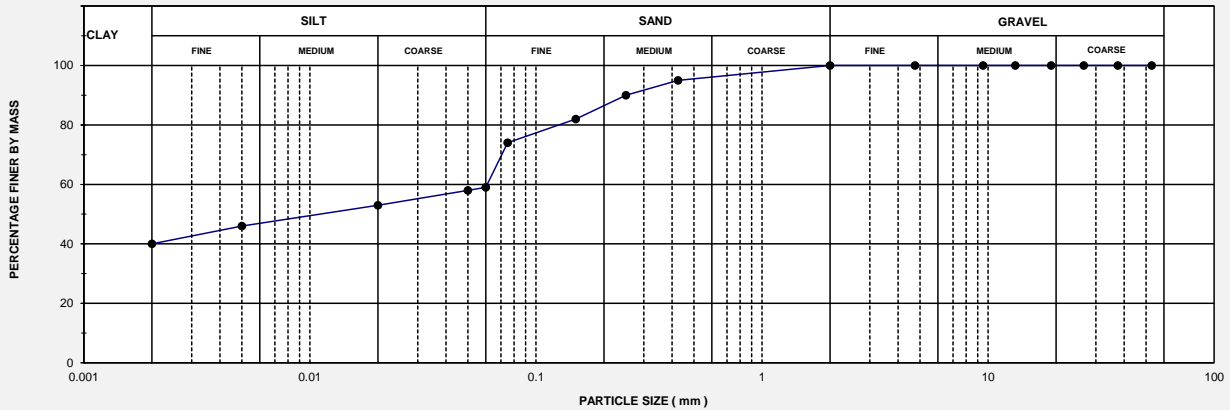


FOUNDATION INDICATOR TEST RESULTS - REP COM 7

Client:	Nomfundo Exploration	Source/Location:	Job No:	2016-C-991
Project Name:	Rand Water Pipeline Project in Vereenig. Layer:	Sample No:	6/7730	
Project No:	0	Lane:	Date:	02/09/2016
Hole/TP No:	TP7	Stabilizing Agent:	Test Method:	TMH1 A1, A5 & ASTM D422
Depth (m):	0.2 - 1.0	Section:	Client Ref No:	
Description:	Black speckled khakhi brown sandy silty	Chainage:	GPS X:	
Additional Info:		Offset:	GPS Y:	

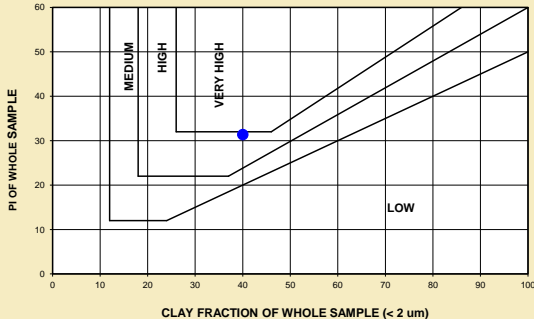
SIEVE ANALYSIS				ATTERBERG LIMITS		SOIL CLASSIFICATION	
Sieve (mm)	% Passing	Sieve (mm)	% Passing				
75.0	100	0.425	95	Liquid Limit (%)	54	% Gravel	0
63.0	100	0.250	90	Plastic Limit (%)	21	% Sand	41
53.0	100	0.150	82	Plasticity Index (%)	33	% Silt	19
37.5	100	0.075	74	Weighted PI (%)	31.4	% Clay	40
26.5	100	0.060	59	Linear Shrinkage (%)	13.0	Activity	0.8
19.0	100	0.050	58	Grading Modulus	0.31	% Soil Mortar	100
13.2	100	0.020	53	Uniformity coefficient	31	Coarse Sand Ratio	0.05
9.5	100	0.005	46	Coefficient of curvature	0.0	TRB Classification	A - 7 - 6
4.75	100	0.002	40			Unified Classification	CH
2.00	100			Remarks:			

PARTICLE SIZE DISTRIBUTION

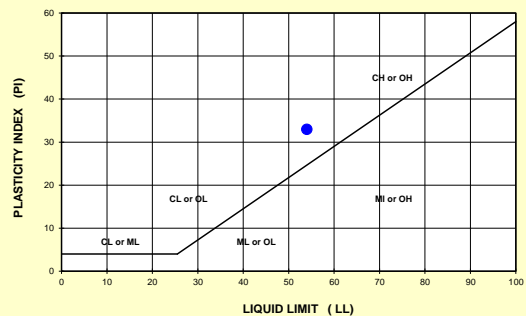


POTENTIAL EXPANSIVENESS

Van der Merwe's Activity Chart



CASAGRANDE 'A' LINE



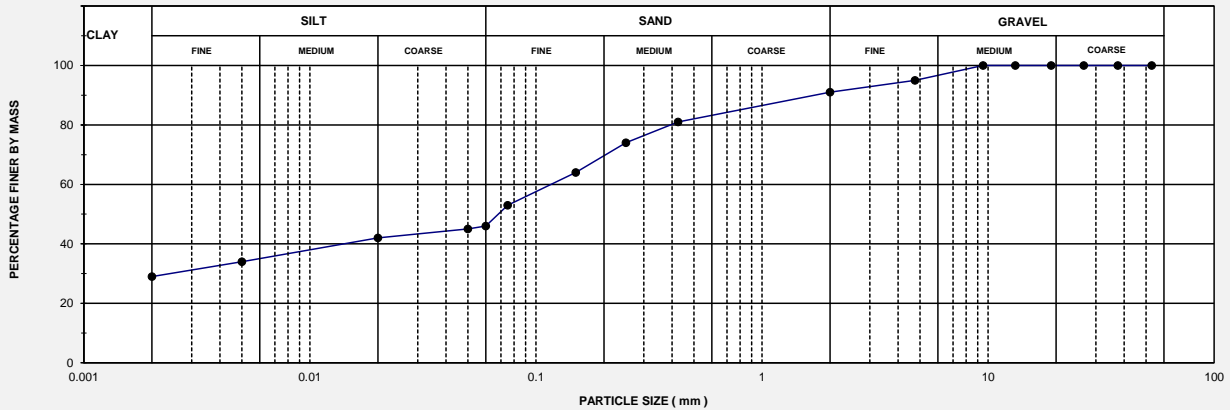


FOUNDATION INDICATOR TEST RESULTS - REP COM 7

Client:	Nomfundo Exploration	Source/Location:	Job No:	2016-C-991
Project Name:	Rand Water Pipeline Project in Vereenig. Layer:	Sample No:	6/7731	
Project No:	0	Lane:	Date:	02/09/2016
Hole/TP No:	TP10	Stabilizing Agent:	Test Method:	TMH1 A1, A5 & ASTM D422
Depth (m):	0.2 - 1.2	Section:	Client Ref No:	
Description:	Orange yellow speckles silty sand ferric Chainage:	GPS X:	GPS Y:	
Additional Info:	Offset:			

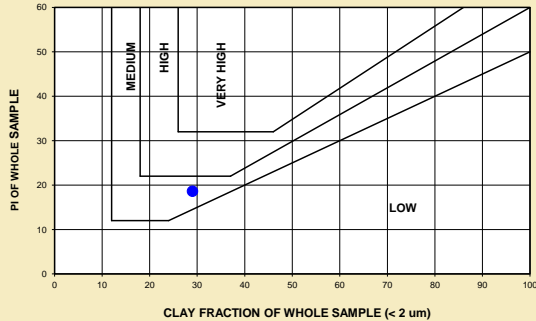
SIEVE ANALYSIS				ATTERBERG LIMITS		SOIL CLASSIFICATION	
Sieve (mm)	% Passing	Sieve (mm)	% Passing				
75.0	100	0.425	81	Liquid Limit (%)	41	% Gravel	9
63.0	100	0.250	74	Plastic Limit (%)	18	% Sand	45
53.0	100	0.150	64	Plasticity Index (%)	23	% Silt	17
37.5	100	0.075	53	Weighted PI (%)	18.6	% Clay	29
26.5	100	0.060	46	Linear Shrinkage (%)	10.0	Activity	0.8
19.0	100	0.050	45	Grading Modulus	0.75	% Soil Mortar	91
13.2	100	0.020	42	Uniformity coefficient	61	Coarse Sand Ratio	0.11
9.5	100	0.005	34	Coefficient of curvature	0.0	TRB Classification	A - 7 - 6
4.75	95	0.002	29			Unified Classification	CL
2.00	91			Remarks:			

PARTICLE SIZE DISTRIBUTION

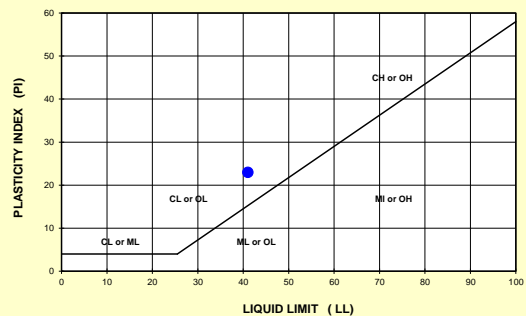


POTENTIAL EXPANSIVENESS

Van der Merwe's Activity Chart



CASAGRANDE 'A' LINE



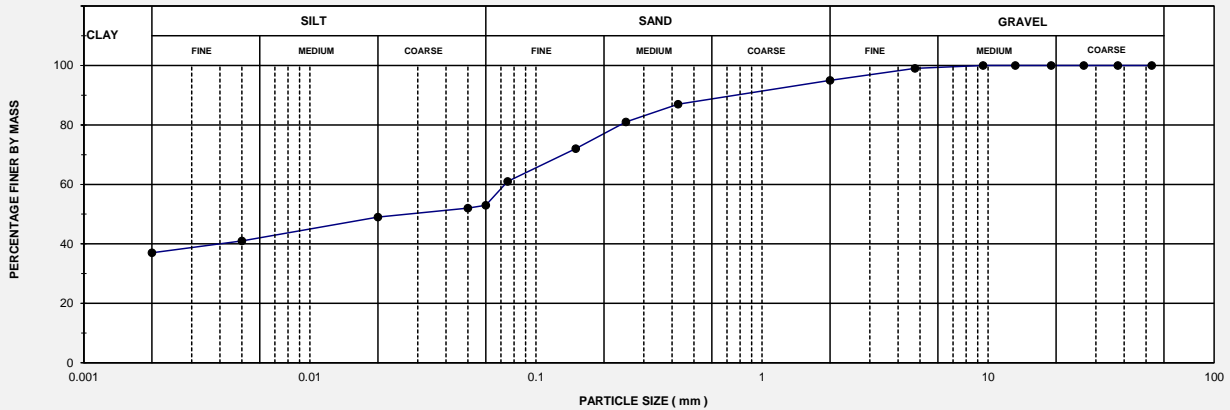


FOUNDATION INDICATOR TEST RESULTS - REP COM 7

Client: Nomfundo Exploration	Source/Location:	Job No: 2016-C-991
Project Name: Rand Water Pipeline Project in Vereenig. Layer:		Sample No: 6/7732
Project No: 0	Lane:	Date: 02/09/2016
Hole/TP No: TP10	Stabilizing Agent:	Test Method: TMH1 A1, A5 & ASTM D422
Depth (m): 1.2 - 3.0	Section:	Client Ref No:
Description: Khakhi blotched calcrete white mottled g	Chainage:	GPS X:
Additional Info:	Offset:	GPS Y:

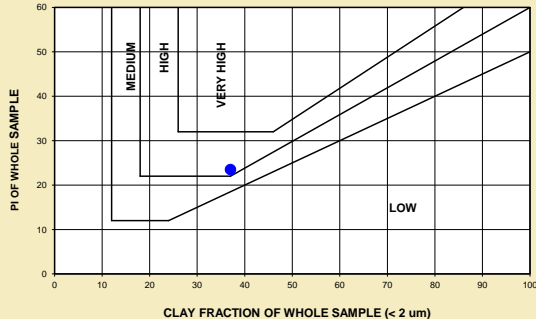
SIEVE ANALYSIS				ATTERBERG LIMITS			SOIL CLASSIFICATION	
Sieve (mm)	% Passing	Sieve (mm)	% Passing					
75.0	100	0.425	87	Liquid Limit (%)	45	% Gravel	5	
63.0	100	0.250	81	Plastic Limit (%)	18	% Sand	42	
53.0	100	0.150	72	Plasticity Index (%)	27	% Silt	16	
37.5	100	0.075	61	Weighted PI (%)	23.5	% Clay	37	
26.5	100	0.060	53	Linear Shrinkage (%)	11.0	Activity	0.7	
19.0	100	0.050	52	Grading Modulus	0.57	% Soil Mortar	95	
13.2	100	0.020	49	Uniformity coefficient	37	Coarse Sand Ratio	0.08	
9.5	100	0.005	41	Coefficient of curvature	0.0	TRB Classification	A - 7 - 6	
4.75	99	0.002	37			Unified Classification	CL	
2.00	95			Remarks:				

PARTICLE SIZE DISTRIBUTION

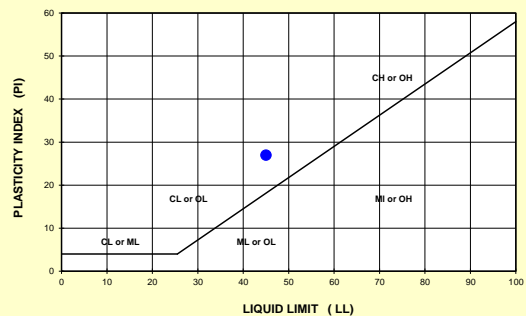


POTENTIAL EXPANSIVENESS

Van der Merwe's Activity Chart



CASAGRANDE 'A' LINE



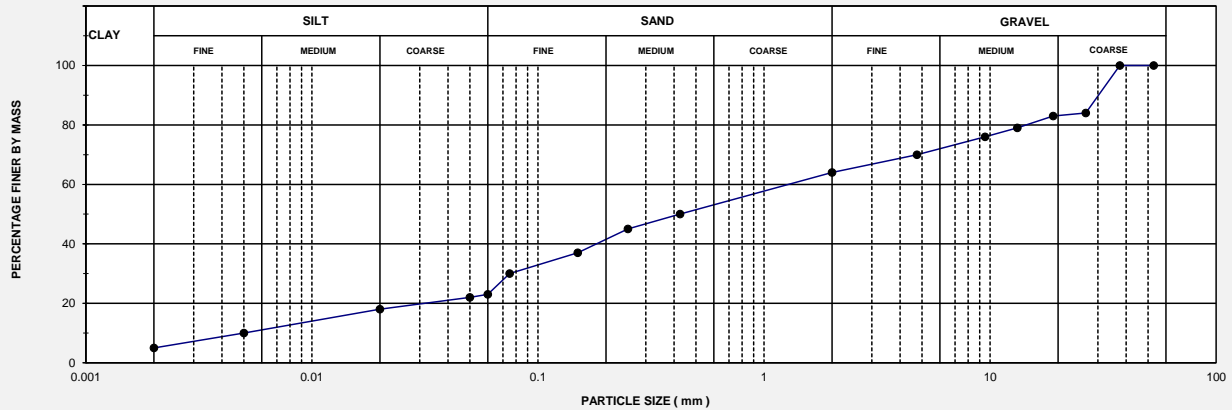


FOUNDATION INDICATOR TEST RESULTS - REP COM 7

Client:	Nomfundo Exploration	Source/Location:		Job No:	2016-C-991
Project Name:	Rand Water Pipeline Project in Vereenig.	Layer:		Sample No:	6/7733
Project No:	0	Lane:		Date:	02/09/2016
Hole/TP No:	TP13	Stabilizing Agent:		Test Method:	TMH1 A1, A5 & ASTM D422
Depth (m):	0.1 - 0.6	Section:		Client Ref No:	
Description:	Dark grey silty sand ASH (Fill)	Chainage:		GPS X:	
Additional Info:		Offset:		GPS Y:	

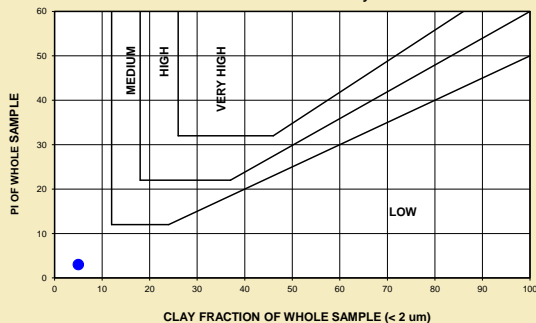
SIEVE ANALYSIS				ATTERBERG LIMITS		SOIL CLASSIFICATION	
Sieve (mm)	% Passing	Sieve (mm)	% Passing				
75.0	100	0.425	50	Liquid Limit (%)	37	% Gravel	36
63.0	100	0.250	45	Plastic Limit (%)	31	% Sand	41
53.0	100	0.150	37	Plasticity Index (%)	6	% Silt	18
37.5	100	0.075	30	Weighted PI (%)	3.0	% Clay	5
26.5	84	0.060	23	Linear Shrinkage (%)	3.5	Activity	1.2
19.0	83	0.050	22	Grading Modulus	1.56	% Soil Mortar	64
13.2	79	0.020	18	Uniformity coefficient	221	Coarse Sand Ratio	0.22
9.5	76	0.005	10	Coefficient of curvature	0.5	TRB Classification	A - 2 - 4
4.75	70	0.002	5			Unified Classification	SM
2.00	64			Remarks:			

PARTICLE SIZE DISTRIBUTION

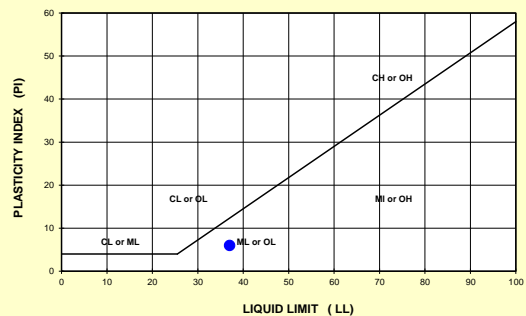


POTENTIAL EXPANSIVENESS

Van der Merwe's Activity Chart



CASAGRANDE 'A' LINE



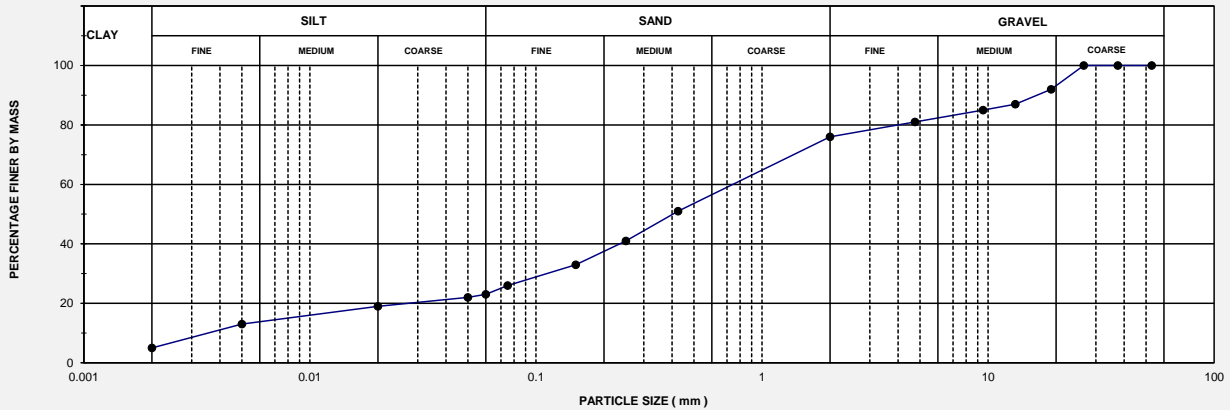


FOUNDATION INDICATOR TEST RESULTS - REP COM 7

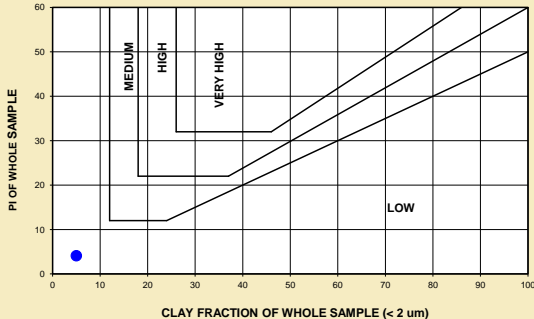
Client: Nomfundo Exploration	Source/Location:	Job No: 2016-C-991
Project Name: Rand Water Pipeline Project in Vereenig.	Layer:	Sample No: 6/7734
Project No: 0	Lane:	Date: 02/09/2016
Hole/TP No: TP13	Stabilizing Agent:	Test Method: TMH1 A1, A5 & ASTM D422
Depth (m): 0.6 - 1.0	Section:	Client Ref No:
Description: White speckled grey ASH (Fill)	Chainage:	GPS X:
Additional Info:	Offset:	GPS Y:

SIEVE ANALYSIS				ATTERBERG LIMITS			SOIL CLASSIFICATION	
Sieve (mm)	% Passing	Sieve (mm)	% Passing					
75.0	100	0.425	51	Liquid Limit (%)	33	% Gravel	24	
63.0	100	0.250	41	Plastic Limit (%)	25	% Sand	53	
53.0	100	0.150	33	Plasticity Index (%)	8	% Silt	18	
37.5	100	0.075	26	Weighted PI (%)	4.1	% Clay	5	
26.5	100	0.060	23	Linear Shrinkage (%)	3.5	Activity	1.6	
19.0	92	0.050	22	Grading Modulus	1.47	% Soil Mortar	76	
13.2	87	0.020	19	Uniformity coefficient	256	Coarse Sand Ratio	0.33	
9.5	85	0.005	13	Coefficient of curvature	3.7	TRB Classification	A - 2 - 4	
4.75	81	0.002	5			Unified Classification	SM	
2.00	76			Remarks:				

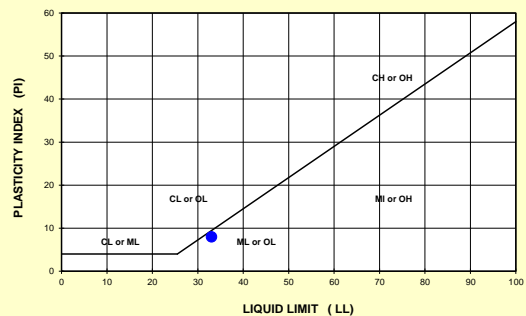
PARTICLE SIZE DISTRIBUTION



POTENTIAL EXPANSIVENESS Van der Merwe's Activity Chart



CASAGRANDE 'A' LINE

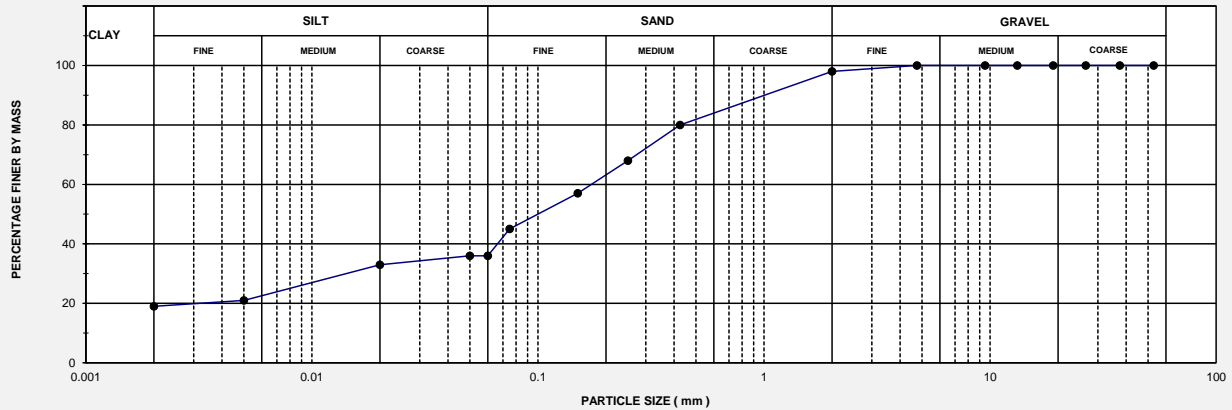


FOUNDATION INDICATOR TEST RESULTS - REP COM 7

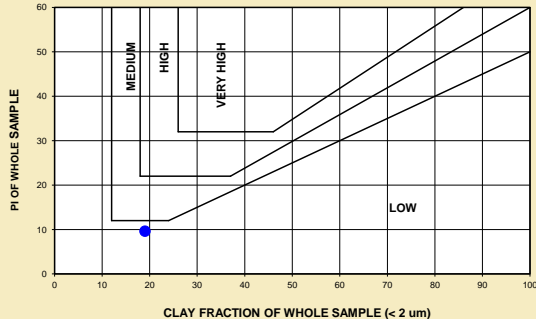
Client:	Nomfundo Exploration	Source/Location:		Job No:	2016-C-991
Project Name:	Rand Water Pipeline Project in Vereenig. Layer:			Sample No:	6/7735
Project No:	0	Lane:		Date:	02/09/2016
Hole/TP No:	TP13	Stabilizing Agent:		Test Method:	TMH1 A1, A5 & ASTM D422
Depth (m):	1.0 - 1.8	Section:		Client Ref No:	
Description:	Dark brown speckled yellow clayey silty : Chainage:			GPS X:	
Additional Info:		Offset:		GPS Y:	

SIEVE ANALYSIS				ATTERBERG LIMITS		SOIL CLASSIFICATION	
Sieve (mm)	% Passing	Sieve (mm)	% Passing				
75.0	100	0.425	80	Liquid Limit (%)	26	% Gravel	2
63.0	100	0.250	68	Plastic Limit (%)	14	% Sand	62
53.0	100	0.150	57	Plasticity Index (%)	12	% Silt	17
37.5	100	0.075	45	Weighted PI (%)	9.6	% Clay	19
26.5	100	0.060	36	Linear Shrinkage (%)	5.5	Activity	0.6
19.0	100	0.050	36	Grading Modulus	0.77	% Soil Mortar	98
13.2	100	0.020	33	Uniformity coefficient	89	Coarse Sand Ratio	0.18
9.5	100	0.005	21	Coefficient of curvature	0.9	TRB Classification	A - 6
4.75	100	0.002	19			Unified Classification	SC
2.00	98			Remarks:			

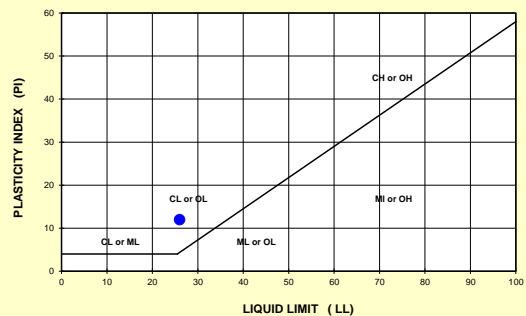
PARTICLE SIZE DISTRIBUTION



POTENTIAL EXPANSIVENESS Van der Merwe's Activity Chart



CASAGRANDE 'A' LINE



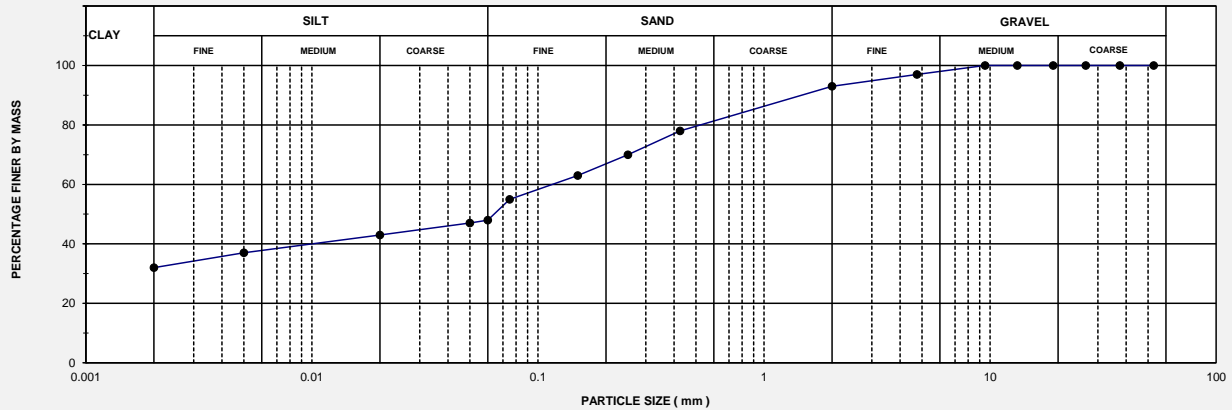


FOUNDATION INDICATOR TEST RESULTS - REP COM 7

Client:	Nomfundo Exploration	Source/Location:		Job No:	2016-C-991
Project Name:	Rand Water Pipeline Project in Vereenig. Layer:			Sample No:	6/7736
Project No:	0	Lane:		Date:	02/09/2016
Hole/TP No:	TP13	Stabilizing Agent:		Test Method:	TMH1 A1, A5 & ASTM D422
Depth (m):	1.8 - 3.0	Section:		Client Ref No:	
Description:	Yellow speckled black grey clayey silty s:Chainage:			GPS X:	
Additional Info:		Offset:		GPS Y:	

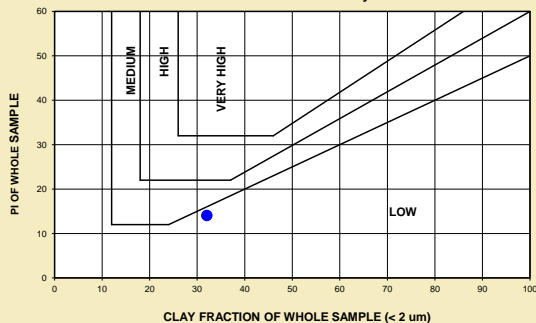
SIEVE ANALYSIS				ATTERBERG LIMITS		SOIL CLASSIFICATION	
Sieve (mm)	% Passing	Sieve (mm)	% Passing				
75.0	100	0.425	78	Liquid Limit (%)	36	% Gravel	7
63.0	100	0.250	70	Plastic Limit (%)	18	% Sand	45
53.0	100	0.150	63	Plasticity Index (%)	18	% Silt	16
37.5	100	0.075	55	Weighted PI (%)	14.0	% Clay	32
26.5	100	0.060	48	Linear Shrinkage (%)	8.0	Activity	0.6
19.0	100	0.050	47	Grading Modulus	0.74	% Soil Mortar	93
13.2	100	0.020	43	Uniformity coefficient	61	Coarse Sand Ratio	0.16
9.5	100	0.005	37	Coefficient of curvature	0.0	TRB Classification	A - 6
4.75	97	0.002	32			Unified Classification	CL
2.00	93			Remarks:			

PARTICLE SIZE DISTRIBUTION

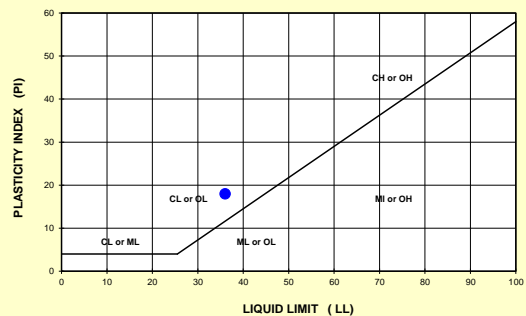


POTENTIAL EXPANSIVENESS

Van der Merwe's Activity Chart



CASAGRANDE 'A' LINE



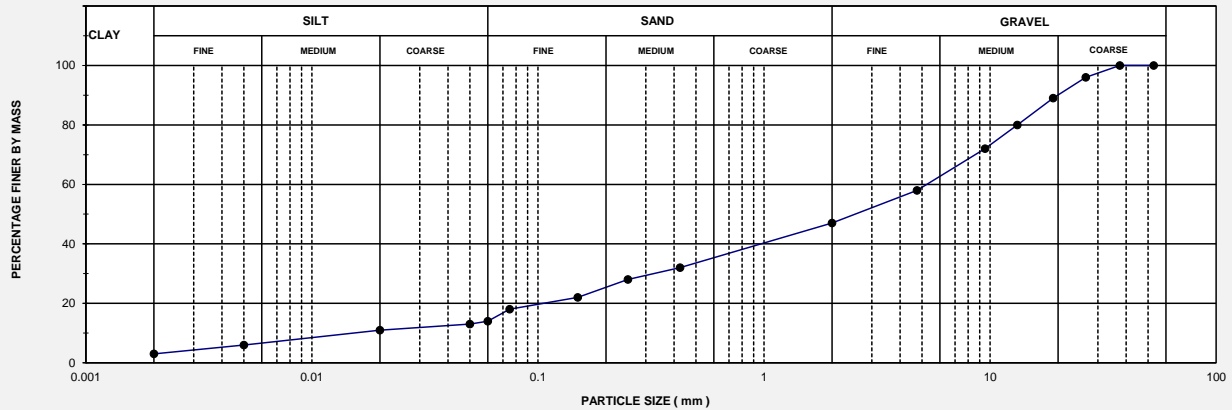


FOUNDATION INDICATOR TEST RESULTS - REP COM 7

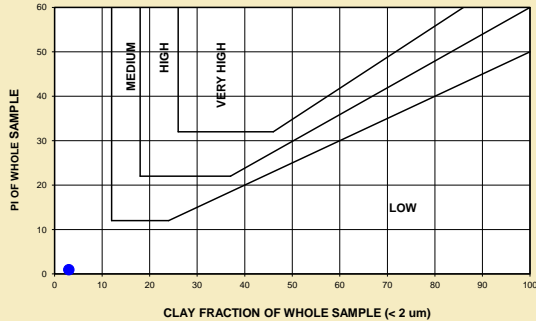
Client: Nomfundo Exploration	Source/Location:	Job No: 2016-C-991
Project Name: Rand Water Pipeline Project in Vereenig.	Layer:	Sample No: 6/7737
Project No: 0	Lane:	Date: 02/09/2016
Hole/TP No: TP14	Stabilizing Agent:	Test Method: TMH1 A1, A5 & ASTM D422
Depth (m): 0.1 - 1.2	Section:	Client Ref No:
Description: Grey speckled white silty sand ASH (Fill)	Chainage:	GPS X:
Additional Info:	Offset:	GPS Y:

SIEVE ANALYSIS				ATTERBERG LIMITS		SOIL CLASSIFICATION	
Sieve (mm)	% Passing	Sieve (mm)	% Passing				
75.0	100	0.425	32	Liquid Limit (%)	38	% Gravel	53
63.0	100	0.250	28	Plastic Limit (%)	35	% Sand	33
53.0	100	0.150	22	Plasticity Index (%)	3	% Silt	11
37.5	100	0.075	18	Weighted PI (%)	1.0	% Clay	3
26.5	96	0.060	14	Linear Shrinkage (%)	1.5	Activity	1.0
19.0	89	0.050	13	Grading Modulus	2.03	% Soil Mortar	47
13.2	80	0.020	11	Uniformity coefficient	286	Coarse Sand Ratio	0.32
9.5	72	0.005	6	Coefficient of curvature	1.1	TRB Classification	A - 1 - b
4.75	58	0.002	3			Unified Classification	GM
2.00	47			Remarks:			

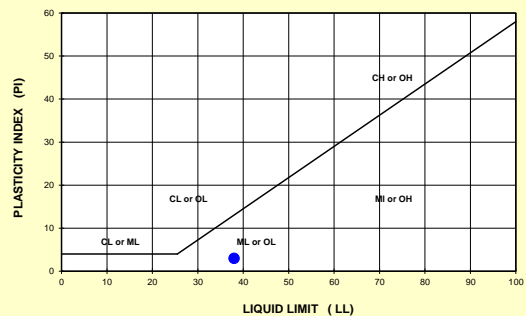
PARTICLE SIZE DISTRIBUTION



POTENTIAL EXPANSIVENESS Van der Merwe's Activity Chart



CASAGRANDE 'A' LINE



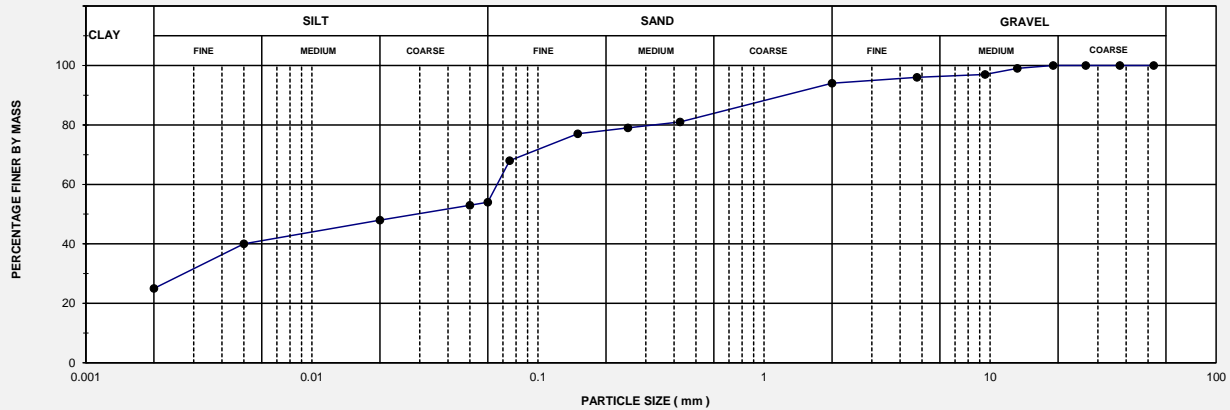


FOUNDATION INDICATOR TEST RESULTS - REP COM 7

Client:	Nomfundo Exploration	Source/Location:		Job No:	2016-C-991
Project Name:	Rand Water Pipeline Project in Vereenig. Layer:			Sample No:	6/7738
Project No:	0	Lane:		Date:	02/09/2016
Hole/TP No:	TP14	Stabilizing Agent:		Test Method:	TMH1 A1, A5 & ASTM D422
Depth (m):	1.8 - 3.0	Section:		Client Ref No:	
Description:	Reddish brown speckled yellow clayey si	Chainage:		GPS X:	
Additional Info:		Offset:		GPS Y:	

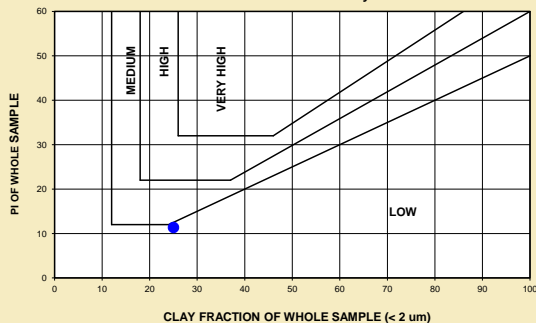
SIEVE ANALYSIS				ATTERBERG LIMITS		SOIL CLASSIFICATION	
Sieve (mm)	% Passing	Sieve (mm)	% Passing				
75.0	100	0.425	81	Liquid Limit (%)	31	% Gravel	6
63.0	100	0.250	79	Plastic Limit (%)	17	% Sand	40
53.0	100	0.150	77	Plasticity Index (%)	14	% Silt	29
37.5	100	0.075	68	Weighted PI (%)	11.3	% Clay	25
26.5	100	0.060	54	Linear Shrinkage (%)	7.0	Activity	0.6
19.0	100	0.050	53	Grading Modulus	0.57	% Soil Mortar	94
13.2	99	0.020	48	Uniformity coefficient	33	Coarse Sand Ratio	0.14
9.5	97	0.005	40	Coefficient of curvature	0.1	TRB Classification	A - 6
4.75	96	0.002	25			Unified Classification	CL
2.00	94			Remarks:			

PARTICLE SIZE DISTRIBUTION

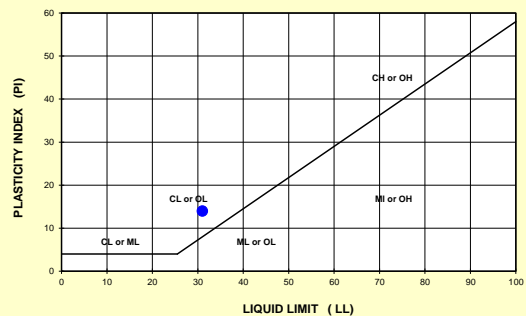


POTENTIAL EXPANSIVENESS

Van der Merwe's Activity Chart



CASAGRANDE 'A' LINE



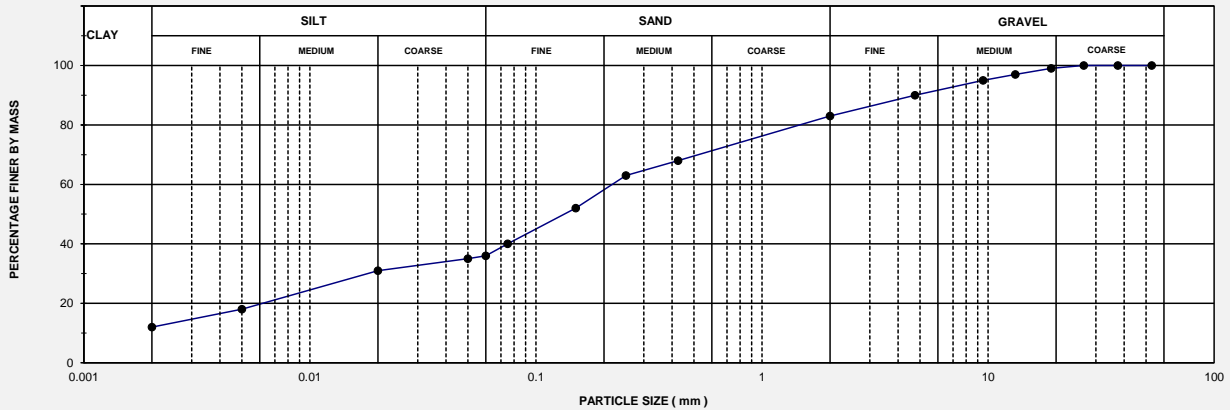


FOUNDATION INDICATOR TEST RESULTS - REP COM 7

Client:	Nomfundo Exploration	Source/Location:		Job No:	2016-C-991
Project Name:	Rand Water Pipeline Project in Vereenig. Layer:			Sample No:	6/7739
Project No:	0	Lane:		Date:	02/09/2016
Hole/TP No:	TP16	Stabilizing Agent:		Test Method:	TMH1 A1, A5 & ASTM D422
Depth (m):	0.2 - 3.0	Section:		Client Ref No:	
Description:	Reddish brown speckled yellow silty sand	Chainage:		GPS X:	
Additional Info:		Offset:		GPS Y:	

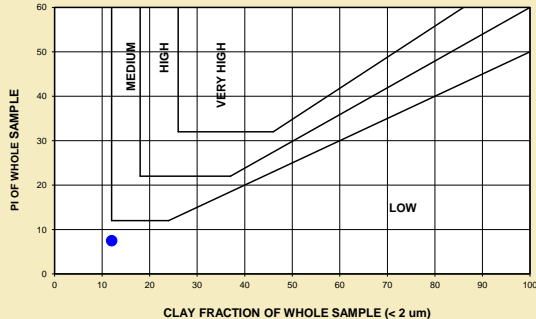
SIEVE ANALYSIS				ATTEBERG LIMITS		SOIL CLASSIFICATION	
Sieve (mm)	% Passing	Sieve (mm)	% Passing				
75.0	100	0.425	68	Liquid Limit (%)	30	% Gravel	17
63.0	100	0.250	63	Plastic Limit (%)	19	% Sand	47
53.0	100	0.150	52	Plasticity Index (%)	11	% Silt	24
37.5	100	0.075	40	Weighted PI (%)	7.5	% Clay	12
26.5	100	0.060	36	Linear Shrinkage (%)	5.5	Activity	0.9
19.0	99	0.050	35	Grading Modulus	1.09	% Soil Mortar	83
13.2	97	0.020	31	Uniformity coefficient	111	Coarse Sand Ratio	0.18
9.5	95	0.005	18	Coefficient of curvature	1.0	TRB Classification	A - 6
4.75	90	0.002	12			Unified Classification	SC
2.00	83			Remarks:			

PARTICLE SIZE DISTRIBUTION

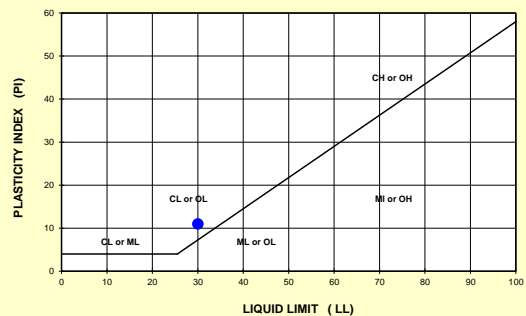


POTENTIAL EXPANSIVENESS

Van der Merwe's Activity Chart



CASAGRANDE 'A' LINE



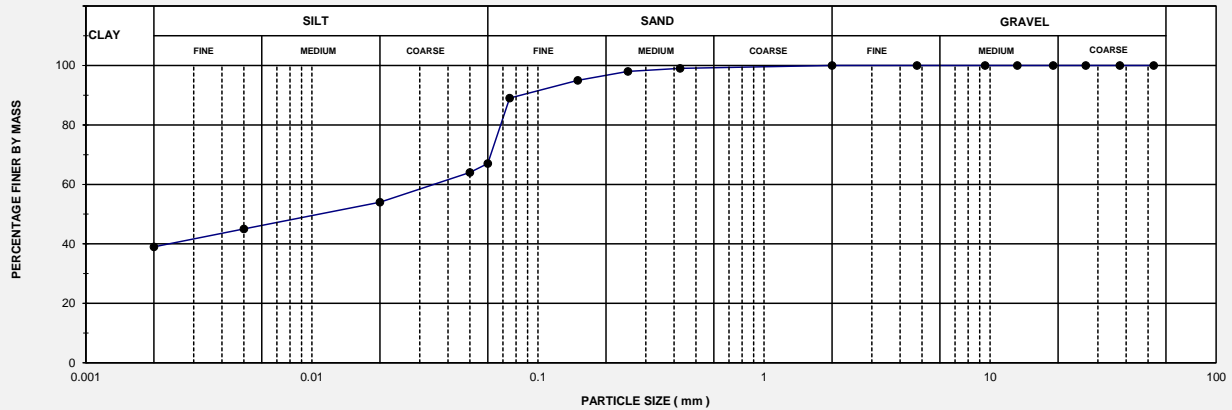


FOUNDATION INDICATOR TEST RESULTS - REP COM 7

Client: Nomfundo Exploration	Source/Location:	Job No: 2016-C-991
Project Name: Rand Water Pipeline Project in Vereenig. Layer:		Sample No: 6/7740
Project No: 0	Lane:	Date: 02/09/2016
Hole/TP No: TP18	Stabilizing Agent:	Test Method: TMH1 A1, A5 & ASTM D422
Depth (m): 2.0 - 3.0	Section:	Client Ref No:
Description: Khakhi blotched calcrete white mottled g	Chainage:	GPS X:
Additional Info:	Offset:	GPS Y:

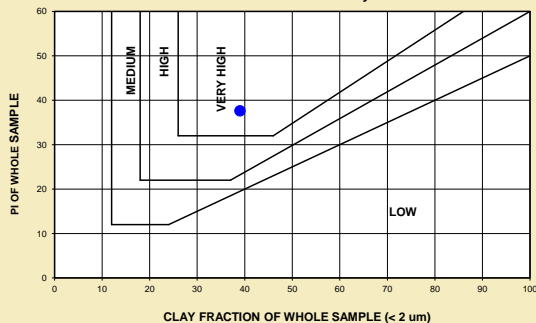
SIEVE ANALYSIS				ATTERBERG LIMITS		SOIL CLASSIFICATION	
Sieve (mm)	% Passing	Sieve (mm)	% Passing				
75.0	100	0.425	99	Liquid Limit (%)	58	% Gravel	0
63.0	100	0.250	98	Plastic Limit (%)	20	% Sand	33
53.0	100	0.150	95	Plasticity Index (%)	38	% Silt	28
37.5	100	0.075	89	Weighted PI (%)	37.6	% Clay	39
26.5	100	0.060	67	Linear Shrinkage (%)	10.5	Activity	1.0
19.0	100	0.050	64	Grading Modulus	0.12	% Soil Mortar	100
13.2	100	0.020	54	Uniformity coefficient	19	Coarse Sand Ratio	0.01
9.5	100	0.005	45	Coefficient of curvature	0.1	TRB Classification	A - 7 - 6
4.75	100	0.002	39			Unified Classification	CH
2.00	100			Remarks:			

PARTICLE SIZE DISTRIBUTION

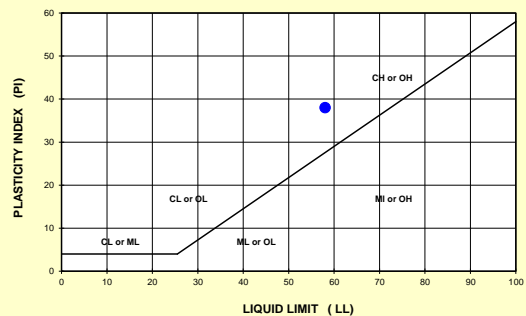


POTENTIAL EXPANSIVENESS

Van der Merwe's Activity Chart



CASAGRANDE 'A' LINE



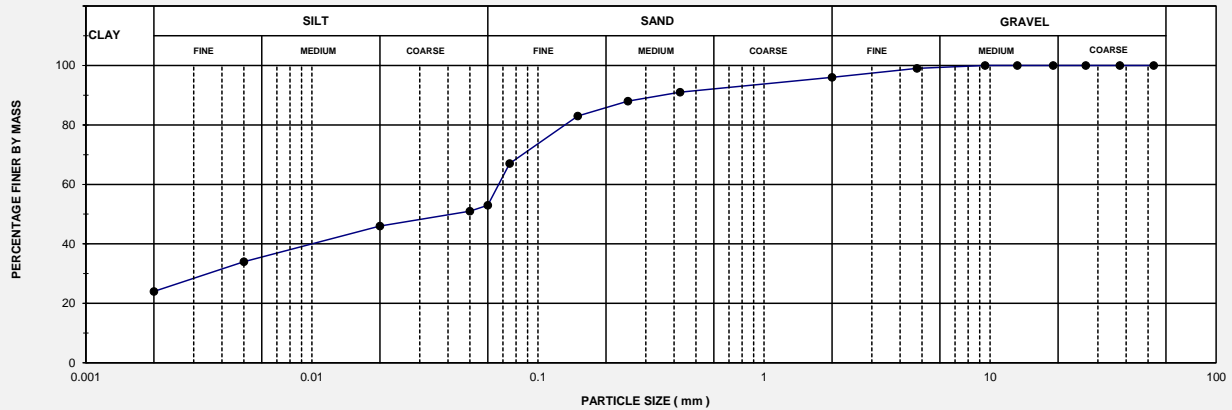


FOUNDATION INDICATOR TEST RESULTS - REP COM 7

Client:	Nomfundo Exploration	Source/Location:		Job No:	2016-C-991
Project Name:	Rand Water Pipeline Project in Vereenig. Layer:			Sample No:	6/7741
Project No:	0	Lane:		Date:	02/09/2016
Hole/TP No:	TP20	Stabilizing Agent:		Test Method:	TMH1 A1, A5 & ASTM D422
Depth (m):	0.8 - 1.8	Section:		Client Ref No:	
Description:	Brown speckled black grey clayey silt sar Chainage:			GPS X:	
Additional Info:		Offset:		GPS Y:	

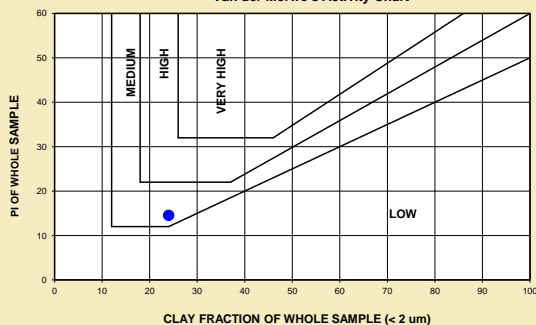
SIEVE ANALYSIS				ATTERBERG LIMITS		SOIL CLASSIFICATION	
Sieve (mm)	% Passing	Sieve (mm)	% Passing				
75.0	100	0.425	91	Liquid Limit (%)	34	% Gravel	4
63.0	100	0.250	88	Plastic Limit (%)	18	% Sand	43
53.0	100	0.150	83	Plasticity Index (%)	16	% Silt	29
37.5	100	0.075	67	Weighted PI (%)	14.6	% Clay	24
26.5	100	0.060	53	Linear Shrinkage (%)	6.0	Activity	0.7
19.0	100	0.050	51	Grading Modulus	0.46	% Soil Mortar	96
13.2	100	0.020	46	Uniformity coefficient	34	Coarse Sand Ratio	0.05
9.5	100	0.005	34	Coefficient of curvature	0.1	TRB Classification	A - 6
4.75	99	0.002	24			Unified Classification	CL
2.00	96			Remarks:			

PARTICLE SIZE DISTRIBUTION

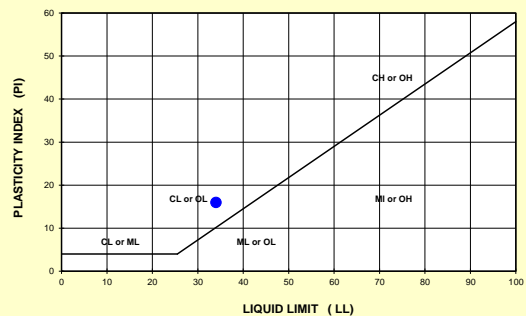


POTENTIAL EXPANSIVENESS

Van der Merwe's Activity Chart



CASAGRANDE 'A' LINE



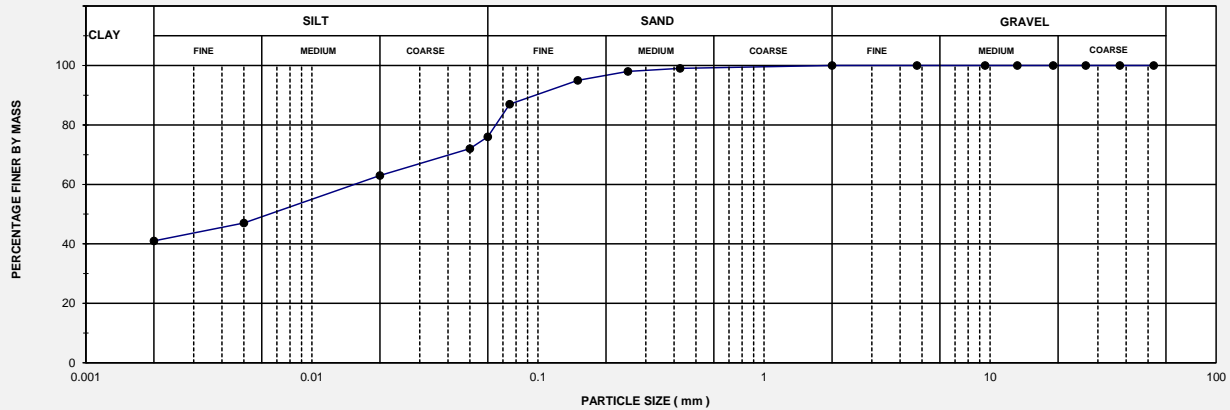


FOUNDATION INDICATOR TEST RESULTS - REP COM 7

Client: Nomfundo Exploration	Source/Location:	Job No: 2016-C-991
Project Name: Rand Water Pipeline Project in Vereenig. Layer:		Sample No: 6/7742
Project No: 0	Lane:	Date: 02/09/2016
Hole/TP No: TP20	Stabilizing Agent:	Test Method: TMH1 A1, A5 & ASTM D422
Depth (m): 1.8 - 3.0	Section:	Client Ref No:
Description: Black sandy silty clay (Alluvium)	Chainage:	GPS X:
Additional Info:	Offset:	GPS Y:

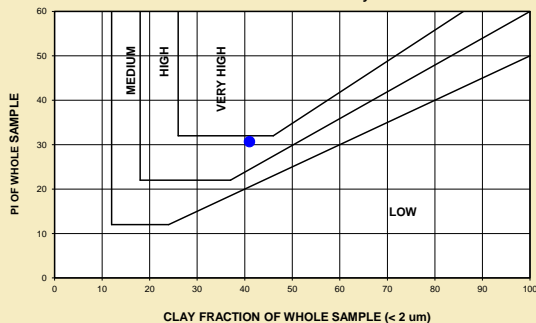
SIEVE ANALYSIS				ATTERBERG LIMITS		SOIL CLASSIFICATION	
Sieve (mm)	% Passing	Sieve (mm)	% Passing				
75.0	100	0.425	99	Liquid Limit (%)	52	% Gravel	0
63.0	100	0.250	98	Plastic Limit (%)	21	% Sand	24
53.0	100	0.150	95	Plasticity Index (%)	31	% Silt	35
37.5	100	0.075	87	Weighted PI (%)	30.7	% Clay	41
26.5	100	0.060	76	Linear Shrinkage (%)	13.0	Activity	0.8
19.0	100	0.050	72	Grading Modulus	0.14	% Soil Mortar	100
13.2	100	0.020	63	Uniformity coefficient	9	Coarse Sand Ratio	0.01
9.5	100	0.005	47	Coefficient of curvature	0.1	TRB Classification	A - 7 - 6
4.75	100	0.002	41			Unified Classification	CH
2.00	100			Remarks:			

PARTICLE SIZE DISTRIBUTION

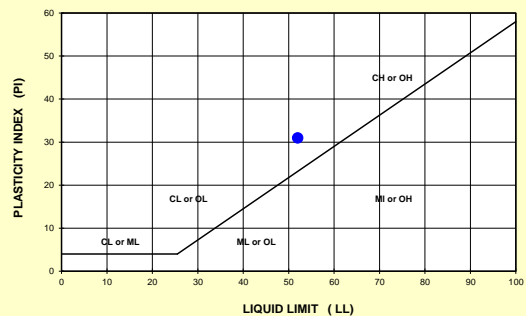


POTENTIAL EXPANSIVENESS

Van der Merwe's Activity Chart



CASAGRANDE 'A' LINE



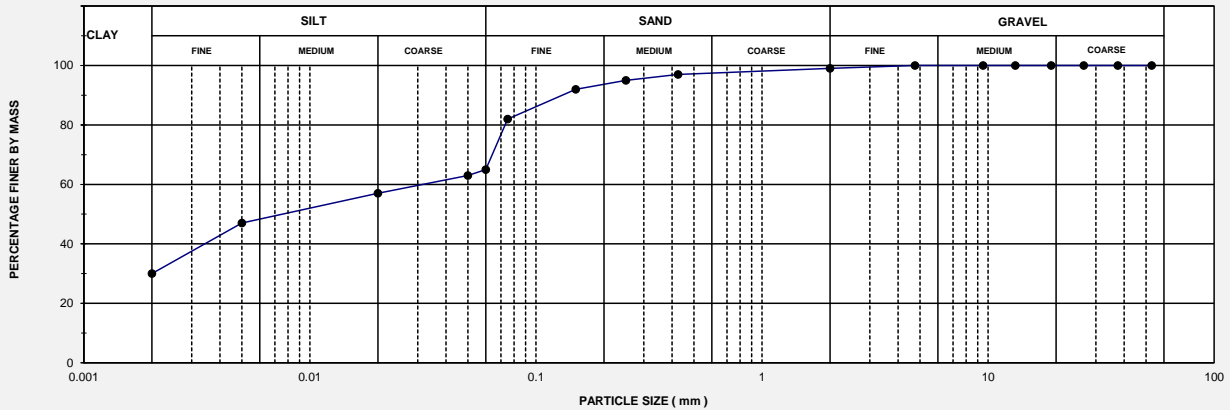


FOUNDATION INDICATOR TEST RESULTS - REP COM 7

Client: Nomfundo Exploration	Source/Location:	Job No: 2016-C-991
Project Name: Rand Water Pipeline Project in Vereenig. Layer:		Sample No: 6/7743
Project No: 0	Lane:	Date: 02/09/2016
Hole/TP No: TP21	Stabilizing Agent:	Test Method: TMH1 A1, A5 & ASTM D422
Depth (m): 0.6 - 1.0	Section:	Client Ref No:
Description: Brown speckled black grey clayey silty ss	Chainage:	GPS X:
Additional Info:	Offset:	GPS Y:

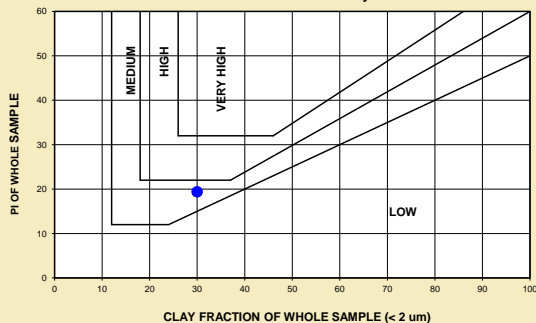
SIEVE ANALYSIS				ATTERBERG LIMITS		SOIL CLASSIFICATION	
Sieve (mm)	% Passing	Sieve (mm)	% Passing				
75.0	100	0.425	97	Liquid Limit (%)	35	% Gravel	1
63.0	100	0.250	95	Plastic Limit (%)	15	% Sand	34
53.0	100	0.150	92	Plasticity Index (%)	20	% Silt	35
37.5	100	0.075	82	Weighted PI (%)	19.4	% Clay	30
26.5	100	0.060	65	Linear Shrinkage (%)	5.5	Activity	0.7
19.0	100	0.050	63	Grading Modulus	0.22	% Soil Mortar	99
13.2	100	0.020	57	Uniformity coefficient	18	Coarse Sand Ratio	0.02
9.5	100	0.005	47	Coefficient of curvature	0.1	TRB Classification	A - 6
4.75	100	0.002	30			Unified Classification	CL
2.00	99			Remarks:			

PARTICLE SIZE DISTRIBUTION

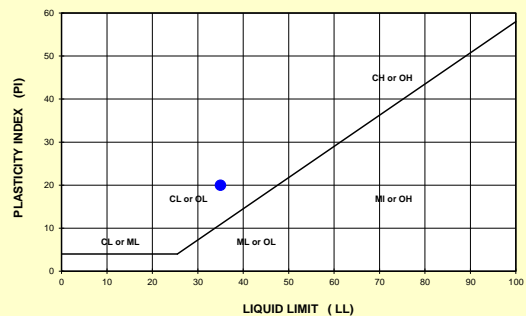


POTENTIAL EXPANSIVENESS

Van der Merwe's Activity Chart



CASAGRANDE 'A' LINE



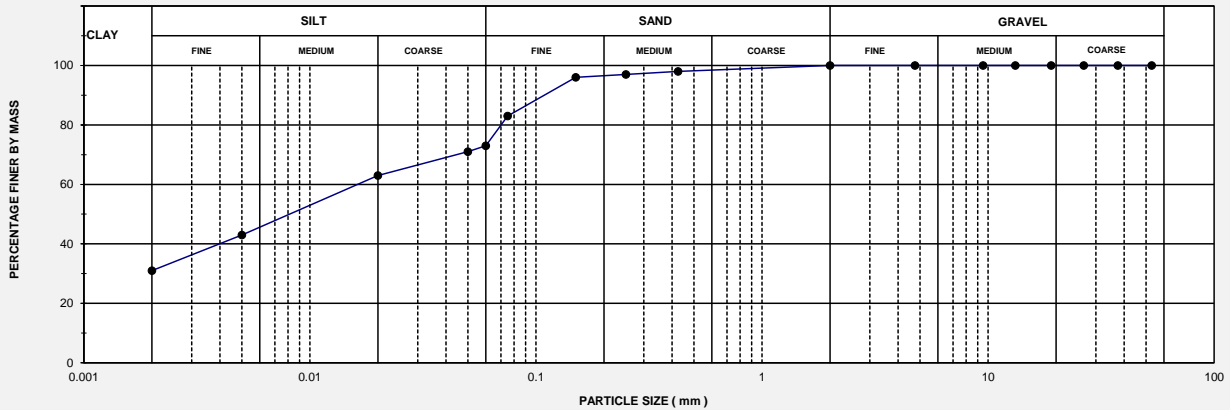


FOUNDATION INDICATOR TEST RESULTS - REP COM 7

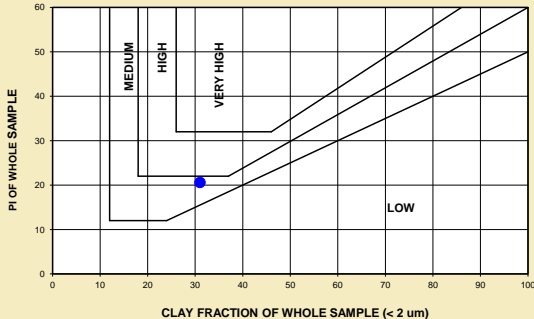
Client: Nomfundo Exploration	Source/Location:	Job No: 2016-C-991
Project Name: Rand Water Pipeline Project in Vereenig. Layer:		Sample No: 6/7744
Project No: 0	Lane:	Date: 02/09/2016
Hole/TP No: TP21	Stabilizing Agent:	Test Method: TMH1 A1, A5 & ASTM D422
Depth (m): 1.8 - 3.0	Section:	Client Ref No:
Description: Khakhi blotched calcrete white mottled g	Chainage:	GPS X:
Additional Info:	Offset:	GPS Y:

SIEVE ANALYSIS				ATTERBERG LIMITS		SOIL CLASSIFICATION	
Sieve (mm)	% Passing	Sieve (mm)	% Passing				
75.0	100	0.425	98	Liquid Limit (%)	38	% Gravel	0
63.0	100	0.250	97	Plastic Limit (%)	17	% Sand	27
53.0	100	0.150	96	Plasticity Index (%)	21	% Silt	42
37.5	100	0.075	83	Weighted PI (%)	20.6	% Clay	31
26.5	100	0.060	73	Linear Shrinkage (%)	9.5	Activity	0.7
19.0	100	0.050	71	Grading Modulus	0.19	% Soil Mortar	100
13.2	100	0.020	63	Uniformity coefficient	9	Coarse Sand Ratio	0.02
9.5	100	0.005	43	Coefficient of curvature	0.1	TRB Classification	A - 6
4.75	100	0.002	31			Unified Classification	CL
2.00	100			Remarks:			

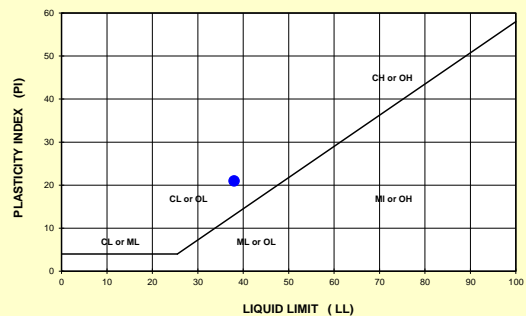
PARTICLE SIZE DISTRIBUTION



POTENTIAL EXPANSIVENESS Van der Merwe's Activity Chart



CASAGRANDE 'A' LINE

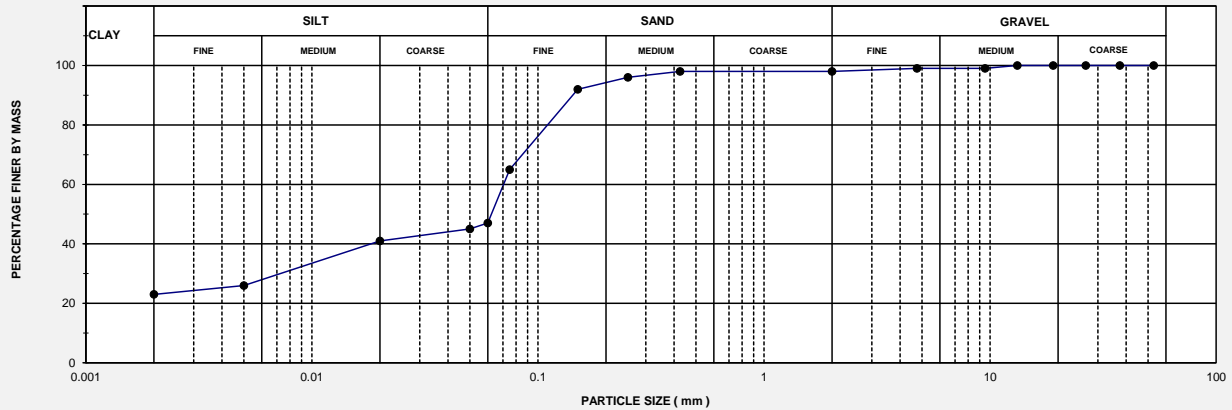


FOUNDATION INDICATOR TEST RESULTS - REP COM 7

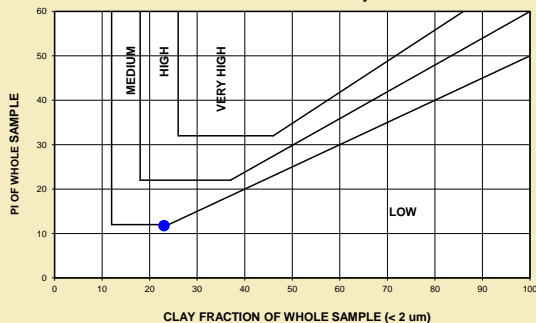
Client:	Nomfundo Exploration	Source/Location:		Job No:	2016-C-991
Project Name:	Rand Water Pipeline Project in Vereenig. Layer:			Sample No:	6/7745
Project No:	0	Lane:		Date:	02/09/2016
Hole/TP No:	TP23	Stabilizing Agent:		Test Method:	TMH1 A1, A5 & ASTM D422
Depth (m):	1.0 - 3.0	Section:		Client Ref No:	
Description:	Khakhi blotched calcrete white mottled g	Chainage:		GPS X:	
Additional Info:		Offset:		GPS Y:	

SIEVE ANALYSIS				ATTERBERG LIMITS		SOIL CLASSIFICATION	
Sieve (mm)	% Passing	Sieve (mm)	% Passing				
75.0	100	0.425	98	Liquid Limit (%)	26	% Gravel	2
63.0	100	0.250	96	Plastic Limit (%)	14	% Sand	51
53.0	100	0.150	92	Plasticity Index (%)	12	% Silt	24
37.5	100	0.075	65	Weighted PI (%)	11.8	% Clay	23
26.5	100	0.060	47	Linear Shrinkage (%)	5.5	Activity	0.5
19.0	100	0.050	45	Grading Modulus	0.39	% Soil Mortar	98
13.2	100	0.020	41	Uniformity coefficient	35	Coarse Sand Ratio	0.00
9.5	99	0.005	26	Coefficient of curvature	0.9	TRB Classification	A - 6
4.75	99	0.002	23			Unified Classification	CL
2.00	98			Remarks:			

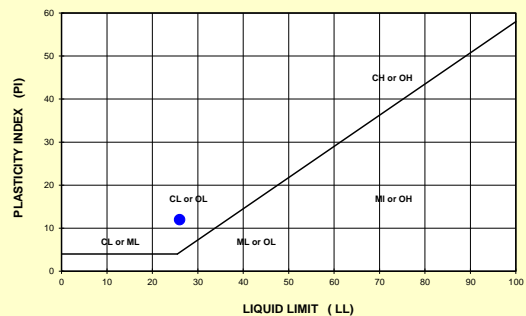
PARTICLE SIZE DISTRIBUTION



POTENTIAL EXPANSIVENESS Van der Merwe's Activity Chart



CASAGRANDE 'A' LINE



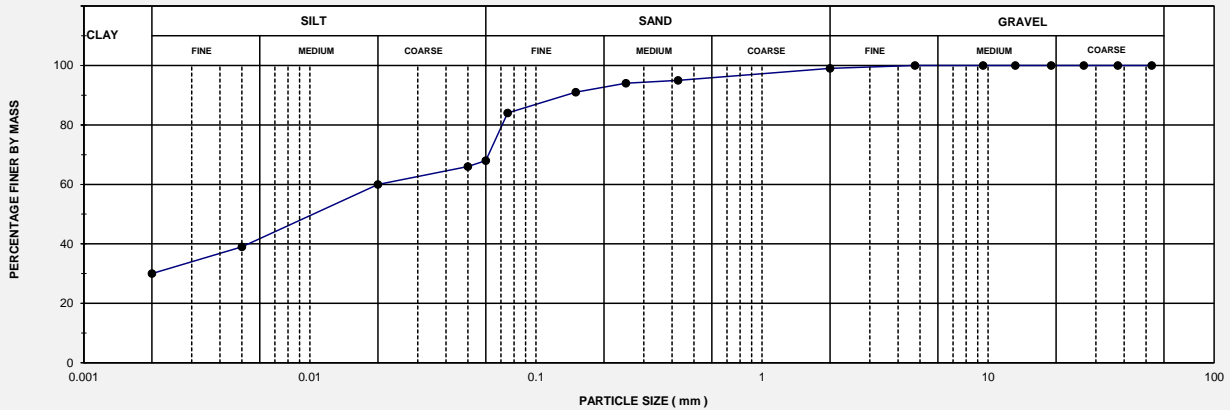


FOUNDATION INDICATOR TEST RESULTS - REP COM 7

Client:	Nomfundo Exploration	Source/Location:		Job No:	2016-C-991
Project Name:	Rand Water Pipeline Project in Vereenig. Layer:			Sample No:	6/7746
Project No:	0	Lane:		Date:	02/09/2016
Hole/TP No:	TP26	Stabilizing Agent:		Test Method:	TMH1 A1, A5 & ASTM D422
Depth (m):	1.0 - 3.0	Section:		Client Ref No:	
Description:	Khakhi blotched calcrete white mottled g	Chainage:		GPS X:	
Additional Info:		Offset:		GPS Y:	

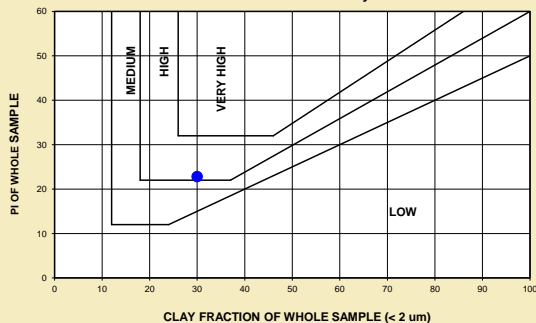
SIEVE ANALYSIS				ATTERBERG LIMITS		SOIL CLASSIFICATION	
Sieve (mm)	% Passing	Sieve (mm)	% Passing				
75.0	100	0.425	95	Liquid Limit (%)	41	% Gravel	1
63.0	100	0.250	94	Plastic Limit (%)	17	% Sand	31
53.0	100	0.150	91	Plasticity Index (%)	24	% Silt	38
37.5	100	0.075	84	Weighted PI (%)	22.8	% Clay	30
26.5	100	0.060	68	Linear Shrinkage (%)	9.5	Activity	0.8
19.0	100	0.050	66	Grading Modulus	0.22	% Soil Mortar	99
13.2	100	0.020	60	Uniformity coefficient	10	Coarse Sand Ratio	0.04
9.5	100	0.005	39	Coefficient of curvature	0.1	TRB Classification	A - 7 - 6
4.75	100	0.002	30			Unified Classification	CL
2.00	99			Remarks:			

PARTICLE SIZE DISTRIBUTION

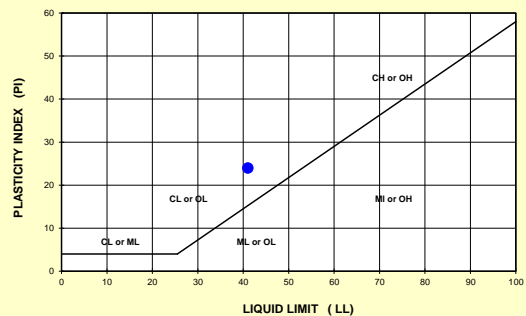


POTENTIAL EXPANSIVENESS

Van der Merwe's Activity Chart



CASAGRANDE 'A' LINE



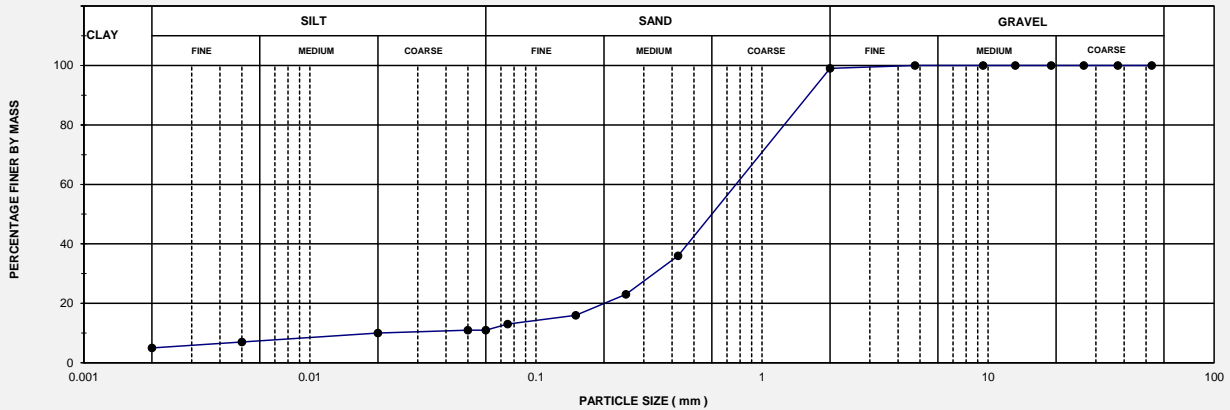


FOUNDATION INDICATOR TEST RESULTS - REP COM 7

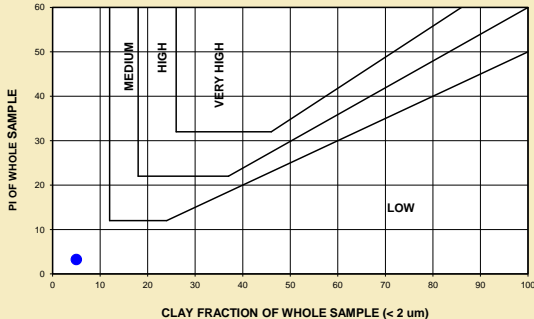
Client: Nomfundo Exploration	Source/Location:	Job No: 2016-C-991
Project Name: Rand Water Pipeline Project in Vereenig. Layer:		Sample No: 6/7748
Project No: 0	Lane:	Date: 02/09/2016
Hole/TP No: TP29	Stabilizing Agent:	Test Method: TMH1 A1, A5 & ASTM D422
Depth (m): 0.7 - 3.0	Section:	Client Ref No:
Description: Yellow speckled black grey clayey silty s: Chainage:		GPS X:
Additional Info:	Offset:	GPS Y:

SIEVE ANALYSIS				ATTERBERG LIMITS		SOIL CLASSIFICATION	
Sieve (mm)	% Passing	Sieve (mm)	% Passing				
75.0	100	0.425	36	Liquid Limit (%)	19	% Gravel	1
63.0	100	0.250	23	Plastic Limit (%)	10	% Sand	88
53.0	100	0.150	16	Plasticity Index (%)	9	% Silt	6
37.5	100	0.075	13	Weighted PI (%)	3.2	% Clay	5
26.5	100	0.060	11	Linear Shrinkage (%)	2.5	Activity	1.8
19.0	100	0.050	11	Grading Modulus	1.52	% Soil Mortar	99
13.2	100	0.020	10	Uniformity coefficient	47	Coarse Sand Ratio	0.64
9.5	100	0.005	7	Coefficient of curvature	5.3	TRB Classification	A - 2 - 4
4.75	100	0.002	5			Unified Classification	SC
2.00	99			Remarks:			

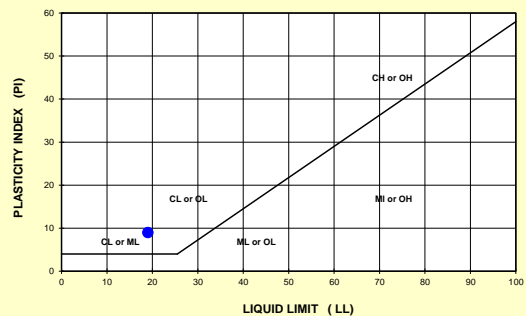
PARTICLE SIZE DISTRIBUTION



POTENTIAL EXPANSIVENESS Van der Merwe's Activity Chart



CASAGRANDE 'A' LINE



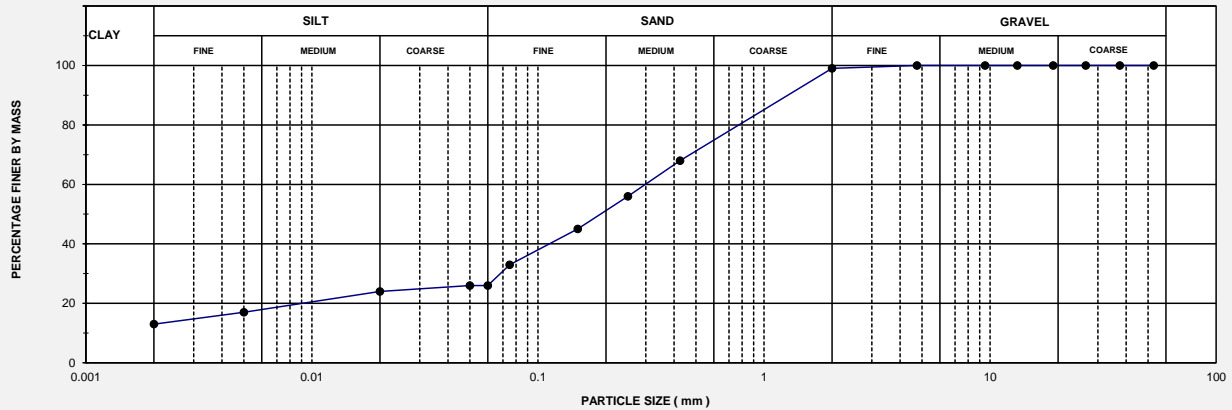


FOUNDATION INDICATOR TEST RESULTS - REP COM 7

Client: Nomfundo Exploration	Source/Location:	Job No: 2016-C-991
Project Name: Rand Water Pipeline Project in Vereenig.	Layer:	Sample No: 6/7749
Project No: 0	Lane:	Date: 02/09/2016
Hole/TP No: TP30	Stabilizing Agent:	Test Method: TMH1 A1, A5 & ASTM D422
Depth (m): 0.5 - 3.0	Section:	Client Ref No:
Description: Yellow speckled black grey clayey silty s	Chainage:	GPS X:
Additional Info:	Offset:	GPS Y:

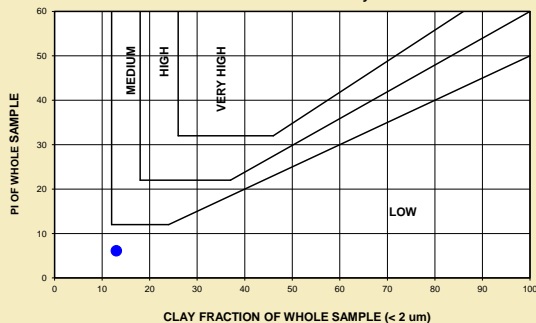
SIEVE ANALYSIS				ATTERBERG LIMITS		SOIL CLASSIFICATION	
Sieve (mm)	% Passing	Sieve (mm)	% Passing				
75.0	100	0.425	68	Liquid Limit (%)	22	% Gravel	1
63.0	100	0.250	56	Plastic Limit (%)	13	% Sand	73
53.0	100	0.150	45	Plasticity Index (%)	9	% Silt	13
37.5	100	0.075	33	Weighted PI (%)	6.1	% Clay	13
26.5	100	0.060	26	Linear Shrinkage (%)	3.5	Activity	0.7
19.0	100	0.050	26	Grading Modulus	1.00	% Soil Mortar	99
13.2	100	0.020	24	Uniformity coefficient	154	Coarse Sand Ratio	0.31
9.5	100	0.005	17	Coefficient of curvature	8.1	TRB Classification	A - 2 - 4
4.75	100	0.002	13			Unified Classification	SC
2.00	99			Remarks:			

PARTICLE SIZE DISTRIBUTION

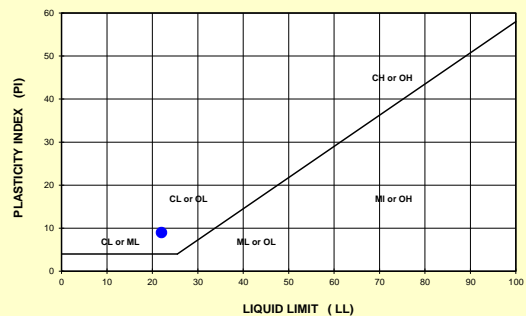


POTENTIAL EXPANSIVENESS

Van der Merwe's Activity Chart



CASAGRANDE 'A' LINE



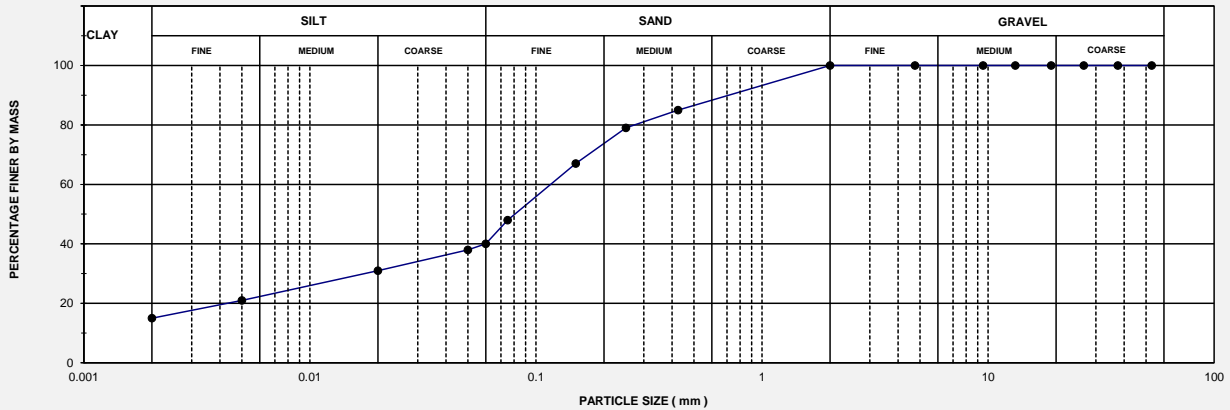


FOUNDATION INDICATOR TEST RESULTS - REP COM 7

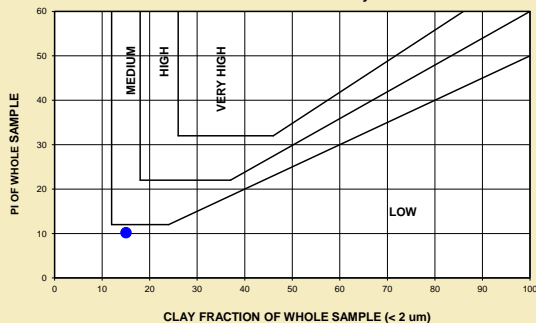
Client: Nomfundo Exploration	Source/Location:	Job No: 2016-C-991
Project Name: Rand Water Pipeline Project in Vereenig. Layer:		Sample No: 6/7750
Project No: 0	Lane:	Date: 02/09/2016
Hole/TP No: TP31	Stabilizing Agent:	Test Method: TMH1 A1, A5 & ASTM D422
Depth (m): 0.8 - 2.8	Section:	Client Ref No:
Description: Yellow speckled black grey clayey silty s:Chainage:		GPS X:
Additional Info:	Offset:	GPS Y:

SIEVE ANALYSIS				ATTERBERG LIMITS		SOIL CLASSIFICATION	
Sieve (mm)	% Passing	Sieve (mm)	% Passing				
75.0	100	0.425	85	Liquid Limit (%)	25	% Gravel	0
63.0	100	0.250	79	Plastic Limit (%)	13	% Sand	60
53.0	100	0.150	67	Plasticity Index (%)	12	% Silt	25
37.5	100	0.075	48	Weighted PI (%)	10.2	% Clay	15
26.5	100	0.060	40	Linear Shrinkage (%)	3.5	Activity	0.8
19.0	100	0.050	38	Grading Modulus	0.67	% Soil Mortar	100
13.2	100	0.020	31	Uniformity coefficient	61	Coarse Sand Ratio	0.15
9.5	100	0.005	21	Coefficient of curvature	1.7	TRB Classification	A - 6
4.75	100	0.002	15			Unified Classification	SC
2.00	100			Remarks:			

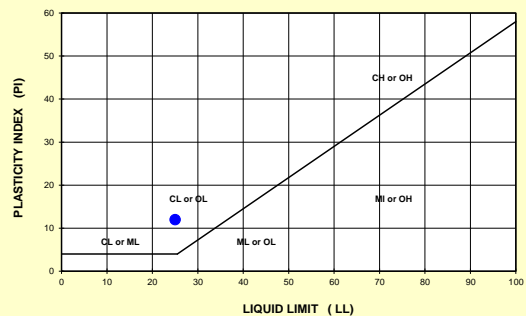
PARTICLE SIZE DISTRIBUTION



POTENTIAL EXPANSIVENESS Van der Merwe's Activity Chart



CASAGRANDE 'A' LINE



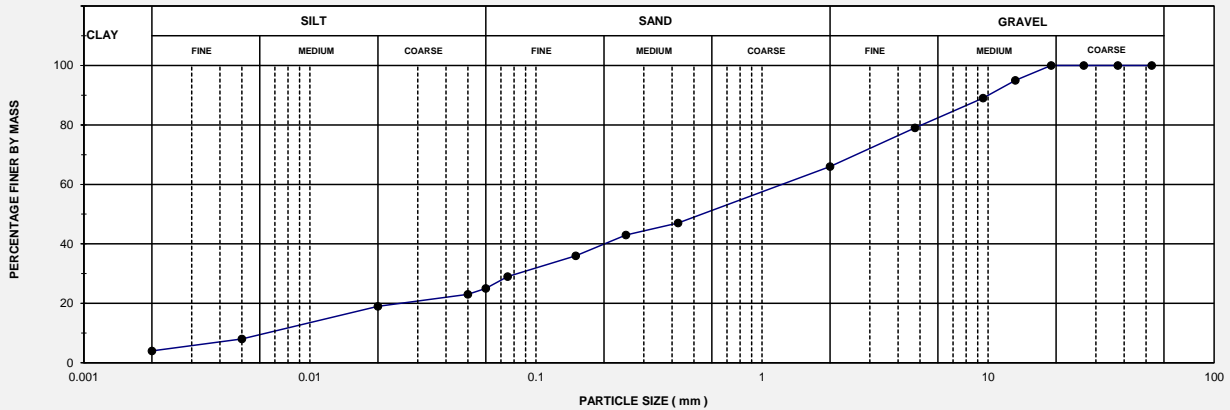


FOUNDATION INDICATOR TEST RESULTS - REP COM 7

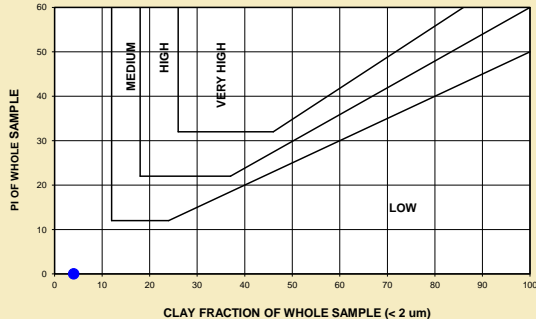
Client: Nomfundo Exploration	Source/Location:	Job No: 2016-C-991
Project Name: Rand Water Pipeline Project in Vereenig. Layer:		Sample No: 6/7752
Project No: 0	Lane:	Date: 02/09/2016
Hole/TP No: TP34	Stabilizing Agent:	Test Method: TMH1 A1, A5 & ASTM D422
Depth (m): 0.4 - 1.2	Section:	Client Ref No:
Description: Grey speckled white silty sand ASH (Fill)	Chainage:	GPS X:
Additional Info:	Offset:	GPS Y:

SIEVE ANALYSIS				ATTERBERG LIMITS		SOIL CLASSIFICATION	
Sieve (mm)	% Passing	Sieve (mm)	% Passing				
75.0	100	0.425	47	Liquid Limit (%)		% Gravel	34
63.0	100	0.250	43	Plastic Limit (%)		% Sand	41
53.0	100	0.150	36	Plasticity Index (%)	NP	% Silt	21
37.5	100	0.075	29	Weighted PI (%)		% Clay	4
26.5	100	0.060	25	Linear Shrinkage (%)	0.0	Activity	0.0
19.0	100	0.050	23	Grading Modulus	1.58	% Soil Mortar	66
13.2	95	0.020	19	Uniformity coefficient	154	Coarse Sand Ratio	0.29
9.5	89	0.005	8	Coefficient of curvature	0.5	TRB Classification	A - 2 - 7
4.75	79	0.002	4			Unified Classification	SM
2.00	66			Remarks:			

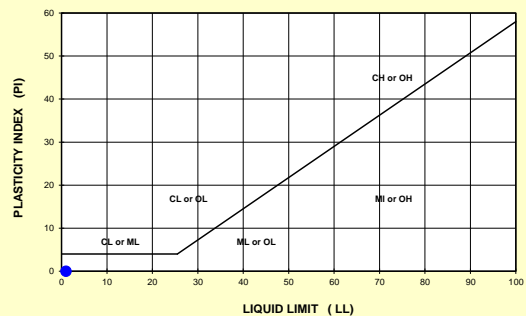
PARTICLE SIZE DISTRIBUTION



POTENTIAL EXPANSIVENESS Van der Merwe's Activity Chart



CASAGRANDE 'A' LINE



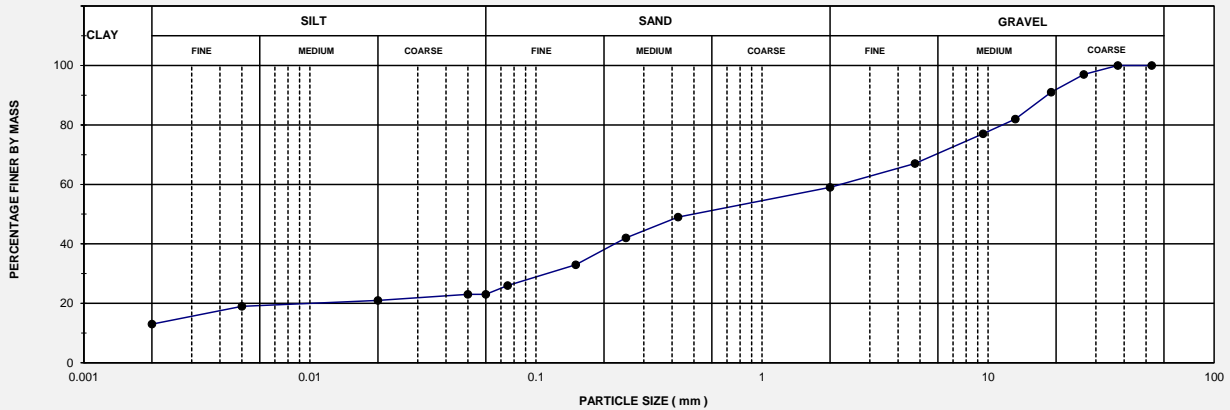


FOUNDATION INDICATOR TEST RESULTS - REP COM 7

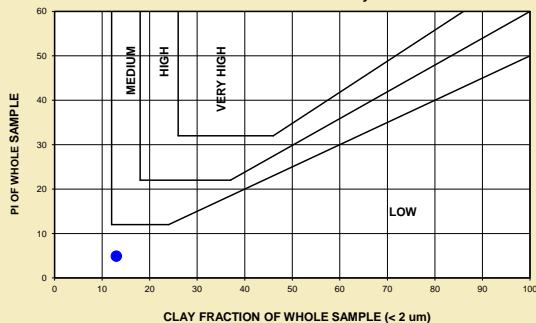
Client: Nomfundo Exploration	Source/Location:	Job No: 2016-C-991
Project Name: Rand Water Pipeline Project in Vereenig. Layer:		Sample No: 6/7753
Project No: 0	Lane:	Date: 02/09/2016
Hole/TP No: TP34	Stabilizing Agent:	Test Method: TMH1 A1, A5 & ASTM D422
Depth (m): 1.2 - 2.6	Section:	Client Ref No:
Description: Khakhi mottled yellow sandy clayey silt (I Chainage:		GPS X:
Additional Info:	Offset:	GPS Y:

SIEVE ANALYSIS				ATTERBERG LIMITS		SOIL CLASSIFICATION	
Sieve (mm)	% Passing	Sieve (mm)	% Passing				
75.0	100	0.425	49	Liquid Limit (%)	21	% Gravel	41
63.0	100	0.250	42	Plastic Limit (%)	11	% Sand	36
53.0	100	0.150	33	Plasticity Index (%)	10	% Silt	10
37.5	100	0.075	26	Weighted PI (%)	4.9	% Clay	13
26.5	97	0.060	23	Linear Shrinkage (%)	4.5	Activity	0.8
19.0	91	0.050	23	Grading Modulus	1.66	% Soil Mortar	59
13.2	82	0.020	21	Uniformity coefficient	1172	Coarse Sand Ratio	0.17
9.5	77	0.005	19	Coefficient of curvature	3.1	TRB Classification	A - 2 - 4
4.75	67	0.002	13			Unified Classification	SC
2.00	59			Remarks:			

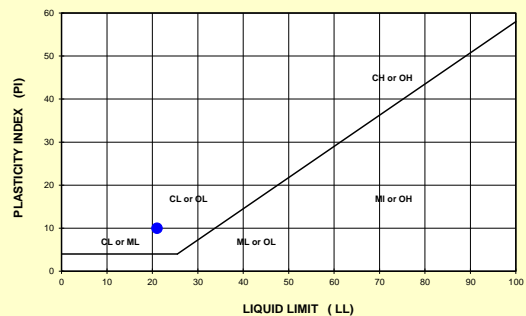
PARTICLE SIZE DISTRIBUTION



POTENTIAL EXPANSIVENESS Van der Merwe's Activity Chart



CASAGRANDE 'A' LINE



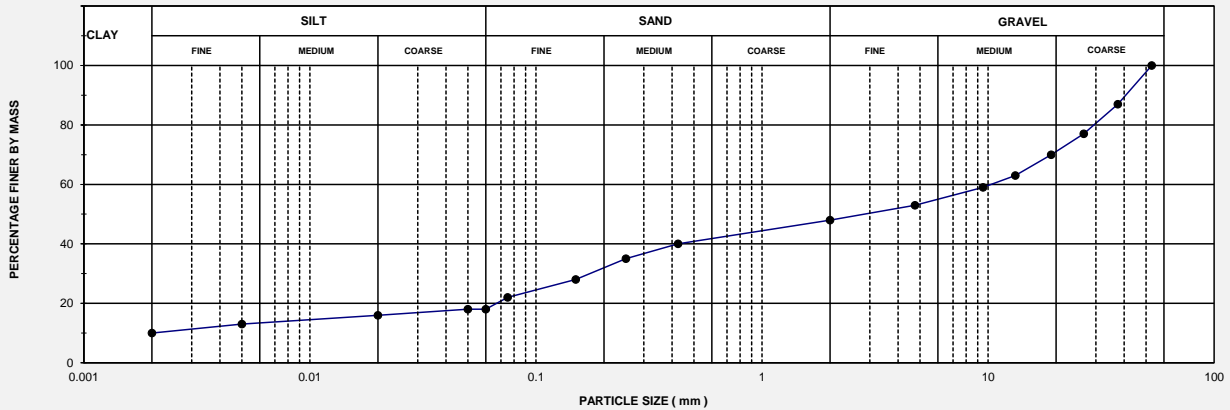


FOUNDATION INDICATOR TEST RESULTS - REP COM 7

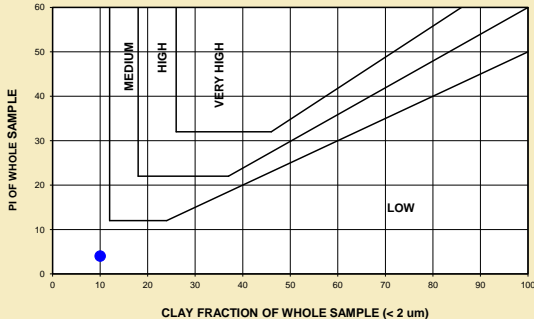
Client: Nomfundo Exploration	Source/Location:	Job No: 2016-C-991
Project Name: Rand Water Pipeline Project in Vereenig. Layer:		Sample No: 6/7754
Project No: 0	Lane:	Date: 02/09/2016
Hole/TP No: TP34	Stabilizing Agent:	Test Method: TMH1 A1, A5 & ASTM D422
Depth (m): 2.6 - 3.0	Section:	Client Ref No:
Description: Khakhi mottled yellow sandy clayey silt a Chainage:		GPS X:
Additional Info:	Offset:	GPS Y:

SIEVE ANALYSIS				ATTERBERG LIMITS			SOIL CLASSIFICATION	
Sieve (mm)	% Passing	Sieve (mm)	% Passing					
75.0	100	0.425	40	Liquid Limit (%)	21	% Gravel	52	
63.0	100	0.250	35	Plastic Limit (%)	11	% Sand	30	
53.0	100	0.150	28	Plasticity Index (%)	10	% Silt	8	
37.5	87	0.075	22	Weighted PI (%)	4.0	% Clay	10	
26.5	77	0.060	18	Linear Shrinkage (%)	4.5	Activity	1.0	
19.0	70	0.050	18	Grading Modulus	1.90	% Soil Mortar	48	
13.2	63	0.020	16	Uniformity coefficient	5213	Coarse Sand Ratio	0.17	
9.5	59	0.005	13	Coefficient of curvature	1.6	TRB Classification	A - 2 - 4	
4.75	53	0.002	10			Unified Classification	GC	
2.00	48			Remarks:				

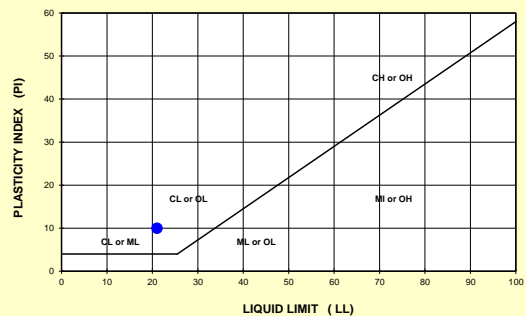
PARTICLE SIZE DISTRIBUTION



POTENTIAL EXPANSIVENESS Van der Merwe's Activity Chart



CASAGRANDE 'A' LINE



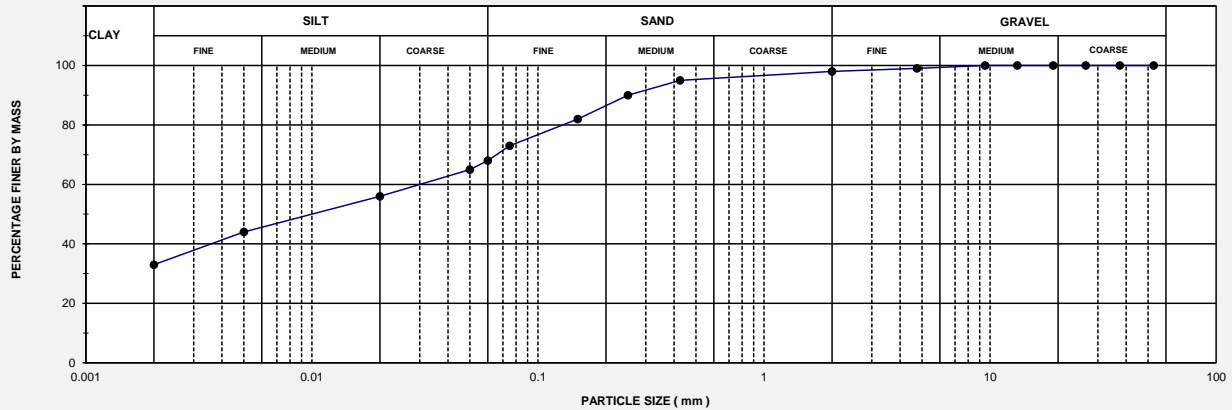


FOUNDATION INDICATOR TEST RESULTS - REP COM 7

Client:	Nomfundo Exploration	Source/Location:		Job No:	2016-C-991
Project Name:	Rand Water Pipeline Project in Vereenig. Layer:			Sample No:	6/7755
Project No:	0	Lane:		Date:	02/09/2016
Hole/TP No:	TP35	Stabilizing Agent:		Test Method:	TMH1 A1, A5 & ASTM D422
Depth (m):	0.3 - 1.0	Section:		Client Ref No:	
Description:	Khakhi mottled yellow sandy clayey silt (I Chainage:			GPS X:	
Additional Info:		Offset:		GPS Y:	

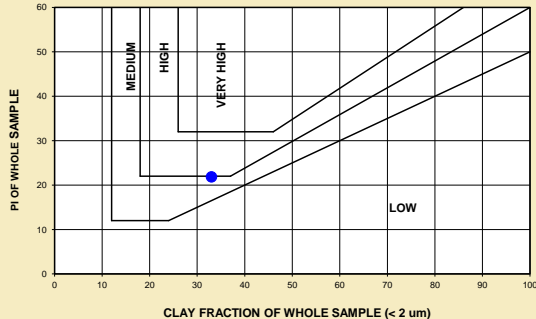
SIEVE ANALYSIS				ATTERBERG LIMITS		SOIL CLASSIFICATION	
Sieve (mm)	% Passing	Sieve (mm)	% Passing				
75.0	100	0.425	95	Liquid Limit (%)	39	% Gravel	2
63.0	100	0.250	90	Plastic Limit (%)	16	% Sand	30
53.0	100	0.150	82	Plasticity Index (%)	23	% Silt	35
37.5	100	0.075	73	Weighted PI (%)	21.9	% Clay	33
26.5	100	0.060	68	Linear Shrinkage (%)	8.5	Activity	0.7
19.0	100	0.050	65	Grading Modulus	0.34	% Soil Mortar	98
13.2	100	0.020	56	Uniformity coefficient	17	Coarse Sand Ratio	0.03
9.5	100	0.005	44	Coefficient of curvature	0.1	TRB Classification	A - 6
4.75	99	0.002	33			Unified Classification	CL
2.00	98			Remarks:			

PARTICLE SIZE DISTRIBUTION

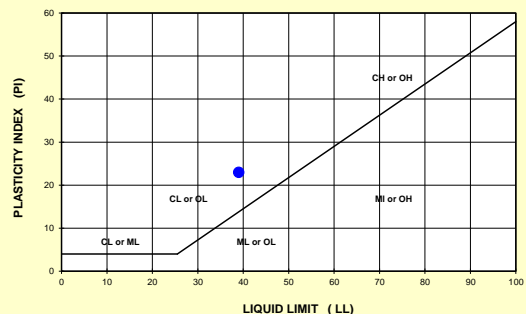


POTENTIAL EXPANSIVENESS

Van der Merwe's Activity Chart



CASAGRANDE 'A' LINE



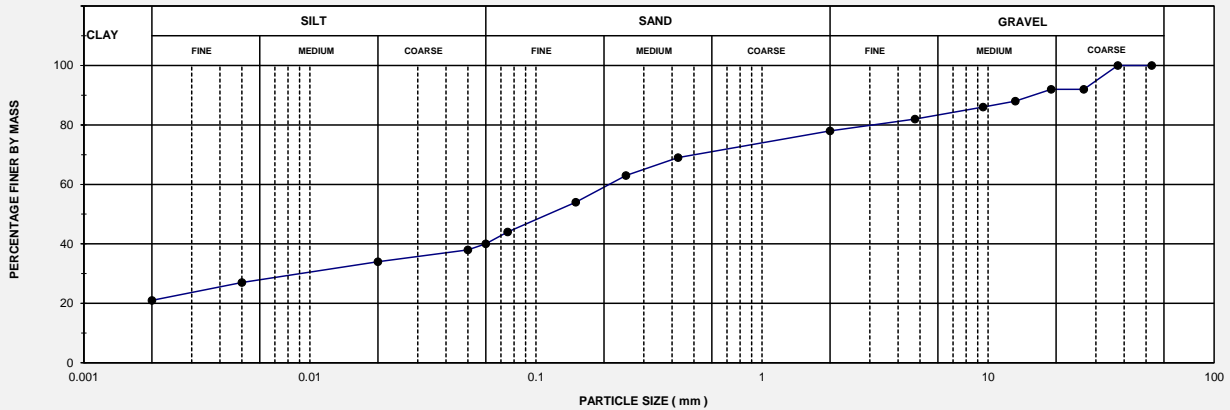


FOUNDATION INDICATOR TEST RESULTS - REP COM 7

Client:	Nomfundo Exploration	Source/Location:		Job No:	2016-C-991
Project Name:	Rand Water Pipeline Project in Vereenig. Layer:			Sample No:	6/7756
Project No:	0	Lane:		Date:	02/09/2016
Hole/TP No:	TP35	Stabilizing Agent:		Test Method:	TMH1 A1, A5 & ASTM D422
Depth (m):	1.0 - 3.0	Section:		Client Ref No:	
Description:	Khakhi mottled yellow sandy clayey silt (I Chainage:			GPS X:	
Additional Info:		Offset:		GPS Y:	

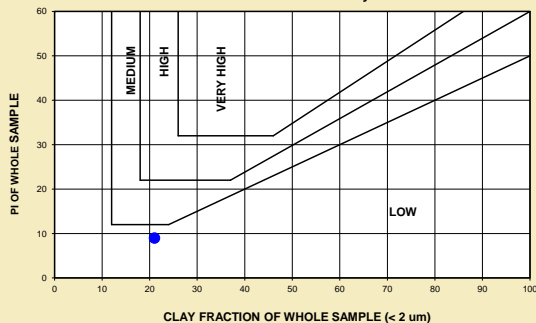
SIEVE ANALYSIS				ATTERBERG LIMITS		SOIL CLASSIFICATION	
Sieve (mm)	% Passing	Sieve (mm)	% Passing				
75.0	100	0.425	69	Liquid Limit (%)	27	% Gravel	22
63.0	100	0.250	63	Plastic Limit (%)	14	% Sand	38
53.0	100	0.150	54	Plasticity Index (%)	13	% Silt	19
37.5	100	0.075	44	Weighted PI (%)	9.0	% Clay	21
26.5	92	0.060	40	Linear Shrinkage (%)	7.5	Activity	0.6
19.0	92	0.050	38	Grading Modulus	1.09	% Soil Mortar	78
13.2	88	0.020	34	Uniformity coefficient	108	Coarse Sand Ratio	0.12
9.5	86	0.005	27	Coefficient of curvature	0.4	TRB Classification	A - 6
4.75	82	0.002	21			Unified Classification	SC
2.00	78			Remarks:			

PARTICLE SIZE DISTRIBUTION

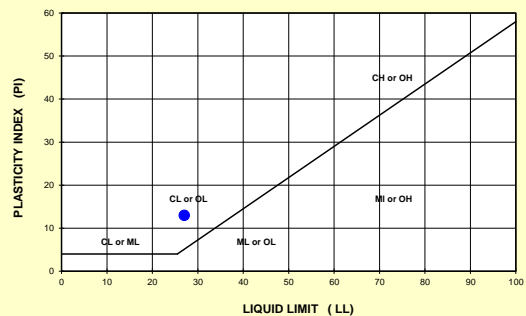


POTENTIAL EXPANSIVENESS

Van der Merwe's Activity Chart



CASAGRANDE 'A' LINE





T0023



SUMMARY OF TEST RESULTS ON SOILS - REP COM 2

Client: Nomfundo Exploration Project: Rand Water Pipeline Project in Vereeniging Ref No: Job No: 2016-C-991 Date: 02/09/2016

Table with columns: Sample No, Hole No / TP No, Depth (m), Description, Percentage Passing Sieve (mm) - TMH1 A1, A5, ASTM D422, Atterberg Limits (TMH1 A2-A4) < 0.425 mm, Maximum Dry Density (kg/m³) & OMC (%) (TMH1 A7), CBR (Modified AASHTO) TMH1 A8, pH + Conductivity (TMH A20 & A21T). Rows include samples 6/7723 to 6/7749 with various soil descriptions and test results.

Remarks: Everything possible is done to ensure that tests are representative and are performed accurately, and that reports and conclusions are quoted correctly. Geostrada or its officials can in no way be held liable for consequential damage or loss due to any error in carrying out the tests, nor for any erroneous statement or opinion contained in a report based on such tests. If a test report is published or reproduced by the client, it will be done in full, without any omission.

ANNEXURE E

COMMERCIAL SOURCES LABORATORY RESULTS

ROADLAB

Civil Engineering Materials Laboratory
(PTY) LTD • 1945/009083/07 • VAT No. 4660115884

Established 1965

HEAD OFFICE
207 Rieffontein Rd Primrose Germiston 1401
P O Box 1476 Germiston 1400
Tel: 011 828 0279 Fax: 011 828 0273
E-mail: info@roadlab.co.za
www.roadlab.co.za

95/CL1001/01/0001/14

29/04/2014

Clinker Supplies (Pty) Ltd
P.O. Box 151
Alberfon
1450

ATTENTION: Mr. Paul Davies

Dear Sir

Test Report: **CLINKER SUPPLIES: AGGREGATE TEST RESULTS (TRACK No.8006)**

Please find the attached test results for the sample/s as submitted to and tested by Roadlab (Pty) Ltd in Primrose, Germiston.
The unambiguous description of the sample/s as received are as follows:

SAMPLE NO	14/A249	14/A250	REMARKS & NOTES		
	CONTAINER USED FOR SAMPLING	Sampling Plastic Bag	Sampling Plastic Bag		
MOISTURE CONDITION OF SAMPLE ON ARRIVAL	Dry	Dry			
HOLE NO. / KM OR CHAINAGE	Not Specified	Not Specified			
ROAD NO OR NAME	Not Specified	Not Specified			
LAYER TESTED / SAMPLED FROM	Black Sand	Base Course			
DATE SAMPLED	27/11/2013	06/05/2014			
DATE RECEIVED	06/05/2014	06/05/2014			
CLIENTS MARKING	None	None			
REQUEST NO.	None	None			
DESCRIPTION OF SAMPLE (COLOR & TYPE)	Black Sand Pale Red Ash	Base Course			
NOMINAL SIZE OF STONE			SPECIFICATION		
			MIN	-	MAX
Sieve size (mm) TMH 1 B4					
75.0					
53.0					
37.5					
26.5					
19.0					
13.2					
9.5					
6.7					
4.75					
2.36					
1.18					
0.600					
0.300					
0.150					
Dust %	0.075				

FM	TMH 1	[B13]						
ACV (%) - DRY		[B1]						
10 % FACT (kN) - DRY		[B2]						
ACV (%) - WET		[B2]						
10 % FACT (kN) - WET		[B2]						
Ethylene Glycol Durability Index #								
Flakiness Index (%)		[B3]						
Organic Material (Y/N) #		[B6]						
Loose Bulk Density Kg/m ³ #	1.093	[B9]		0.726				
Compacted Bulk Density Kg/m ³ #	1.152	[B9]		0.776				
Bulking Factor #	1.05			1.07				
Soluble Sulfates (%) #		[B17]						
Ave. Least Dimension: (mm)		[B18(a)]						
Sand Equivalent (%) #		[B19]						
Relative density Kg/m ³ #		[B108b]						
Conductivity (ms/m) #		[A21T]						
ARD (-4.75mm) Kg/m ³	3.958	[B15]						
BRD (-4.75mm) Kg/m ³	3.572	[B15]						
ARD (+4.75mm) Kg/m ³		[B14]		2.026				
BRD (+4.75mm) Kg/m ³		[B14]		1.697				
MgSO ₄ Soundness #		[B39]						
Polished Stone Value #		[BS 812]						
Water absorption (%)	2.7	[B15]						
Water absorption (%)		[B14]		9.6				
Sample Method used	TMH 5 #		N/A	N/A				

- This is not a accredited test

Page 1/1

Kind Regards

INTERIM

Mr Deon Juckers
Technical Signatory

INTERIM

Mr Charel van Biljon
Laboratory Manager

REV 000

Remarks :
* Opinions & Interpretations are not included in our schedule of Accreditation
The samples were subjected to analysis according to TMH 1
Sanas Accredited Laboratory - T 0296
The results reported relate only to the sample tested
Further use of the above information is not the responsibility or liability of Roadlab
Documents may only be reproduced or published in their full context
Compiled By : Miss Trisha Padotan

RL-as-76-03

ROADLAB

Civil Engineering Materials Laboratory
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Established 1965

HEAD OFFICE
207 Rieffontein Rd Primrose Germiston 1401
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95/CL1001/01/0001/14

29/04/2014

Clinker Supplies (Pty) Ltd
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1450

ATTENTION: Mr. Paul Davies

Dear Sir

Test Report: **CLINKER SUPPLIES: AGGREGATE TEST RESULTS (TRACK No.8006)**

Please find the attached test results for the sample/s as submitted to and tested by Roadlab (Pty) Ltd in Primrose, Germiston.
The unambiguous description of the sample/s as received are as follows:

SAMPLE NO	14/A249	14/A250	REMARKS & NOTES		
	CONTAINER USED FOR SAMPLING	Sampling Plastic Bag	Sampling Plastic Bag		
MOISTURE CONDITION OF SAMPLE ON ARRIVAL	Dry	Dry			
HOLE NO. / KM OR CHAINAGE	Not Specified	Not Specified			
ROAD NO OR NAME	Not Specified	Not Specified			
LAYER TESTED / SAMPLED FROM	Black Sand	Base Course			
DATE SAMPLED	27/11/2013	06/05/2014			
DATE RECEIVED	06/05/2014	06/05/2014			
CLIENTS MARKING	None	None			
REQUEST NO.	None	None			
DESCRIPTION OF SAMPLE (COLOR & TYPE)	Black Sand Pale Red Ash	Base Course			
NOMINAL SIZE OF STONE			MIN	-	MAX
Sieve size (mm) TMH 1 B4					
75.0					
53.0					
37.5					
26.5					
19.0					
13.2					
9.5					
6.7					
4.75					
2.36					
1.18					
0.600					
0.300					
0.150					
Dust %	0.075				

FM	TMH 1	[B13]						
ACV (%) - DRY		[B1]						
10 % FACT (kN) - DRY		[B2]						
ACV (%) - WET		[B2]						
10 % FACT (kN) - WET		[B2]						
Ethylene Glycol Durability Index #								
Flakiness Index (%)		[B3]						
Organic Material (Y/N) #		[B6]						
Loose Bulk Density Kg/m ³ #	1.093	[B9]		0.726				
Compacted Bulk Density Kg/m ³ #	1.152	[B9]		0.776				
Bulking Factor #	1.05			1.07				
Soluble Sulfates (%) #		[B17]						
Ave. Least Dimension (mm)		[B18(a)]						
Sand Equivalent (%) #		[B19]						
Relative density Kg/m ³ #		[B108b]						
Conductivity (ms/m) #		[A21T]						
ARD (-4.75mm) Kg/m ³	3.958	[B15]						
BRD (-4.75mm) Kg/m ³	3.572	[B15]						
ARD (+4.75mm) Kg/m ³		[B14]		2.026				
BRD (+4.75mm) Kg/m ³		[B14]		1.697				
MgSO ₄ Soundness #		[B39]						
Polished Stone Value #		[BS 812]						
Water absorption (%)	2.7	[B15]						
Water absorption (%)		[B14]		9.6				
Sample Method used	TMH 5 #		N/A	N/A				

- This is not a accredited test

Page 1/1

Kind Regards

INTERIM

Mr Deon Juckers
Technical Signatory

INTERIM

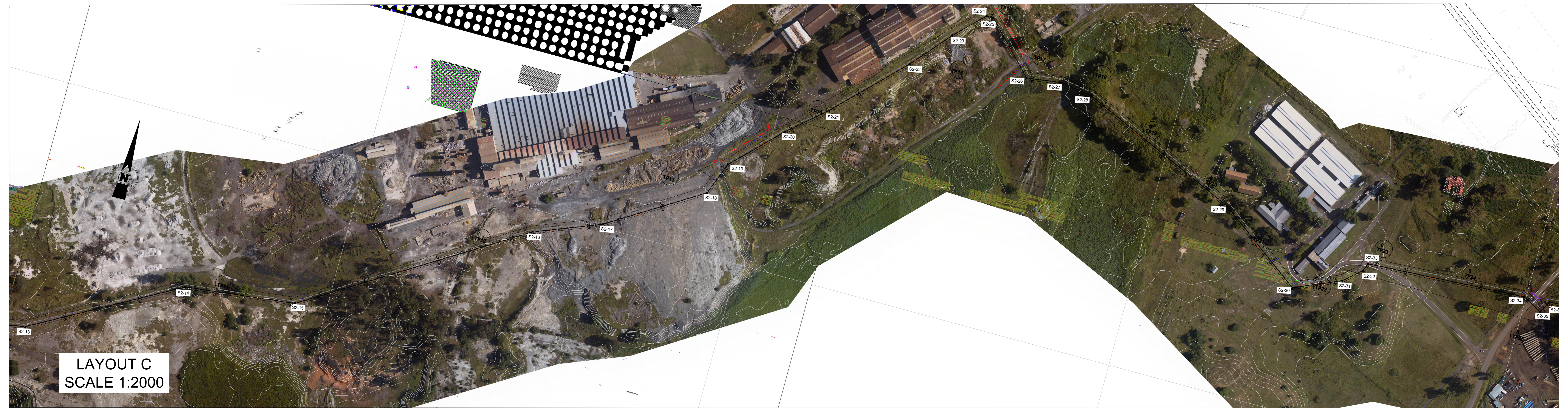
Mr Charel van Biljon
Laboratory Manager

REV 000

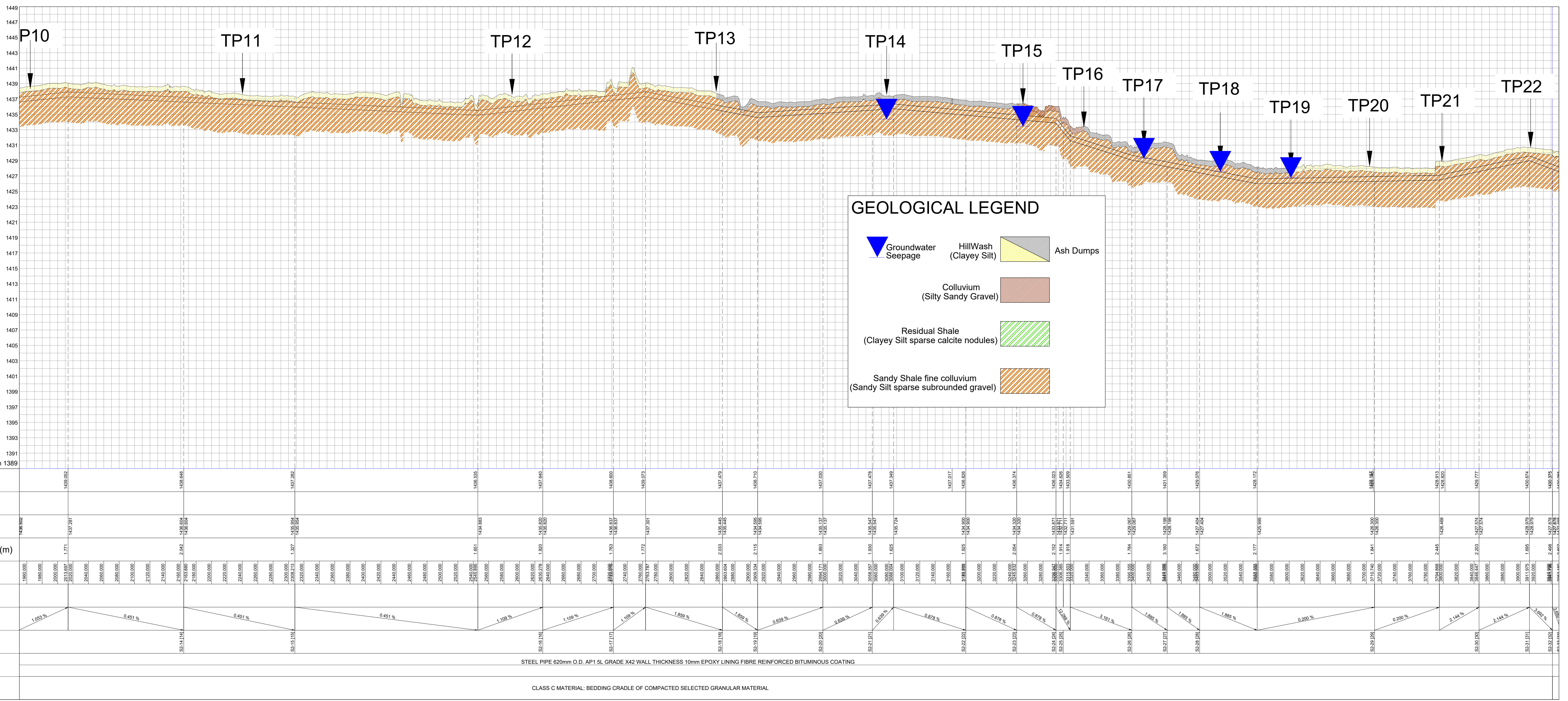
Remarks :
* Opinions & Interpretations are not included in our schedule of Accreditation
The samples were subjected to analysis according to TMH 1
Sanas Accredited Laboratory - T 0296
The results reported relate only to the sample tested
Further use of the above information is not the responsibility or liability of Roadlab
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Compiled By : Miss Trisha Padotan

RL-as-76-03

ANNEXURE F
LONGITUDINAL SECTIONS
TEST RESULTS



LAYOUT C
SCALE 1:2000



HOR:1:2000
VERT:1:200

Station	Ground Level (m)	Ground Cover to Top of Pipe (m)	Pipe Invert Level (m)	Cut from Ground to Trench Floor (m)	Horizontal Peg Distance (m)	Slope Peg Distance (m)	Grade (Per 100m) (%)
1900+00	1438.62		1438.62				1.053%
1950+00	1438.62		1438.62				0.451%
2000+00	1438.62		1438.62				0.451%
2050+00	1438.62		1438.62				1.109%
2100+00	1438.62		1438.62				1.109%
2150+00	1438.62		1438.62				1.899%
2200+00	1438.62		1438.62				1.899%
2250+00	1438.62		1438.62				0.878%
2300+00	1438.62		1438.62				0.878%
2350+00	1438.62		1438.62				3.191%
2400+00	1438.62		1438.62				1.688%
2450+00	1438.62		1438.62				1.885%
2500+00	1438.62		1438.62				0.200%
2550+00	1438.62		1438.62				0.200%
2600+00	1438.62		1438.62				2.144%
2650+00	1438.62		1438.62				2.144%
2700+00	1438.62		1438.62				3.500%
2750+00	1438.62		1438.62				0.000%
2800+00	1438.62		1438.62				0.000%
2850+00	1438.62		1438.62				0.000%
2900+00	1438.62		1438.62				0.000%
2950+00	1438.62		1438.62				0.000%
3000+00	1438.62		1438.62				0.000%
3050+00	1438.62		1438.62				0.000%
3100+00	1438.62		1438.62				0.000%
3150+00	1438.62		1438.62				0.000%
3200+00	1438.62		1438.62				0.000%
3250+00	1438.62		1438.62				0.000%
3300+00	1438.62		1438.62				0.000%
3350+00	1438.62		1438.62				0.000%
3400+00	1438.62		1438.62				0.000%
3450+00	1438.62		1438.62				0.000%
3500+00	1438.62		1438.62				0.000%
3550+00	1438.62		1438.62				0.000%
3600+00	1438.62		1438.62				0.000%
3650+00	1438.62		1438.62				0.000%
3700+00	1438.62		1438.62				0.000%
3750+00	1438.62		1438.62				0.000%
3800+00	1438.62		1438.62				0.000%
3850+00	1438.62		1438.62				0.000%
3900+00	1438.62		1438.62				0.000%

STEEL PIPE 620mm O.D. AP1 5L GRADE X42 WALL THICKNESS 10mm EPOXY LINING FIBRE REINFORCED BITUMINOUS COATING
CLASS C MATERIAL - BEDDING CRADLE OF COMPACTED SELECTED GRANULAR MATERIAL

LEGEND	LEGEND	CATHODIC PROTECTION LEGEND:
AIR VALVE WITHOUT ACCESS MANHOLE	BEACON (BCN)	EXISTING CROSS BOND FACILITY
AIR VALVE WITH ACCESS MH	PIPE LOCATOR TEST POINT (PLT)	NEW CROSS BOND FACILITY
INLINE SLUICE VALVE	METER	NEW CP SYSTEM
INLINE BUTTERFLY VALVE	ELECTRIC POLE (EP)	EXISTING CP SYSTEM
SCOUR VALVE	TELEPHONE POLE (TP)	ZINC RIBBON
REFLEX VALVE	FENCE	EARTH MAT
PIPE JACKING	SURVEY STATION	
PIPE BEND REFERENCE MARKER		

NOTES:
 1. ALL PEGS ARE 12mm ROUND IRON PEGS ON CENTRE LINE OF PIPELINE.
 2. HORIZONTAL BENDS ARE SQUARE PEGS
 3. OVER HEAD ELECTRICAL PYLONS - BEWARE OF UNDERGROUND EARTH STRAPS AND CABLES BETWEEN PYLONS
 4. ENSURE ACCESS TO PROPERTIES ARE AVAILABLE WHERE DRIVEWAYS ARE AFFECTED
 5. FOR CONCRETE ENCASEMENT REFER TO CIVIL DRAWINGS
 6. ALL ATTEMPTS ARE UNDERTAKEN BY THE SURVEYOR TO LOCATE ALL AFFECTED SERVICES ABOVE AND BELOW GROUND, SURFACE. IT IS OF UTMOST IMPORTANCE THAT THE CONTRACTOR TAKE THE NECESSARY PRECAUTIONS WHEN EXCAVATING IN THE VICINITY OF SERVICES.

REVISIONS					REFERENCE DRAWINGS	
No.	Date	Checked	Approved	Description	Number	Title
A	17-10-2016	A.M	A.M.	ISSUED FOR APPROVAL		

Box 1137
Johannesburg
2000
Tel: 011 682-0911

L.W. Lange
(P. Eng.)
Senior Manager
Assets

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SAP No: _____ checked: _____
 Contract No: A.M Accepted: _____
 Originator: _____ Date: 17/10/2016

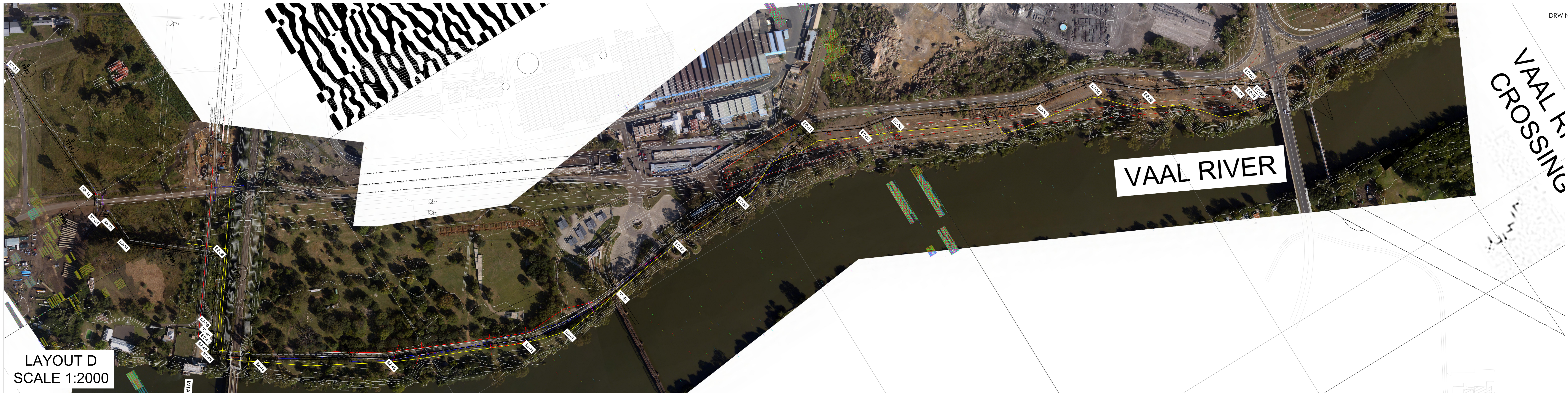
Drawn By: W.K
 Checked By: A.M
 Date: 17/10/2016

Approved: _____
 Registration No: _____
 Pr. Eng.

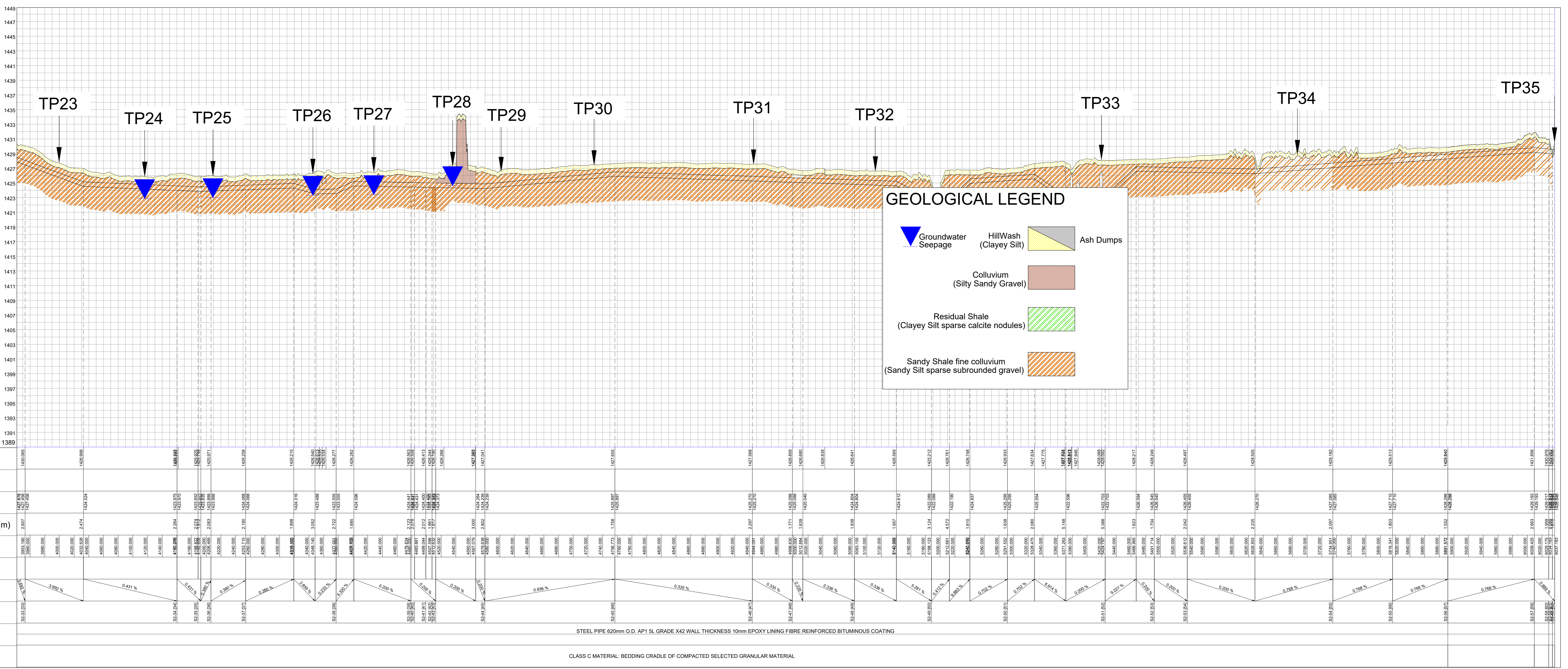
**B19 SLUDGE PIPE LINE PHASE 2
ZUIKERBOSCH - VEREENIGING
PLAN AND LONGITUDINAL SECTION
CH 1940.00m - CH 3940.00m**

SCALE: AS SHOWN
 DRW No: RW201501/SLS/02

VAAL RIVER CROSSING



LAYOUT D
SCALE 1:2000



HOR:1:2000
VERT:1:200

LEGEND	LEGEND	CATHODIC PROTECTION LEGEND:
AIR VALVE WITHOUT ACCESS MANHOLE	BEACON (BCN)	EXISTING CROSS BOND FACILITY
AIR VALVE WITH ACCESS MH	PIPE LOCATOR TEST POINT (PLT)	NEW CP SYSTEM
INLINE SLUICE VALVE	METER	EXISTING CP SYSTEM
INLINE BUTTERFLY VALVE	ELECTRIC POLE (EP)	ZINC RIBBON
SCOUR VALVE	TELEPHONE POLE (TP)	EARTH MAT
REFLUX VALVE	FENCE	
PIPE JACKING	SURVEY STATION	
PIPE BEND REFERENCE MARKER		

NOTES:

- ALL PEGS ARE 12mm ROUND IRON PEGS ON CENTRE LINE OF PIPELINE.
- HORIZONTAL BENDS ARE SQUARE PEGS
- OVER HEAD ELECTRICAL PYLONS - BEWARE OF UNDERGROUND EARTH STRAPS AND CABLES BETWEEN PYLONS
- ENSURE ACCESS TO PROPERTIES ARE AVAILABLE WHERE DRIVEWAYS ARE AFFECTED
- FOR CONCRETE ENCASUREMENT REFER TO CIVIL DRAWINGS
- ALL ATTEMPTS ARE UNDERTAKEN BY THE SURVEYOR TO LOCATE ALL AFFECTED SERVICES ABOVE AND BELOW GROUND, SURFACE. IT IS OF UTMOST IMPORTANCE THAT THE CONTRACTOR TAKE THE NECESSARY PRECAUTIONS WHEN EXCAVATING IN THE VICINITY OF SERVICES.

REVISIONS				REFERENCE DRAWINGS	
No.	Date	Checked	Approved	Number	Title
A	17-10-2016	A.M.	A.M.		

RAND WATER

Box 1137
Johannesburg
2000
Tel: 011 682-9911

L.W. Lange
(Pr. Eng.)
Senior Manager
Assets

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SAP No	Checked	Approved
Contract No	Accepted	Registration No.
Originator	Date	

Drawn By: **W.K.**
Checked By: **A.M.**
Date: **17/10/2016**

B19 SLUDGE PIPELINE PHASE 2 ZUIKERBOSCH-VEREENIGING PLAN AND LONGITUDINAL SECTION CH 3940.00m - CH 6037.18m	
SCALE: AS SHOWN	DRW No: RW201501/SL5/03

ANNEXURE G

RESISTIVITY SURVEY REPORT

ENGINEERING & EXPLORATION GEOPHYSICAL SERVICES CC

CK94/10526/23 Geophysical Contractors



170 Jakaranda Street,
Doringkloof,
Gauteng, 0157
012 - 6673369 (tel.) 6675186 (fax)
E-mail: eegs@iafrica.com
25th November, 2016

Nomfundo Exploration & Consulting (Pty) Ltd,
43 Hadley Street,
Riviera,
Pretoria

Attn: Mr G.Rabodiba

Dear Sir,

B19 PIPELINE - SOIL RESISTIVITY SURVEY

1. Introduction

The results are given here of a soil resistivity survey carried out in Vereeniging along the route of the B19 pipeline. Fieldwork was performed in the week starting the 14th November. The survey consisted of thirty soundings, at positions indicated to E&EGS (Appendix 1). There was rainfall before and during the survey.

2. Instrumentation, data collection methods and reduction procedure

2.1 Instrumentation

The data were collected with an ABEM SAS1000 resistivity meter coupled to a 10-64 switch box. Resistivity measurements are obtained by injecting a current into the ground through two electrodes and measuring the resulting potential between another electrode pair. Measurements obtained when systematically increasing the electrode separation - a sounding - record apparent resistivity variations with depth, whilst a set of adjacent soundings provides lateral coverage. The ABEM automates the collection of such data sets.

2.2 Data collection

A Wenner configuration (equally spaced electrodes set in a line) was used for the resistivity imaging, as is required for this type of survey. A half-metre electrode separation was employed and the measurement range was $\frac{1}{2}$, 1, $1\frac{1}{2}$, and 2 metres, and then by one metre increments to 6 metres. Where space allowed the traverses were laid out perpendicular to the route to avoid coupling with any pipes already in place.

2.3 Data reduction and presentation

Any spikes (unrepresentative readings) were removed from the data with the aid of the program Res2Dinv, then the average resistance was calculated for each electrode separation. Apparent resistivity was calculated from average resistance. The data were also modelled.

3. Results

The test site positions are shown on figure 1 whilst the data are tabulated and the models reproduced in Appendix 2. Although not directly equivalent to the tabulated values (apparent resistivity is an average or bulk sample of resistivity for a particular electrode separation) the following resistivity values may be used as a guide to corrosion potential:

- < 20 ohm metres – severe;
- 20-100 ohm metres – moderate;
- 100-300 ohm metres – mild;
- >300 ohm metres – unlikely.

By convention only, the depth that is assigned to a particular (apparent) resistivity value is equivalent to the electrode separation used to measure that value. A better indication of resistivity variations is obtained from the modelled data, and plotting the pipeline depth onto the models will assist in a final classification in areas that are particularly heterogeneous.

Yours sincerely,

A handwritten signature in blue ink, appearing to read 'R.W. Day', is written over a light blue grid background.

R.W.Day. Pr.Sci.Nat.

Appendix 1

B19 Pipeline Soil Resistivity Survey

Traverse start and end points (Lo27 WGS84)

Line-St.	LoY	LoX
1-00	-93232	2952797
1-20	-93268	2952787
2-00	-93149	2952940
2-20	-93166	2952949
3-00	-93115	2953076
3-20	-93057	2953084
4-00	-93043	2953204
4-20	-93056	2953211
5-00	-92994	2953389
5-20	-93011	2953383
6-00	-93004	2953546
6-20	-93007	2953527
7-00	-93033	2953740
7-20	-93055	2953734
8-00	-92940	2953952
8-20	-92962	2953958
9-00	-92843	2954086
9-20	-92840	2954092
10-00	-92787	2954108
10-20	-92777	2954122
11-00	-92625	2954121
11-20	-92642	2954135
12-00	-92553	2954194
12-20	-92558	2954172
13-00	-92309	2954185
13-20	-92324	2954202
14-00	-92101	2954135
14-20	-92110	2954114
15-00	-91858	2954013
15-20	-91843	2954031

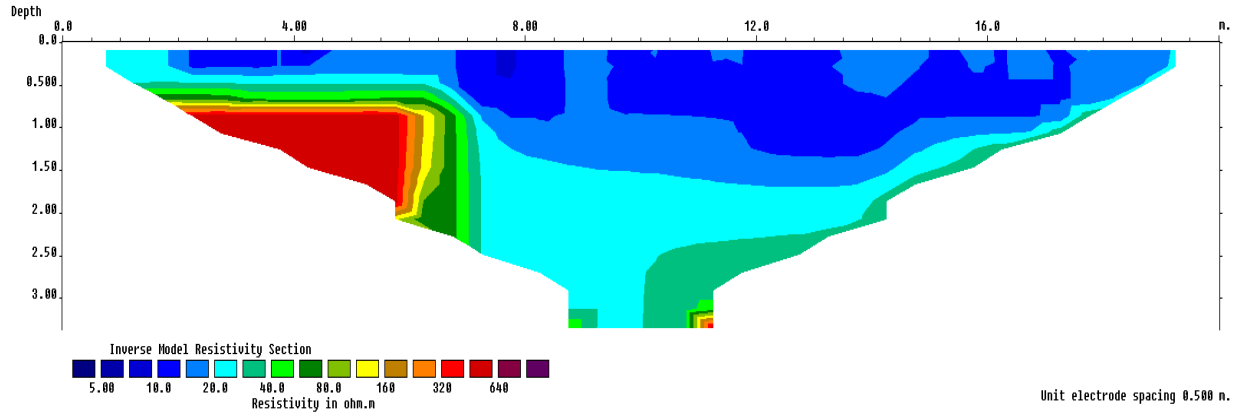
Line-St.	LoY	LoX
16-00	-91712	2954047
16-20	-91703	2954063
17-00	-91571	2954190
17-20	-91588	2954206
18-00	-91411	2954302
18-20	-91394	2954311
19-00	-91083	2954488
19-20	-91093	2954509
20-00	-90885	2954534
20-20	-90885	2954557
21-00	-90709	2954627
21-20	-90717	2954645
22-00	-90520	2954680
22-20	-90522	2954700
23-00	-90305	2954714
23-20	-90302	2954734
24-00	-90122	2954620
24-20	-90142	2954617
25-00	-90074	2954422
25-20	-90095	2954414
26-00	-90019	2954247
26-20	-90032	2954262
27-00	-90176	2954120
27-20	-90189	2954136
28-00	-90402	2953950
28-20	-90425	2953958
29-00	-90477	2953887
29-20	-90490	2953902
30-00	-90551	2953827
30-20	-90568	2953837

Appendix 2

B19 Pipeline Soil Resistivity Survey

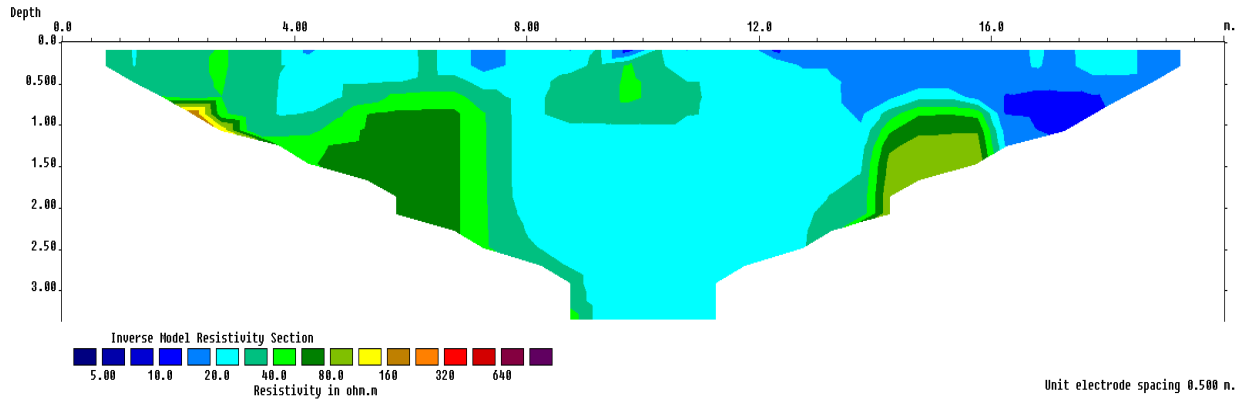
Tabulated and modelled results

Site 01



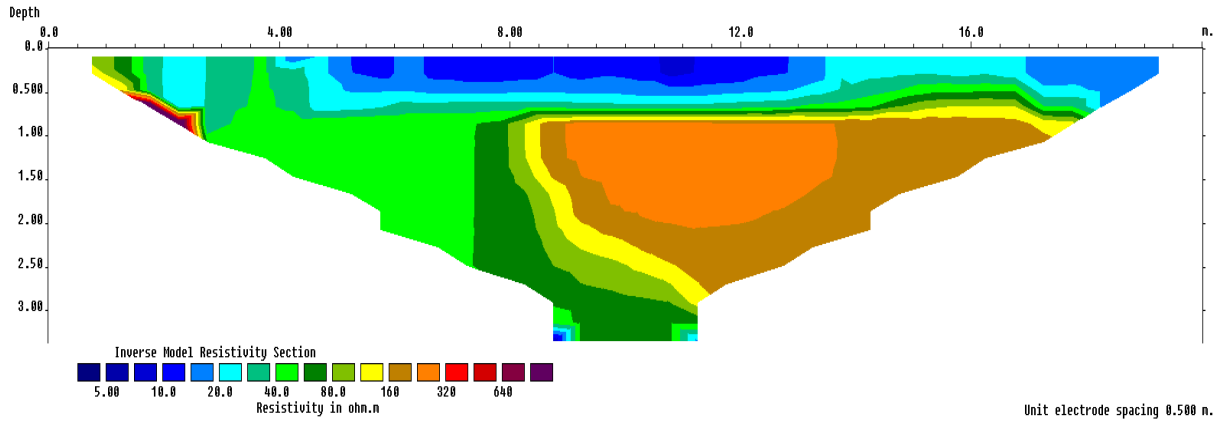
B19 Pipeline: 01 W-E		
A	Count	App.Res.
0.5	37	16.87
1	35	22.18
1.5	32	26.47
2	15	30.39
3	12	37.71
4	9	43.67
5	6	45.80
6	3	54.33

Site 02



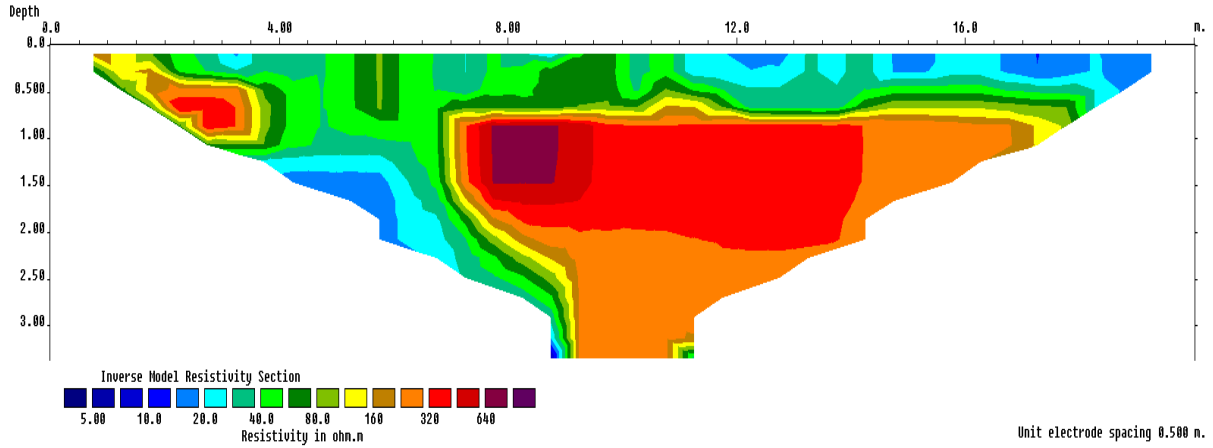
B19 Pipeline 02 W-E		
A	Count	App.Res.
0.5	37	25.08
1	35	28.34
1.5	32	30.80
2	15	32.31
3	12	33.87
4	9	34.65
5	6	34.61
6	3	35.47

Site 03



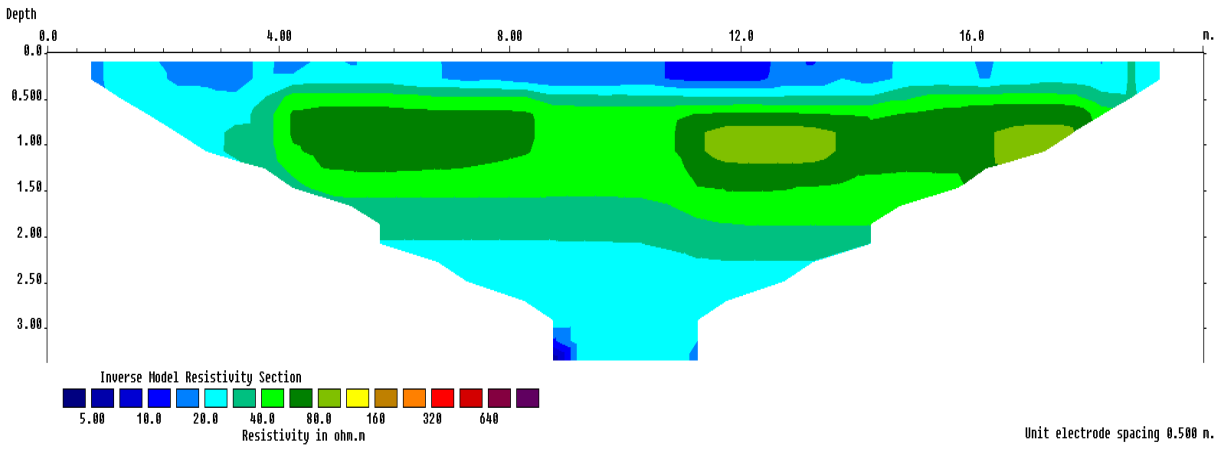
B19 Pipeline 03 W-E		
A	Count	App.Res.
0.5	37	27.87
1	35	37.84
1.5	32	43.88
2	15	50.08
3	12	56.13
4	9	55.00
5	6	48.46
6	3	44.99

Site 04



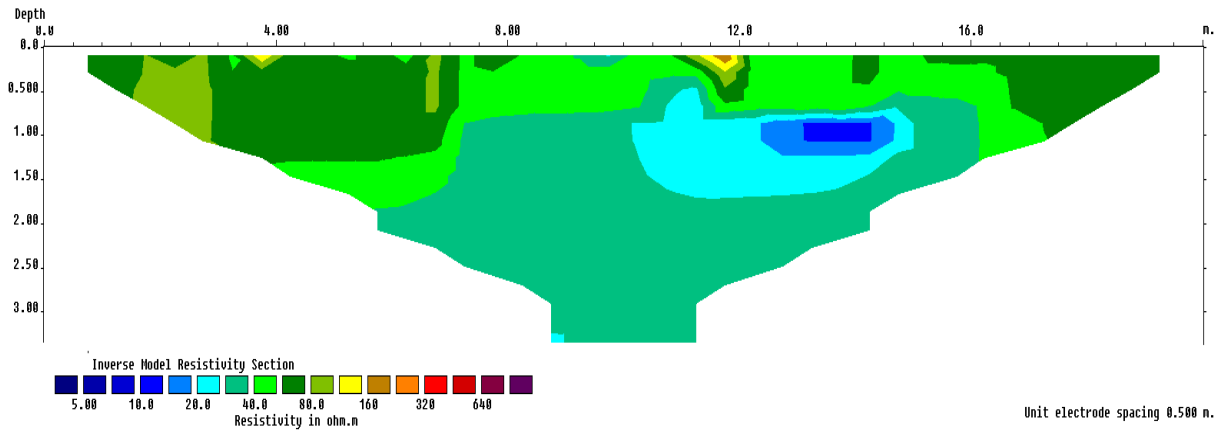
B19 Pipeline 04 W-E		
A	Count	App.Res.
0.5	37	42.70
1	35	56.76
1.5	32	65.64
2	15	72.15
3	12	81.73
4	9	79.45
5	6	76.19
6	3	72.68

Site 05



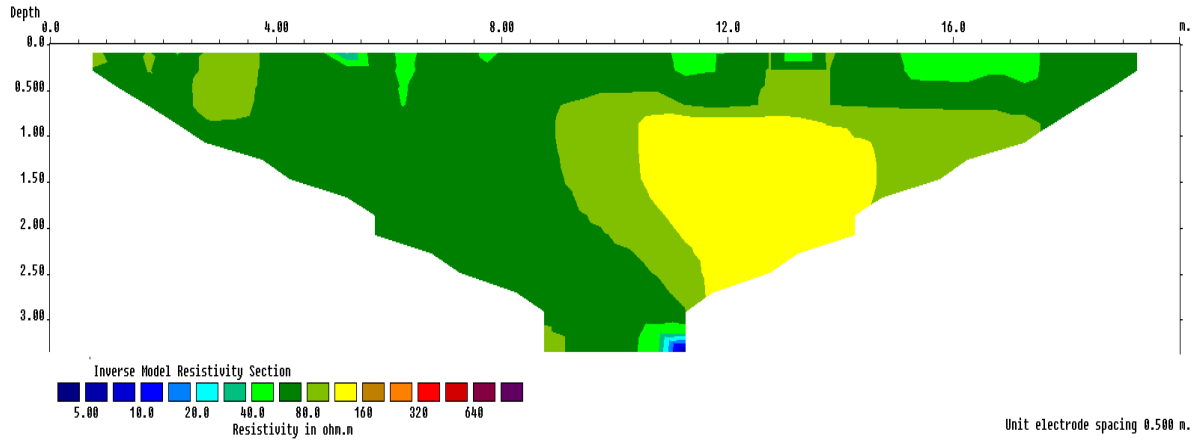
B19 Pipeline 05 W-E		
A	Count	App.Res.
0.5	37	22.79
1	35	30.93
1.5	32	34.16
2	15	34.64
3	12	32.57
4	9	28.54
5	6	23.92
6	3	20.52

Site 06



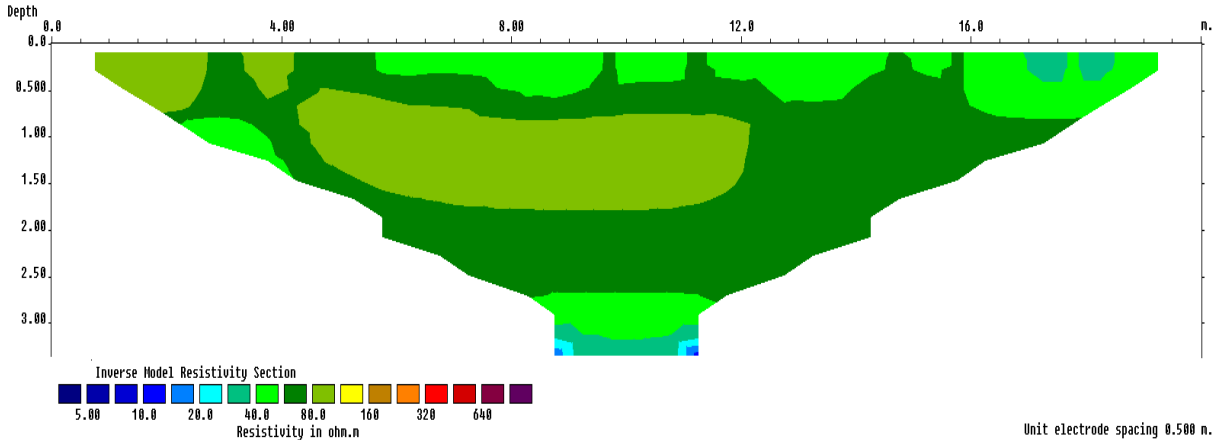
B19 Pipeline 06 N-S		
A	Count	App.Res.
0.5	37	56.85
1	35	51.62
1.5	32	44.81
2	15	40.65
3	12	35.97
4	9	33.88
5	6	31.86
6	3	29.79

Site 07



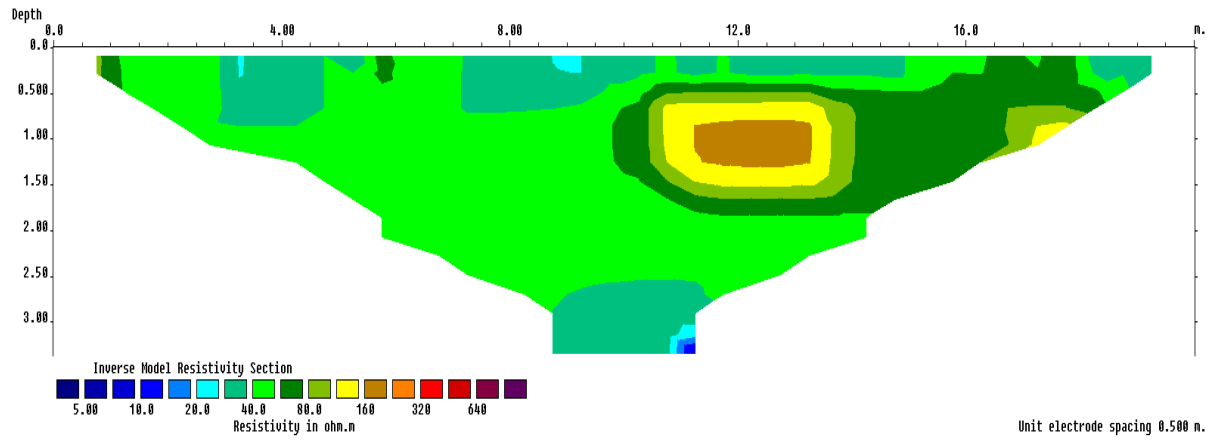
B19 Pipeline 07 W-E		
A	Count	App.Res.
0.5	37	68.21
1	35	74.37
1.5	32	77.32
2	15	77.83
3	12	77.01
4	9	73.85
5	6	69.78
6	3	60.25

Site 08



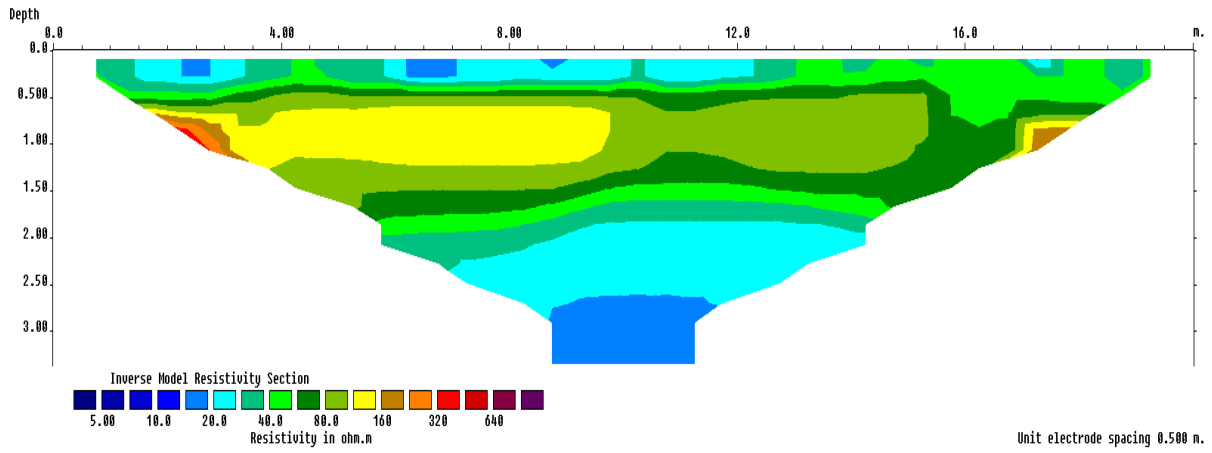
B19 Pipeline 08 W-E		
A	Count	App.Res.
0.5	37	59.90
1	35	63.49
1.5	32	64.00
2	15	62.47
3	12	55.86
4	9	46.56
5	6	37.57
6	3	32.13

Site 09



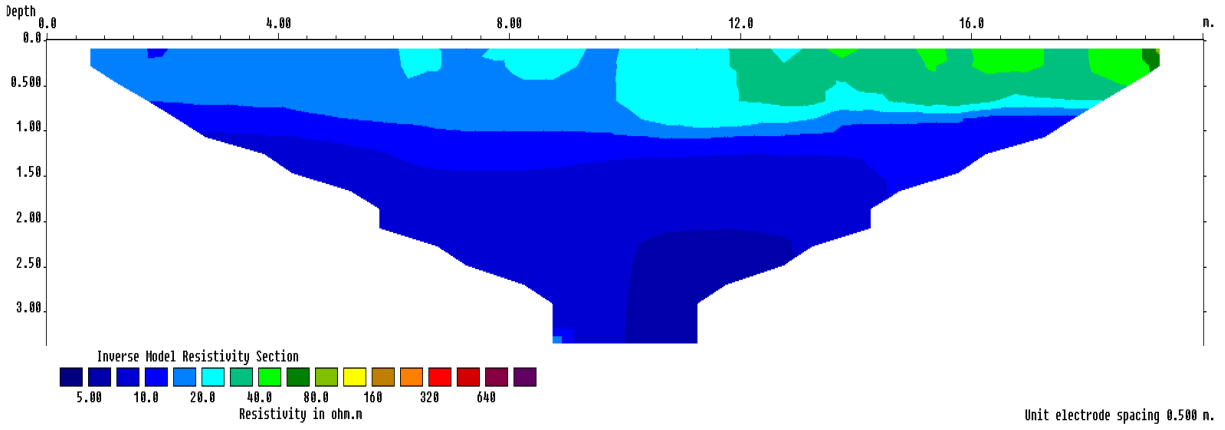
B19 Pipeline 09 W-E		
A	Count	App.Res.
0.5	29	46.52
1	28	52.91
1.5	26	54.13
2	11	51.75
3	8	48.22
4	7	41.34
5	5	37.09
6	3	32.62

Site 10



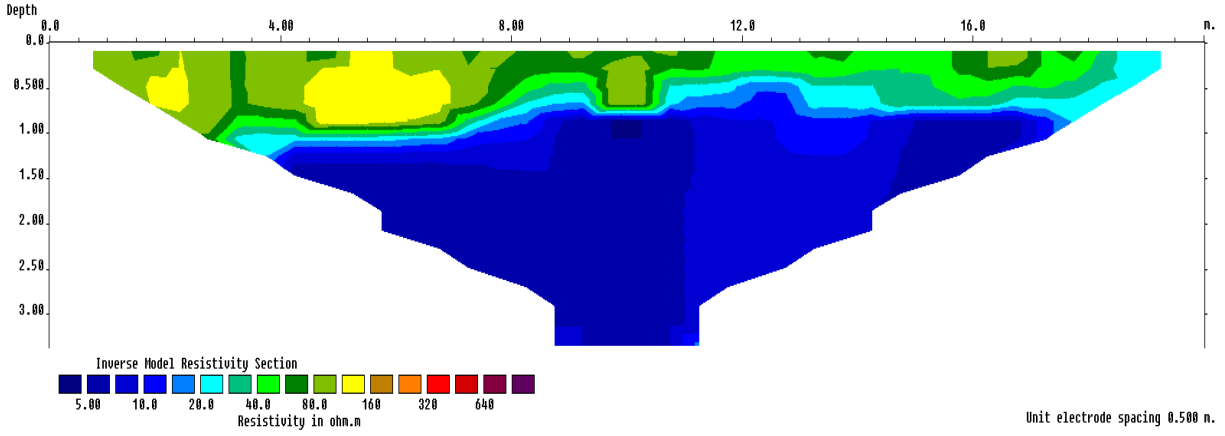
B19 Pipeline 10 N-S		
A	Count	App.Res.
0.5	37	38.40
1	35	52.58
1.5	32	56.99
2	15	57.22
3	12	51.13
4	9	42.13
5	6	34.20
6	3	30.31

Site 11



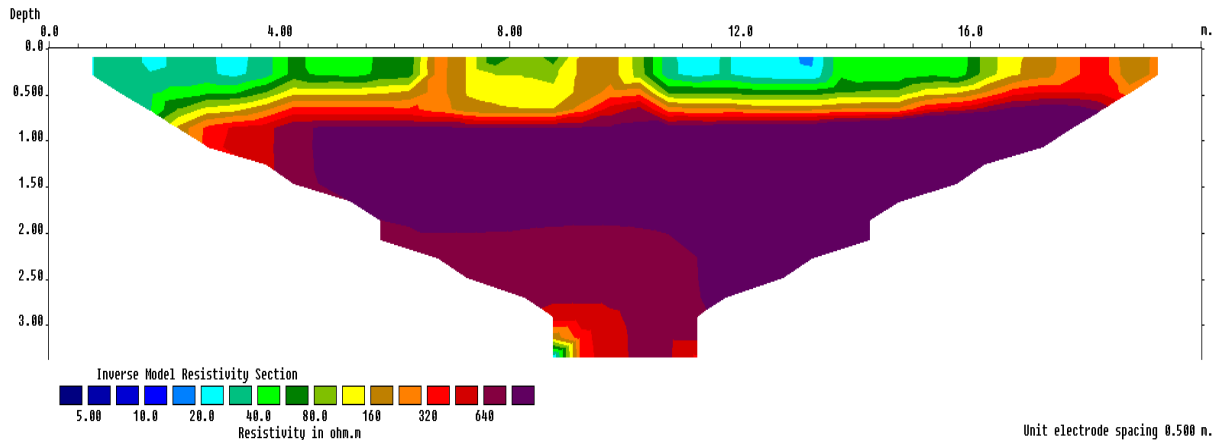
B19 Pipeline 11 W-E		
A	Count	App.Res.
0.5	37	23.69
1	35	19.29
1.5	32	15.58
2	15	13.26
3	12	10.66
4	9	9.70
5	6	9.75
6	3	10.28

Site 12



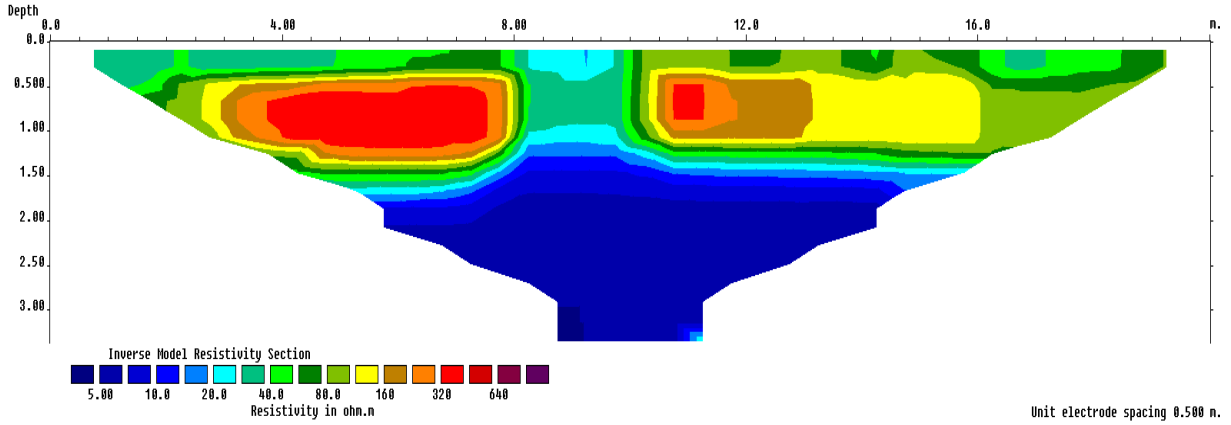
B19 Pipeline 12 N-S		
A	Count	App.Res.
0.5	37	61.37
1	35	43.66
1.5	32	27.53
2	15	18.02
3	12	10.81
4	9	9.32
5	6	9.11
6	3	9.41

Site 13



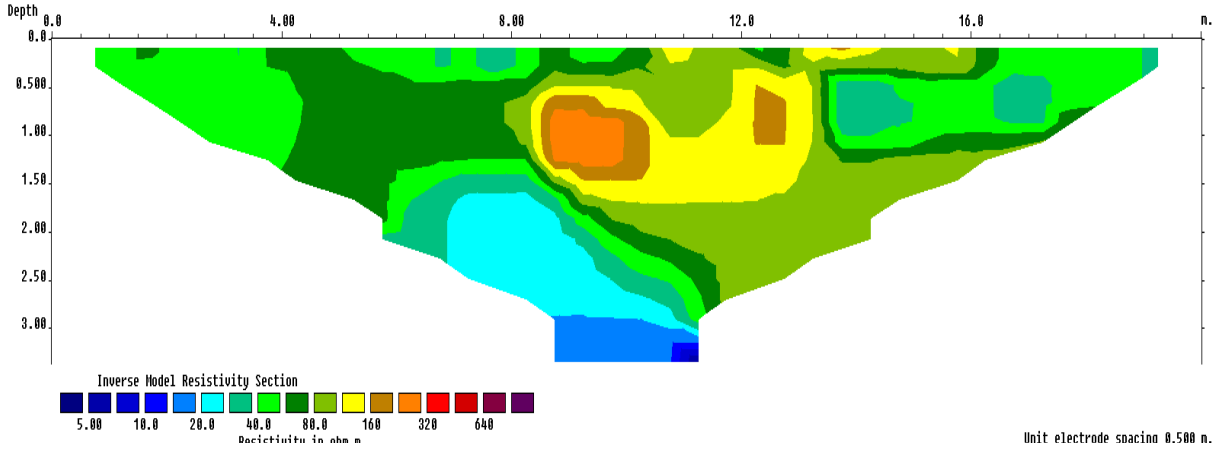
B19 Pipeline 13 W-E		
A	Count	App.Res.
0.5	37	110.51
1	35	181.65
1.5	32	230.06
2	15	265.69
3	12	303.82
4	9	346.70
5	6	413.81
6	3	394.73

Site 14



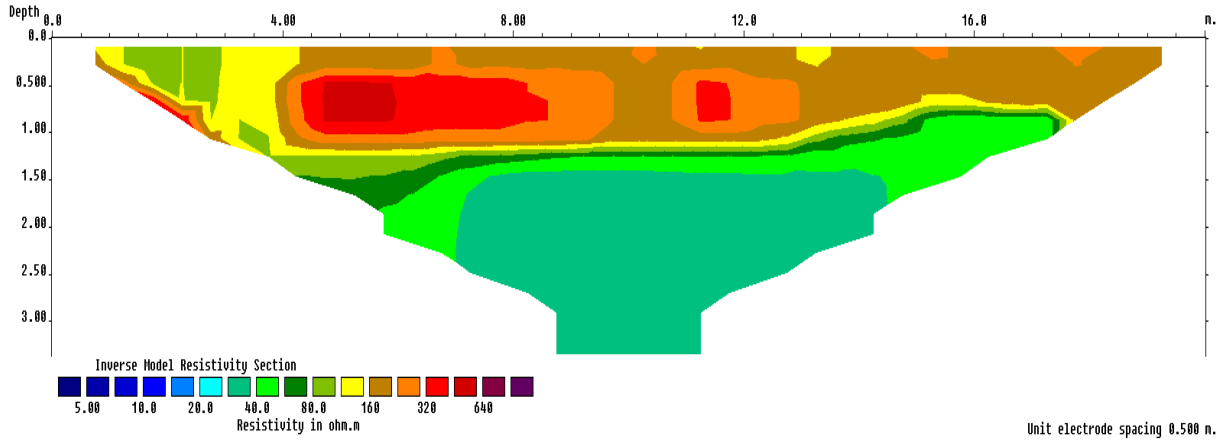
B19 Pipeline 14 N-S		
A	Count	App.Res.
0.5	37	63.20
1	35	78.51
1.5	32	73.59
2	15	59.76
3	12	34.71
4	9	23.34
5	6	17.39
6	3	14.83

Site 15



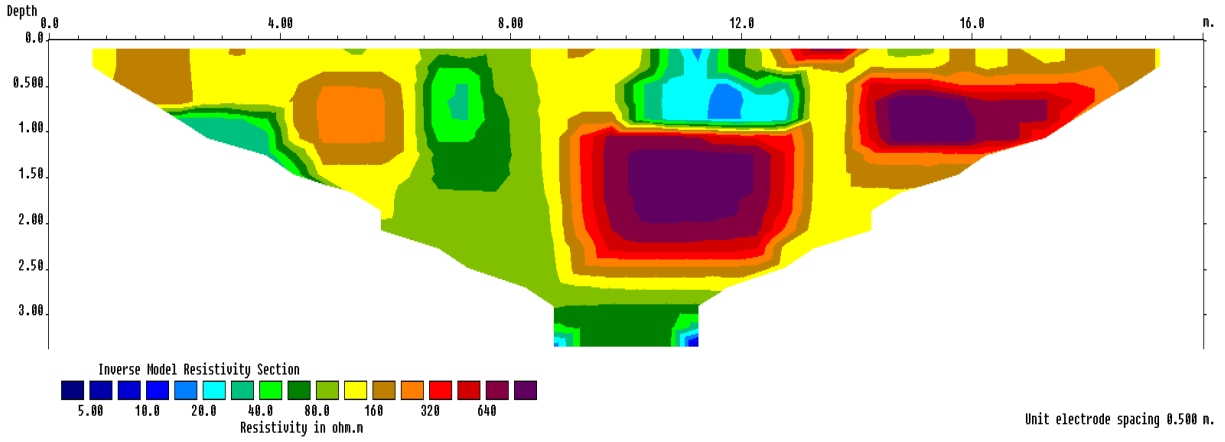
B19 Pipeline 15 N-S		
A	Count	App.Res.
0.5	37	61.25
1	35	64.96
1.5	32	66.20
2	15	63.47
3	12	51.38
4	9	40.98
5	6	33.32
6	2	27.34

Site 16



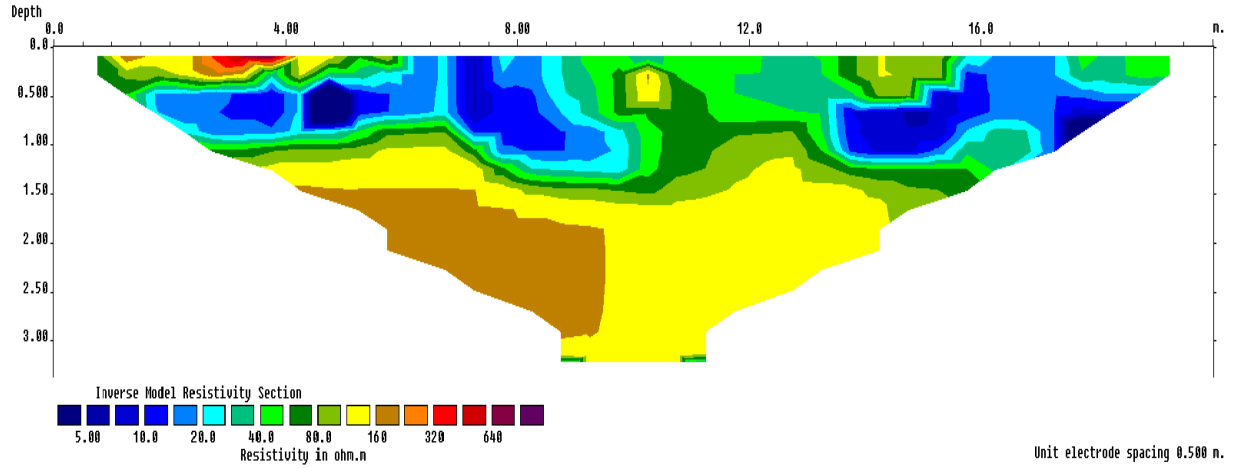
B19 Pipeline 16 N-S		
A	Count	App.Res.
0.5	37	185.98
1	35	178.63
1.5	32	145.51
2	15	114.30
3	12	73.56
4	9	54.69
5	6	44.79
6	3	41.21

Site 17



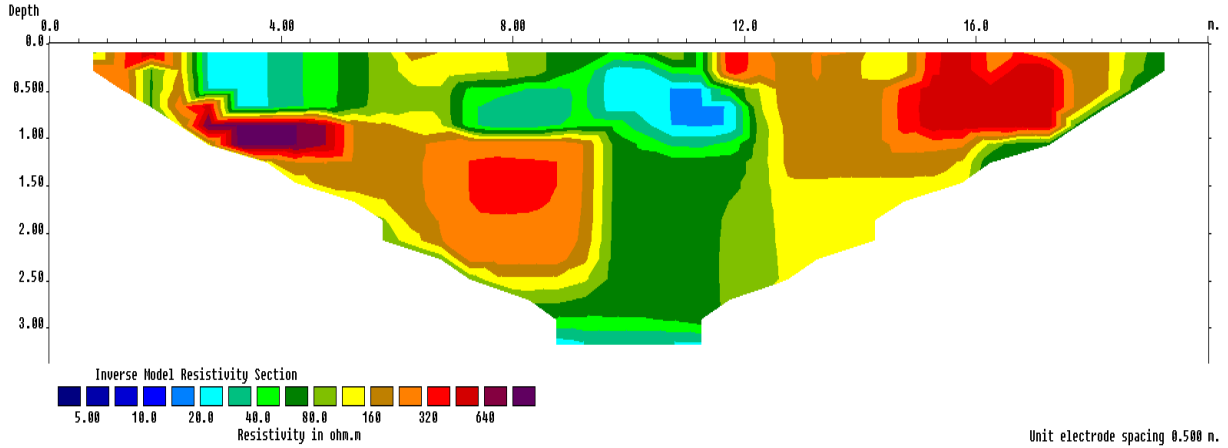
B19 Pipeline 17 W-E		
A	Count	App.Res.
0.5	37	136.36
1	35	142.57
1.5	32	138.72
2	15	130.10
3	12	116.16
4	9	87.51
5	6	62.26
6	3	37.13

Site 18



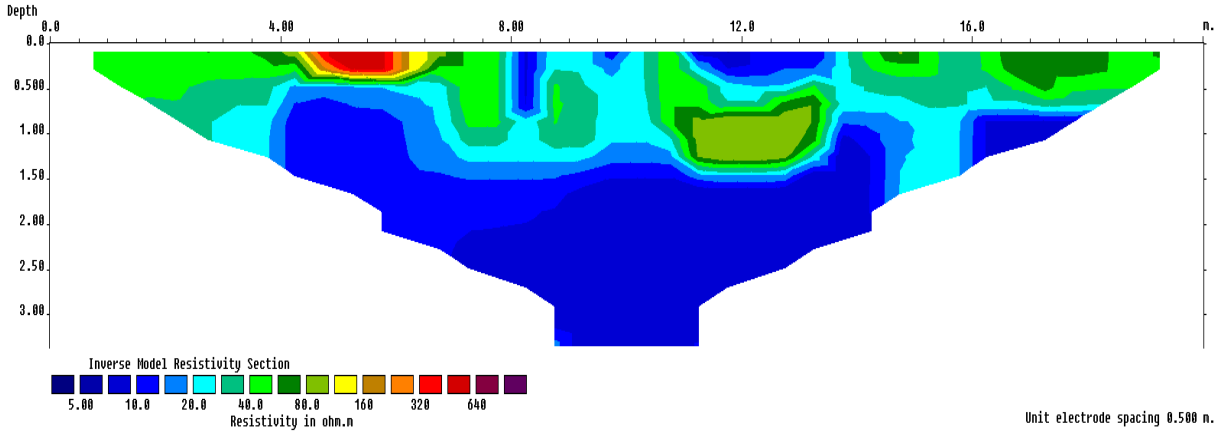
B19 Pipeline 18 W-E		
A	Count	App.Res.
0.5	37	43.72
1	35	30.77
1.5	32	34.11
2	15	37.74
3	12	42.32
4	9	41.99
5	6	38.82
6	3	37.21

Site 19



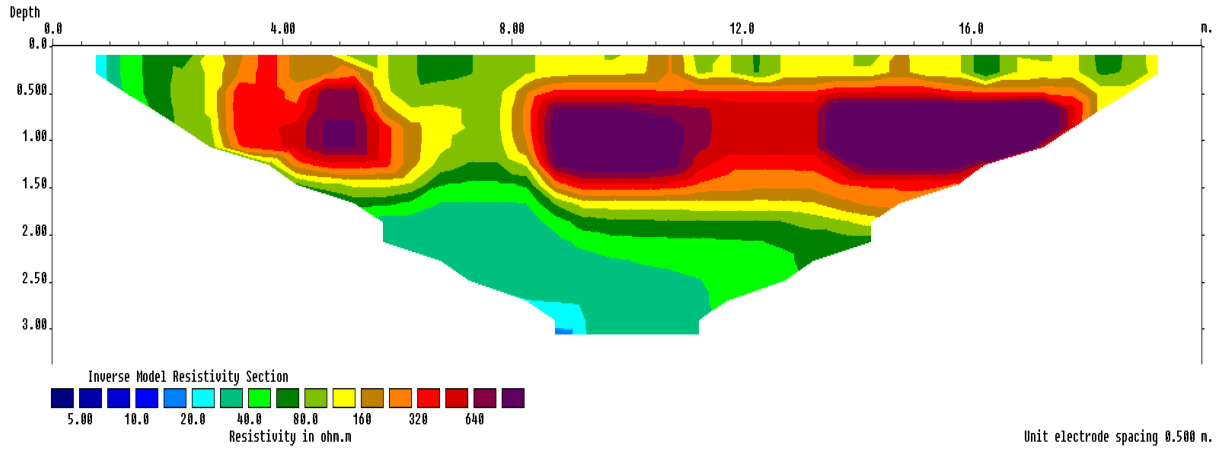
B19 Pipeline 19 N-S		
A	Count	App.Res.
0.5	37	133.92
1	35	123.45
1.5	32	109.80
2	15	96.61
3	12	79.62
4	9	61.57
5	6	41.91
6	2	30.38

Site 20



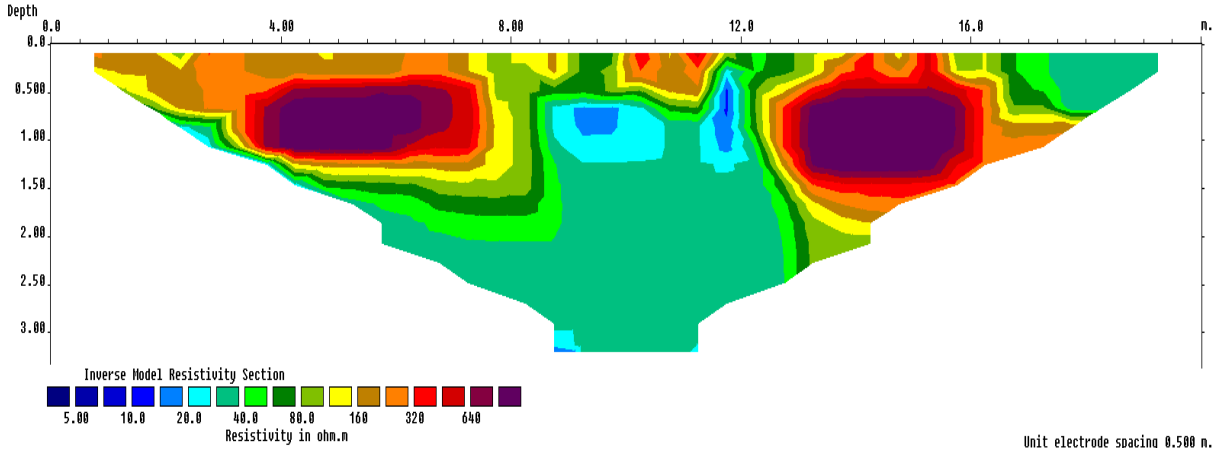
B19 Pipeline 20 N-S		
A	Count	App.Res.
0.5	37	41.36
1	35	27.00
1.5	32	21.81
2	15	18.22
3	12	14.93
4	9	12.16
5	6	11.50
6	3	11.76

Site 21



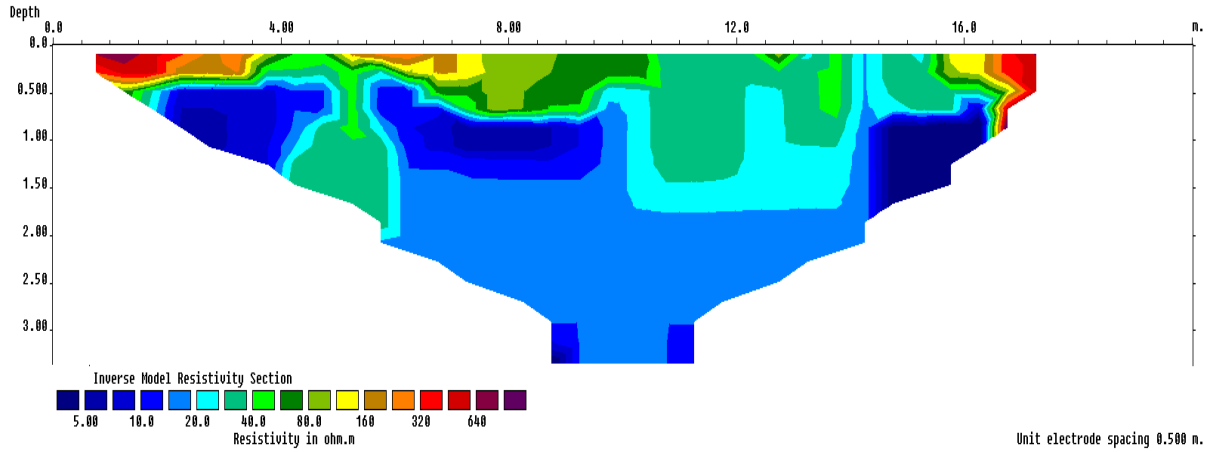
B19 Pipeline 21 N-S		
A	Count	App.Res.
0.5	37	151.86
1	35	203.77
1.5	32	225.50
2	15	214.83
3	12	172.14
4	9	128.49
5	6	99.76
6	3	63.51

Site 22



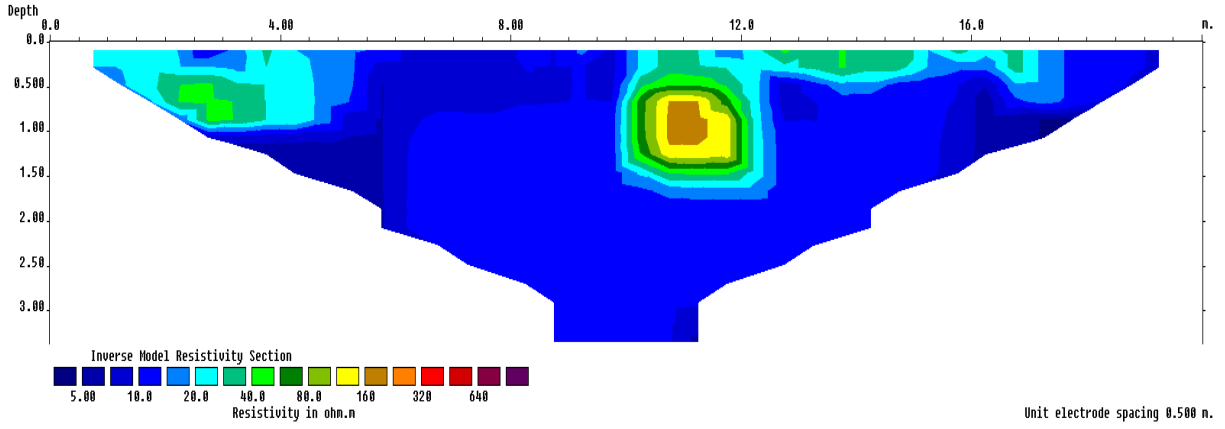
B19 Pipeline 22 N-S		
A	Count	App.Res.
0.5	37	153.75
1	35	168.10
1.5	32	154.38
2	15	123.92
3	12	73.49
4	9	52.35
5	6	32.06
6	3	22.97

Site 23



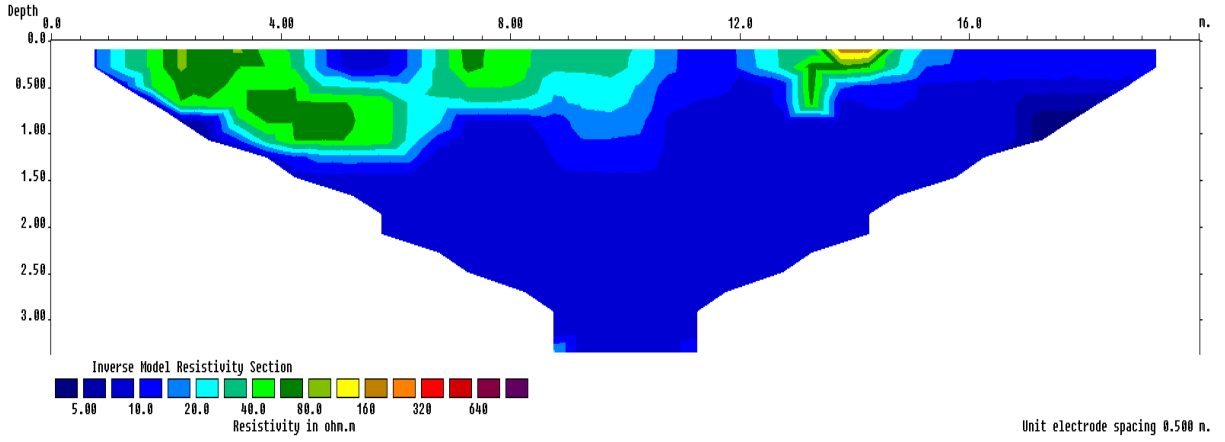
B19 Pipeline 23 N-S		
A	Count	App.Res.
0.5	37	123.58
1	35	59.67
1.5	32	30.58
2	15	21.10
3	12	14.35
4	9	12.21
5	6	10.92
6	3	11.49

Site 24



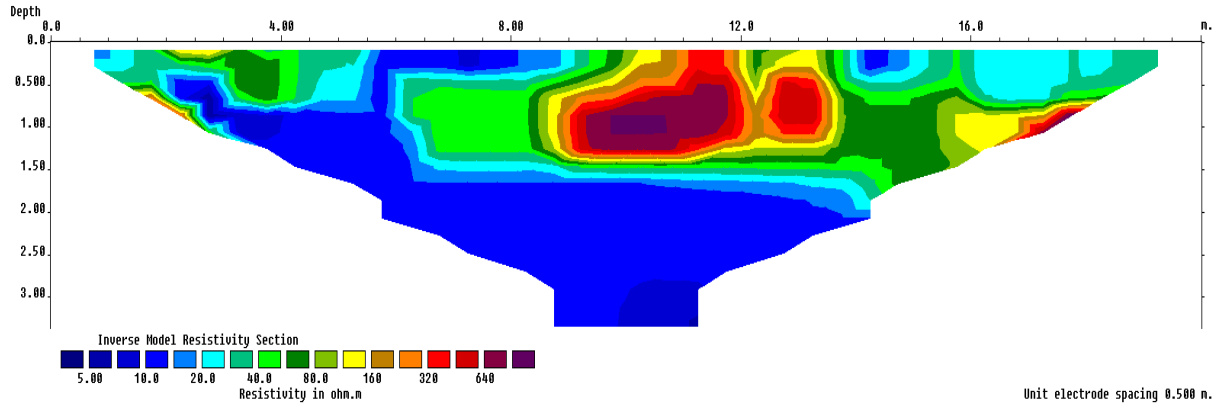
B19 Pipeline 24 W-E		
A	Count	App.Res.
0.5	37	17.14
1	35	15.42
1.5	32	13.77
2	15	12.58
3	12	11.10
4	9	10.02
5	6	10.35
6	3	10.64

Site 25



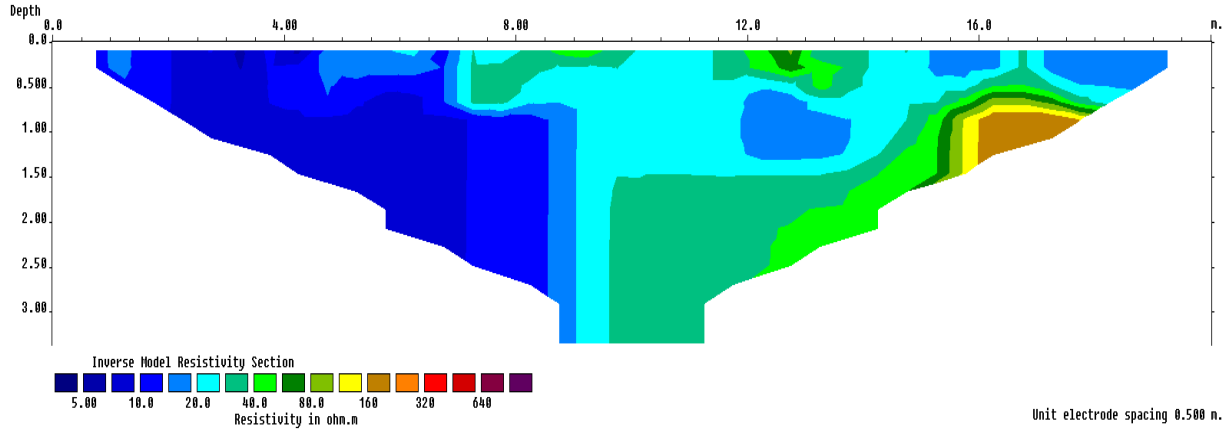
B19 Pipeline 25 W-E		
A	Count	App.Res.
0.5	37	23.92
1	35	17.24
1.5	32	14.72
2	15	13.21
3	12	10.82
4	9	10.34
5	6	10.73
6	3	11.34

Site 26



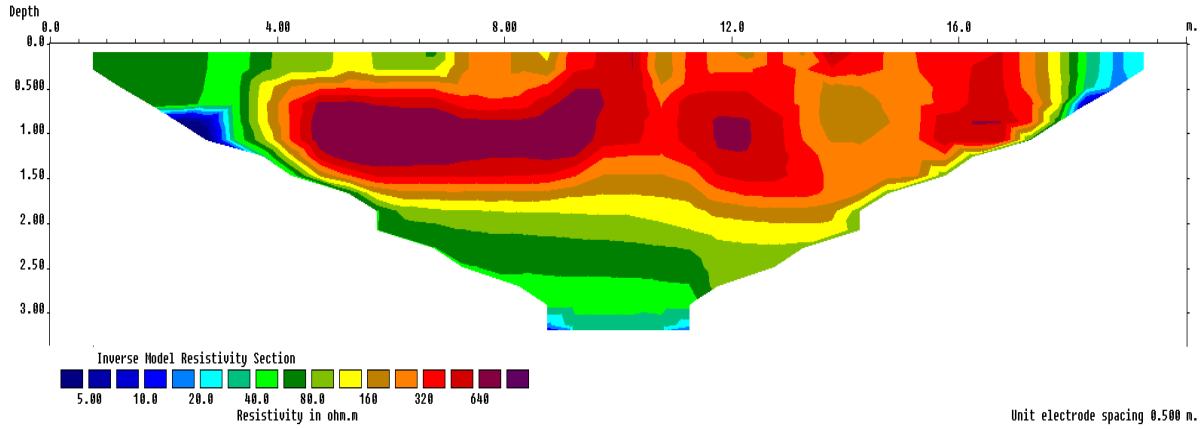
B19 Pipeline 26 NW-SE		
A	Count	App.Res.
0.5	37	56.87
1	35	64.15
1.5	32	58.80
2	15	50.36
3	12	37.78
4	9	28.56
5	6	24.31
6	3	24.96

Site 27



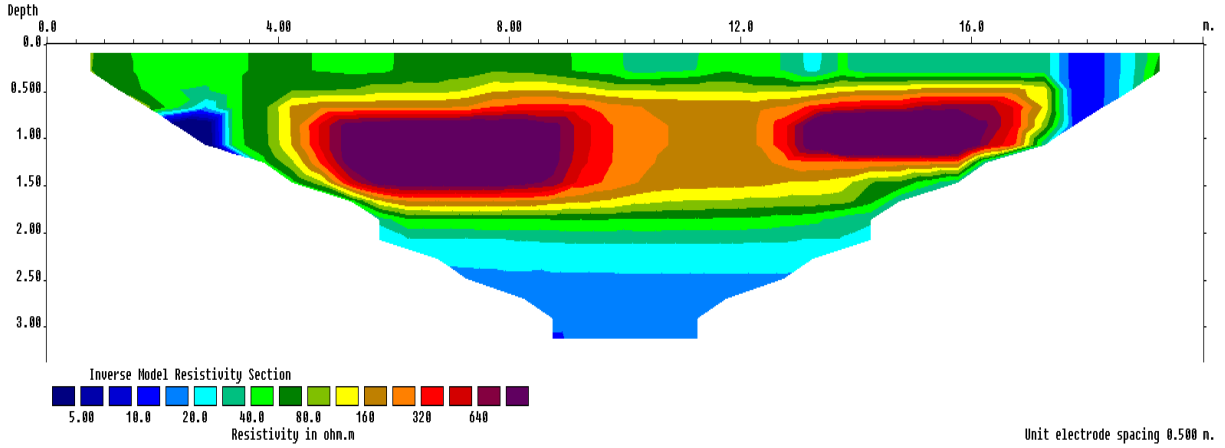
B19 Pipeline 27 NW-SE		
A	Count	App.Res.
0.5	37	21.59
1	35	22.28
1.5	32	23.48
2	15	24.26
3	12	25.03
4	9	25.59
5	6	27.21
6	3	28.14

Site 28



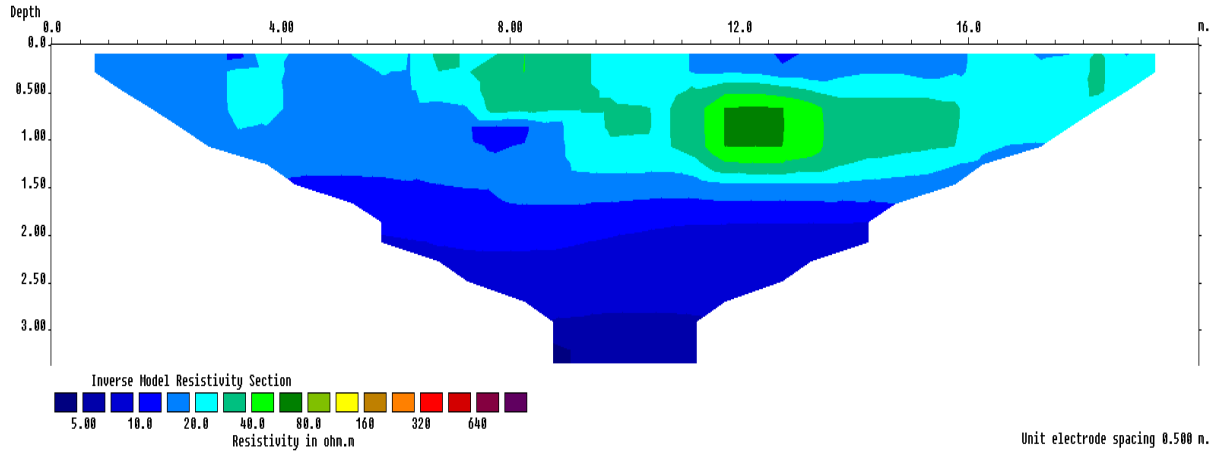
B19 Pipeline 28 W-E		
A	Count	App.Res.
0.5	37	215.28
1	35	230.46
1.5	32	223.92
2	15	193.09
3	12	125.70
4	9	70.63
5	6	37.00
6	3	15.15

Site 29

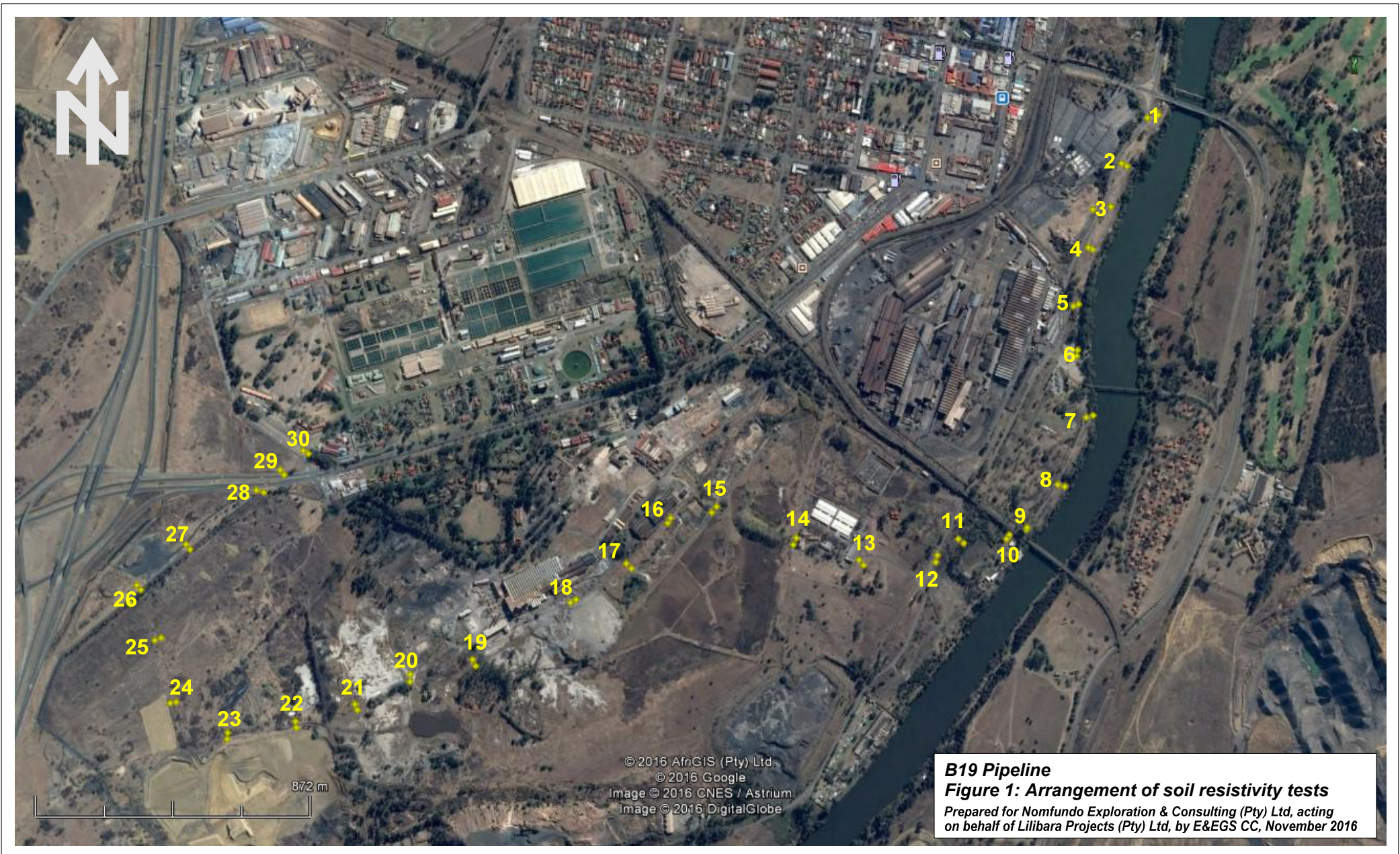


B19 Pipeline 29 W-E		
A	Count	App.Res.
0.5	37	56.99
1	35	83.69
1.5	32	104.94
2	15	111.56
3	12	92.68
4	9	67.03
5	6	36.22
6	3	15.61

Site 30



B19 Pipeline 30 W-E		
A	Count	App.Res.
0.5	37	21.90
1	35	21.63
1.5	32	19.95
2	15	17.55
3	12	13.23
4	9	10.15
5	6	8.49
6	3	7.56



B19 Pipeline
Figure 1: Arrangement of soil resistivity tests
Prepared for Nomfundo Exploration & Consulting (Pty) Ltd, acting on behalf of Lilibara Projects (Pty) Ltd, by E&EGS CC, November 2016